CONTRACT SPECIFICATIONS

For The Construction Of:

CHEYNE LANDFILL SEPTAGE LAGOONS

SP 3265

Yakima County Public Services Project
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## INFORMATIONAL BID DOCUMENTS

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INFORMATIONAL BID DOCUMENTS
INSTRUCTIONS TO BIDDERS

DELIVERY OF PROPOSALS

Sealed bids will be received at the following location before the specified time:

Yakima County Public Services, Fourth Floor County Courthouse, 128 N. 2nd Street, Yakima, Washington 98901 until 2:00 p.m. of the bid opening date.

Each proposal, or bid shall be completely sealed in a separate package, addressed to the Director of Public Services of Yakima County with the name of the improvements for which the bid is submitted plainly written on the outside of the package. No oral, telephonic, facsimile, or telegraphic Bids or modifications shall be accepted.

DATE OF OPENING BIDS

The bid opening date for this project shall be August 19, 2009. The bids shall be opened and read after 2:00 p.m. on that date at the following location:

Yakima County Public Services Office, 4th Floor Yakima County Courthouse, 128 North 2nd Street, Yakima, Washington 98901.

Bidders are encouraged to inspect the site and work area prior to submitting a bid. A prebid conference will be held at 2:00 P.M. on Tuesday, August 11, 2009 at the Cheyne Landfill Site, located at the north end of Cheyne Road, approximately 6 miles north of Zillah, Washington; attendance is recommended. All visitors to the site must check in with the site attendant at the scale house at Cheyne Landfill upon arrival.

RIGHT TO REJECT BIDS:

The right is reserved to reject any and all proposals, to accept the proposal or proposals deemed best for the County, or to advertise for new proposals, when in the opinion of the Board the best interest of the County shall be promoted thereby.

PROPOSAL GUARANTY:

A certified check, cashier check, cash or bid bond made payable to the Treasurer of the County of Yakima for an amount equal to at least five percent (5%) of the total amount bid must accompany each bid as evidence of good faith and as a guarantee that if awarded the Contract the bidder shall execute the Contract and give Bond as required.

FORM FURNISHED:

Each bid must be made on the form attached to these Specifications.

Yakima County in accordance with Title VI of the Civil Rights Act of 1964 and 78 Stat. 252, 42 USC 2000d—42 and Title 49, Code of Federal Regulations, Department of Transportation, Subtitle A, Office of the Secretary, part 21, Nondiscrimination in Federally assisted programs of the Department of Transportation issued pursuant to such Act, hereby notifies all bidders that it will affirmatively ensure that any contract entered into pursuant to this advertisement, minority business enterprises will be afforded full opportunity to submit bids in response to this invitation and will not be discriminated against on the grounds of race, color, sex, or national origin in consideration for an award.

YAKIMA COUNTY IS AN EQUAL OPPORTUNITY EMPLOYER
PROPOSAL

This certifies that the undersigned has examined the location of the noted project:

SP 3265 - CHEYNE LANDFILL SEPTAGE LAGOONS

And that the Plans, Specifications and Contract governing the work embraced in these improvements, and the method by which payment will be made for said work, is understood. The undersigned hereby proposes to undertake and complete the work embraced in these improvements, or as much as can be completed with the money available, in accordance with the said Plans, Specifications, and Contract, and the following schedule of rates and prices:

NOTE: Unit Prices for all items, all extensions, and total amount of bid shall be shown. Sales Tax shall be included in Unit Prices. No oral, telephonic, facsimile, or telegraphic Bids or modifications shall be considered or accepted.

<table>
<thead>
<tr>
<th>Item No.</th>
<th>Description</th>
<th>Approx. Quantity</th>
<th>Unit</th>
<th>Unit Price</th>
<th>Total Item Amount</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>PREPARATION</td>
<td></td>
<td></td>
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</tr>
<tr>
<td>1</td>
<td>MOBILIZATION</td>
<td>1</td>
<td>L.S.</td>
<td>$</td>
<td>$</td>
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<td></td>
<td>GRADING</td>
<td></td>
<td></td>
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<td>2</td>
<td>POND EXCAVATION, INCL. HAUL</td>
<td>102.300</td>
<td>C.Y.</td>
<td>$</td>
<td>$</td>
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<td>3</td>
<td>EMBANKMENT COMPACTION</td>
<td>8.100</td>
<td>C.N.</td>
<td>$</td>
<td>$</td>
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<tr>
<td>4</td>
<td>GEOMEMBRANE DOUBLE LINER SYSTEM</td>
<td>205.000</td>
<td>S.F.</td>
<td>$</td>
<td>$</td>
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<tr>
<td></td>
<td>DRAINAGE</td>
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<tr>
<td>5</td>
<td>GEOTEXTILE</td>
<td>205.000</td>
<td>S.F.</td>
<td>$</td>
<td>$</td>
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<td>6</td>
<td>GEOCELL</td>
<td>160</td>
<td>S.F.</td>
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<td>$</td>
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<td>7</td>
<td>LEACHATE PUMP SYSTEM</td>
<td>4</td>
<td>EACH</td>
<td>$</td>
<td>$</td>
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<td>8</td>
<td>PROTECTIVE COVER SOIL</td>
<td>7.600</td>
<td>C.Y.</td>
<td>$</td>
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<td>9</td>
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<td>1</td>
<td>L.S.</td>
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<td>10</td>
<td>WALKWAY AND DECANTER VALVE STRUCTURE</td>
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<td>EACH</td>
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<td>11</td>
<td>PVC SANITARY SEWER PIPE 12 IN. DIAM.</td>
<td>600</td>
<td>L.F.</td>
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<td>12</td>
<td>SCHEDULE A CULVERT PIPE 18 IN. DIAM.</td>
<td>330</td>
<td>L.F.</td>
<td>$</td>
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<td>13</td>
<td>SURFACE WATER DITCH</td>
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<td>L.F.</td>
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<td>SURFACING</td>
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<td>14</td>
<td>CRUSHED SURFACING TOP COURSE</td>
<td>5.800</td>
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<td>$</td>
<td>$</td>
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<td></td>
<td>HOT MIX ASPHALT</td>
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<td>HMA CL. 1/2 IN. PG 64-28</td>
<td>186</td>
<td>TON</td>
<td>$</td>
<td>$</td>
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<tr>
<td></td>
<td><strong>EROSION CONTROL AND PLANTING</strong></td>
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<tr>
<td>16</td>
<td>SEEDING, FERTILIZING, AND MULCHING</td>
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<td>17</td>
<td>SILT FENCE</td>
<td>1,100 L.F.</td>
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<td>18</td>
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<td>5,000 SQ.</td>
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<td><strong>OTHER ITEMS</strong></td>
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<td>20</td>
<td>WELL DECOMMISSION</td>
<td>1 L.S.</td>
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<td>CONC. CLASS 4000</td>
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<td>ILLUMINATION AND ELECTRICAL SYSTEM COMPLETE</td>
<td>1 L.S.</td>
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<td>SPCC PLAN</td>
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<td>26</td>
<td>MINOR CHANGES</td>
<td>EST. F.A.</td>
<td>$15,000.00</td>
<td>$15,000.00</td>
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**Sub Total** $   

**Sales Tax @ 7.90%** $   

**Total Bid** $   

**NOTE:** The Total Bid shall be used for contract and bond amount.
PROPOSAL – Continued

The bidder is hereby advised that by signature of this proposal he/she is deemed to have acknowledged all requirements and signed all certificates contained herein.

A proposal guaranty in an amount of five percent (5%) of the total bid, based upon the approximate estimate of quantities at the above prices and in the form as indicated below, is attached hereto:

CASH [ ] IN THE AMOUNT OF _______________________

CASHIER’S CHECK [ ] _______________________________ DOLLARS

CERTIFIED CHECK [ ] ($_______) PAYABLE TO THE COUNTY TREASURER

PROPOSAL BOND [ ] IN THE AMOUNT OF 5 PERCENT (5%) OF THE BID

Bidder acknowledges receipt of the following Addendums:

No. Date

The undersigned has telephoned the Office of the Yakima County Engineer for verification of the number of Addendums issued.

SIGNATURE OR AUTHORIZED OFFICIAL(S)

Title: __________________________

Firm Name: __________________________

Address: __________________________

Phone: __________________________

Washington Registration No.: __________________________

Federal ID Tax No.: __________________________

UBI No.: __________________________

E-Mail: __________________________

Signed and sworn (or affirmed) before me on __________________________ Date

_______________________________

NOTARY PUBLIC

My appointment expires __________________________ (Seal and Stamp)

NOTE: (1) This proposal is not transferable and any alteration of the firm’s name entered hereon without prior permission from the County Engineer shall be cause for considering the proposal irregular and subsequent rejection of the bid.

(2) Please refer to Section 1-02.6 of the Standard Specifications, re: “Preparation of Proposal”

(3) Should it be necessary to modify this proposal either in writing or by electronic means, please make reference to the following proposal number in your communications SP 3265.
**SUBCONTRACTORS**

1. List the names of the subcontractors with whom the bidder will subcontract if awarded the contract. Bidder may name itself for any or all of the work, including one or more categories of work.

2. For all other types and scope of work, the County will assume the bidder will perform with its own forces any and all types and scope of work for which a subcontractor is not identified.

3. Bidder shall not list more than one subcontractor for each category of work identified, unless subcontractors vary with bid alternates, in which case the bidder must indicate which subcontractor will be used for which alternate.

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<tr>
<th>Type/Scope of Work</th>
<th>Name and Address of Subcontractor Or Bidder</th>
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LETTER OF RESPONSIBILITY

Date: 
County Project No.: SP 3265

TO:
BOARD OF COUNTY COMMISSIONERS OF YAKIMA COUNTY, WASHINGTON
(Party awarding principal contract)

Dear Sirs:

I hereby maintain that I am a responsible bidder as contemplated by the policies of the State of Washington (Chapter 157, Laws of Washington of 1937).

a. My permanent place of business is ____________________________,
   which I have maintained for ________ years.

b. I have adequate plant equipment to do expeditiously and properly the work contemplated for Yakima County, Washington.

DESCRIPTION OF WORK:

SP 3265 – Cheyne Landfill Septage Lagoons

I have the following equipment available for this work:

____________________________________________________________________
____________________________________________________________________


c. I have adequate funds to promptly meet obligations incident to this work.
   Bank reference: ________________________________
   ____________________________________________
   ____________________________________________


d. I have had experience in this class of work, having constructed the following improvements.

I hereby certify that the above is a true and accurate statement.

Very truly yours,

_________________________________________
Contractor

NOTE: This sheet need not be submitted, unless so requested by the Engineer subsequent to opening of bid. This "letter of responsibility" shall not be construed to be a request for Prequalification of bidder.
DEFINITION OF TERMS

In interpreting these specifications, the following definitions shall prevail:


SECRETARY OF TRANSPORTATION: Secretary of Transportation of the State of Washington.

BOARD: The Board of County Commissioners of Yakima County.

ENGINEER: County, or construction engineer, or his duly authorized assistants (including Geologist) by whom all explanations and directions necessary for the satisfactory prosecution and completion of the work described in these specifications will be given.

CONTRACTOR: The person, firm, co-partnership, or corporation, or any lawful agent of such person, firm, partnership or corporation constituting one of the principals to the contract and undertaking to perform the work herein specified.

CONTRACT: The Agreement between the Contractor and the County of Yakima acting through the Board of County Commissioners. The contract shall include the accepted “Proposal”, “Plans”, “Specifications” and “Contract Bond”, also any and all supplemental agreements which reasonably could be required to complete the construction of the work in a substantial and acceptable manner.

PROPOSAL: The written offer, or copy thereof of the bidder to perform the work proposed.

PLANS: The officially approved drawings, or reproductions thereof attached to this contract.

SPECIFICATIONS: The directions, provisions and requirements contained herein, together with all written agreements made, or to be made pertaining to the method and manner of performing the work, or to the quantities and qualities of materials to be furnished under the contract.

CONTRACT BOND: The approved form of security furnished by the Contractor and his surety as a guarantee of good faith on the part of the Contractor to execute the work in accordance with the terms of the contract.

LABORATORY: The laboratories of the Department of Transportation, or other laboratories designated by the engineer.

AMOUNT OF THE CONTRACT: For the purpose of awarding the contract and determining the amount of the bond, the lump sum bid, or the summation of the products of the approximate quantities shown on the plans or otherwise stated by the unit prices will be considered the total amount of the bid and the full amount of the contract price.
NON-COLLUSION DECLARATION

I, by signing the proposal, hereby declare, under penalty of perjury under the laws of the United States that the following statements are true and correct:

1. That the undersigned person(s), firm, association or corporation has (have) not, either directly or indirectly, entered into any agreement, participated in any collusion, or otherwise taken any action in restraint of free competitive bidding in connection with the project for which this proposal is submitted.

2. That by signing the signature page of this proposal, I am deemed to have signed and have agreed to the provisions of this declaration.

NOTICE TO ALL BIDDERS

To report bid rigging activities call:

1-800-424-9071

The U. S. Department of Transportation (USDOT) operates the above toll-free “hotline” Monday through Friday, 8:00 a.m. to 5:00 p.m., eastern time. Anyone with knowledge of possible bid rigging, bidder collusion, or other fraudulent activities should use the “hotline” to report such activities.

The “hotline” is part of USDOT’s continuing effort to identify and investigate highway construction contract fraud and abuse and is operated under the direction of the USDOT Inspector General. All information will be treated confidentially and caller anonymity will be respected.
Certification Regarding
Debarment, Suspension, Ineligibility and Voluntary Exclusion
Lower Tier Covered Transactions

This certification is required by the regulations implementing Executive Order 12549, Debarment and Suspension, 29 CFR Part 98, Section 98.510, Participant’s responsibilities. The regulations were published as Part VII of the May 26, 1998 Federal Register (pages 19160-19211).

(BEFORE COMPLETING CERTIFICATION, READ ATTACHED INSTRUCTIONS WHICH ARE AN INTEGRAL PART OF THE CERTIFICATION)

(1) The prospective recipient of federal assistance funds certifies, by submission of this proposal, that neither it nor its principals are presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participation in this transaction by any federal department or agency.

(2) Where the prospective recipient of federal assistance funds is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

Name and Title of Authorized Representative:

Signature __________ Date __________
CONTRACT

THIS AGREEMENT, made and entered into between Yakima County acting under and by virtue of Titles 36 and 39 RCW, hereinafter called the “COUNTY” and ____________________________, hereinafter called the “CONTRACTOR”.

That in consideration of the terms and conditions contained herein and attached and made a part of this agreement, the parties hereto covenant and agree as follows:

I. The CONTRACTOR shall do all work and furnish all tools and equipment for SP 3265: CHEYNE LANDFILL SEPTAGE LAGOONS, and shall perform any changes in the work in accordance with the Contract Documents.

II. The CONTRACTOR shall provide and bear the expense of all equipment, material and labor of any sort whatsoever that may be required for the transfer of materials and for constructing and completing the work provided for in the Contract Documents except those items mentioned therein to be furnished by Yakima County.

III. The COUNTY hereby promises and agrees to pay the CONTRACTOR according to the attached Specifications and the schedule of unit or itemized prices at the time and in the manner and upon the conditions provided for in the Contract Documents.

IV. The CONTRACTOR for itself, and for its heirs, executors, administrators, successors and assigns does hereby agree to the full performance of all the covenants herein contained upon the part of the CONTRACTOR.

V. It is further provided that no liability shall attach to the COUNTY by reason of entering into this Contract, except as expressly provided herein.

VI. The parties agree that, for the purpose of this agreement, the CONTRACTOR is an independent contractor and neither the CONTRACTOR nor any employee of the CONTRACTOR is an employee of the COUNTY. Neither the CONTRACTOR nor any employee of the CONTRACTOR is entitled to any benefits that the COUNTY provides its employees. The CONTRACTOR is solely responsible for payment of any statutory workers compensation or employer’s liability insurance as required by state law.

IN WITNESS WHEREOF, the CONTRACTOR has executed this instrument, on the date indicated below and Yakima County has caused this instrument to be executed in the name of said COUNTY by and through the Board of Yakima County Commissioners on the date indicated below.

Executed by the CONTRACTOR, ___________, 20__.

CONTRACTOR

________________________
Signature

________________________
Print or Type Name of Person Signing

________________________
Title

Foregoing Contract approved and ratified, ___________, 20__

SURETY

________________________
Approved as to form:

BOARD OF YAKIMA COUNTY COMMISSIONERS

________________________
J. Rand Elliott, Chairman

________________________
Michael D. Leita, Commissioner

________________________
Kevin J. Bouchey, Commissioner

ATTEST: Clerk of the Board

________________________
Christina S. Steiner

Informational Bid Documents
SP 3265
10
PERFORMANCE BOND
(RCW 39.08)

KNOW ALL MEN BY THESE PRESENTS, That__________________________, as “PRINCIPAL”, and
__________________________, a corporation authorized to do business in the State of
Washington, as “SURETY”, are jointly and severally held and bound unto Yakima County, Washington in the penal sum
__________________________ Dollars ($______________________), for the payment of
which by these presents we jointly and severally bind ourselves, our heirs, executors, administrators, assigns, and successors.

THE CONDITION of this bond is such that WHEREAS, on __________________, 20____, the PRINCIPAL
executed a certain Contract with the County, by the terms of which PRINCIPAL agrees to furnish all material and labor and will
undertake and complete the construction of SP 3265: CHEYNE LANDFILL SEPTAGE LAGOONS according to the maps,
plans and specifications made a part of said Contract, which Contract is attached hereto and by this reference is incorporated herein
and made a part hereof. FURTHER, the SURETY agrees to be bound by the laws of the State of Washington and subjected to the
jurisdiction of the State of Washington.

NOW, THEREFORE, if the PRINCIPAL shall faithfully perform all the provisions of such contract and pay all laborers,
mechanics, subcontractors and materialmen, and all persons who supply such persons or subcontractors with provisions or supplies for
the carrying on of such work, then this obligation to be void, otherwise to remain in full force and effect.

Dated this _______day of ____________________, 20____.

PRINCIPAL
By: ____________________________
Title: ____________________________

SURETY
By: ____________________________
   Attorney-in-Fact

APPROVED: YAKIMA COUNTY
By: ____________________________
   Chair of the Board of
   Yakima County Commissioners

Date: ____________________________, 20____

Approved as to form:

Deputy Prosecuting Attorney

Name of Local Office of Agent

Address of Local Office Agent

BOND NUMBER

YAKIMA COUNTY CONTRACT NUMBER
AMENDMENTS TO THE
STANDARD SPECIFICATIONS
INTRODUCTION

The following Amendments and Special Provisions shall be used in conjunction with the 2008 Standard Specifications for Road, Bridge, and Municipal Construction.

AMENDMENTS TO THE STANDARD SPECIFICATIONS

The following Amendments to the Standard Specifications are made a part of this contract and supersede any conflicting provisions of the Standard Specifications. For informational purposes, the date following each Amendment title indicates the implementation date of the Amendment or the latest date of revision.

Each Amendment contains all current revisions to the applicable section of the Standard Specifications and may include references which do not apply to this particular project.

DIVISION 1
GENERAL REQUIREMENTS

SECTION 1-03, AWARD AND EXECUTION OF CONTRACT
April 7, 2008

1-03.1 Consideration of Bids
This section is supplemented with the following new sub-section.

1-03.1(1) Tied Bids
After opening Bids, if two or more lowest responsive Bid totals are exactly equal, then the tie-breaker will be determined by drawing as described in this Section. Two or more slips of paper will be marked as follows: one marked “Winner” and the other(s) marked “unsuccessful”. The slips will be folded to make the marking unseen. The slips will be placed inside a box. One authorized representative of each Bidder shall draw a slip from the box. Bidders shall draw in alphabetic order by the name of the firm as registered with the Washington State Department of Licensing. The slips shall be unfolded and the firm with the slip marked “Winner” will be determined to be the successful Bidder and eligible for Award of the Contract. Only those Bidders that submitted a Bid total that is exactly equal to the lowest responsive Bid are eligible to draw.
SECTION 1-04, SCOPE OF THE WORK
April 7, 2008

1-04.4(1) Minor Changes
The first sentence in the first paragraph is revised to read:

Payments or credits for changes amounting to $15,000 or less may be made under the bid item "Minor Change."

1-04.5 Procedure and Protest by the Contractor
In the second paragraph, number 2, the reference to 7 calendar days is revised to 14 calendar days.

The second sentence in the fifth paragraph is revised to read:

The determination will be provided within 14-calendar days after receipt of the Contractor's supplemental written statement (including any additional information requested by the Project Engineer to support a continuing protest) described in item 2 above.

SECTION 1-05, CONTROL OF WORK
April 7, 2008

1-05.1 Authority of the Engineer
The fourth paragraph is revised to read:

At the Contractor's risk, the Project Engineer may suspend all or part of the Work according to Section 1-08.6.

1-05.12 Final Acceptance
The second paragraph is revised to read:

The Contractor agrees that neither completion nor final acceptance shall relieve the Contractor of the responsibility to indemnify, defend, and protect the Contracting Agency against any claim or loss resulting from the failure of the Contractor (or the subcontractors or lower tier subcontractors) to pay all laborers, mechanics, subcontractors, materialpersons, or any other person who provides labor, supplies, or provisions for carrying out the Work or for any payments required for unemployment compensation under Title 50 RCW or for industrial insurance and medical aid required under Title 51 RCW.

SECTION 1-07, LEGAL RELATIONS AND RESPONSIBILITIES TO THE PUBLIC
April 6, 2009

1-07.2(2) State Sales Tax: Work on State-Owned or Private Land
The following new paragraph is inserted in front of the first paragraph:
State Department of Revenue Rule 170 and its related rules apply for this section.

1-07.8 High Visibility Apparel
This section is revised to read:

The Contractor shall require all personnel under their control (including service providers, Subcontractors and lower tier Subcontractors) that are on foot in the work zone and are exposed to vehicle traffic or construction equipment to wear the high visibility apparel described in this Section.

The Contractor shall ensure that a competent person as identified in the MUTCD selects the appropriate high-visibility apparel suitable for the job-site conditions.

High visibility garments shall always be the outermost garments.

High visibility garments shall be in a condition compliant with the ANSI 107-2004 and shall be used in accordance with manufacturer recommendations.

This section is supplemented with the following new sub-sections.

1-07.8(1) Traffic Control Personnel
All personnel performing the Work described in Section 1-10 (including traffic control supervisors, flaggers, spotters, and others performing traffic control labor of any kind), shall comply with the following:

1. During daylight hours with clear visibility, workers shall wear a high-visibility ANSI/ISEA 107-2004 Class 2 or 3 vest or jacket, and hardhat meeting the high visibility headwear requirements of WAC 296-155-305; and

2. During hours of darkness (1/2-hour before sunset to 1/2-hour after sunrise) or other low visibility conditions (snow, fog, etc.), workers shall wear a high-visibility ANSI/ISEA 107-2004 Class 2 or 3 vest or jacket, high visibility lower garment meeting ANSI/ISEA 107-2004 Class E, and hardhats meeting the high visibility headwear requirements of WAC 296-155-305.

1-07.8(2) Non-Traffic Control Personnel
All personnel, except those performing the Work described in Section 1-10, shall wear high visibility apparel meeting the ANSI/ISEA 107-2004 Class 2 or 3 standard.

1-07.9(1) General
The following new paragraph is inserted to follow the sixth paragraph:

The Contractor shall ensure that any firm (Supplier, Manufacturer, or Fabricator) that falls under the provisions of RCW 39.12 because of the definition “Contractor” in WAC 296-127-010, complies with all the requirements of RCW 39.12.
1-07.15 Temporary Water Pollution/Erosion Control
This section is supplemented with the following:

Stormwater or dewatering water that has come in contact with concrete rubble, concrete pours, or cement treated soils shall be maintained to pH 8.5 or less before it is allowed to enter waters of the state. If pH exceeds 8.5, the Contractor shall immediately discontinue work and initiate treatment according to the plan to lower the pH. Work may resume, with treatment, once the pH of the stormwater is 8.5 or less or it can be demonstrated that the runoff will not reach surface waters.

High pH process water shall not be discharged to waters of the state. Unless specific measures are identified in the Special Provisions, high pH process water may be infiltrated, dispersed in vegetation or compost, or pumped to a sanitary sewer system. Water being infiltrated or dispersed shall have no chance of discharging directly to waters of the state, including wetlands or conveyances that indirectly lead to waters of the state. High pH process water shall be treated to within a range of 6.5 to 8.5 pH units prior to infiltration to ensure the discharge does not cause a violation of groundwater quality standards. If water is pumped to the sanitary sewer, the Contractor shall provide a copy of permits and requirements for placing the material into a sanitary sewer system prior to beginning the work. Process water may be collected and disposed of by the Contractor off the project site. The Contractor shall provide a copy of the permit for an approved waste site for the disposal of the process water prior to the start of work which generates the process water.

1-07.15(1) Spill Prevention, Control and Countermeasures Plan
This section is revised to read:

The Contractor shall prepare a project-specific spill prevention, control, and countermeasures plan (SPCC Plan) that will be used for the duration of the project. The Contractor shall submit the plan to the Project Engineer no later than the date of the preconstruction conference. No on-site construction activities may commence until WSDOT accepts an SPCC Plan for the project.

The term “hazardous materials”, as used in this Specification, is defined in Chapter 447 of the WSDOT Environmental Procedures Manual (M31-11). Occupational safety and health requirements that pertain to SPCC Plan implementation are contained in but not limited to WAC 296-824 and WAC 296-843.

Implementation Requirements
The SPCC Plan shall be updated by the Contractor throughout project construction so that the written plan reflects actual site conditions and practices. The Contractor shall update the SPCC Plan at least annually and maintain a copy of the updated SPCC Plan on the project site. All project employees shall be trained in spill prevention and containment, and shall know where the SPCC Plan and spill response kits are located and have immediate access to them.
If hazardous materials are encountered or spilled during construction, the Contractor shall do everything possible to control and contain the material until appropriate measures can be taken. The Contractor shall supply and maintain spill response kits of appropriate size within close proximity to hazardous materials and equipment.

The Contractor shall implement the spill prevention measures identified in the SPCC Plan before performing any of the following:

1. Placing materials or equipment in staging or storage areas.
2. Refueling, washing, or maintaining equipment.

**SPCC Plan Element Requirements**

The SPCC Plan shall set forth the following information in the following order:

1. **Responsible Personnel**
   Identify the name(s), title(s), and contact information for the personnel responsible for implementing and updating the plan, including all spill responders.

2. **Spill Reporting**
   List the names and telephone numbers of the federal, State, and local agencies the Contractor shall notify in the event of a spill.

3. **Project and Site Information**
   Describe the following items:
   
   A. The project Work.
   
   B. The site location and boundaries.
   
   C. The drainage pathways from the site.
   
   D. Nearby waterways and sensitive areas and their distances from the site.

4. **Potential Spill Sources**
   Describe each of the following for all potentially hazardous materials brought or generated on-site (including materials used for equipment operation, refueling, maintenance, or cleaning):
   
   A. Name of material and its intended use.
   
   B. Estimated maximum amount on-site at any one time.
   
   C. Location(s) (including any equipment used below the ordinary high water line) where the material will be staged, used, and stored and the distance(s) from nearby waterways and sensitive areas.
D. Decontamination location and procedure for equipment that comes into contact with the material.

E. Disposal procedures.

5. Pre-Existing Contamination
Describe any pre-existing contamination and contaminant sources (such as buried pipes or tanks) in the project area that are described in the Contract documents. Identify equipment and work practices that will be used to prevent the release of contamination.

6. Spill Prevention and Response Training
Describe how and when all personnel (including refueling contractors and Subcontractors) will be trained in spill prevention, containment and response in accordance with the Plan. Describe how and when all spill responders will be trained in accordance with WAC 296-824.

7. Spill Prevention
Describe the following items:

   A. Spill response kit contents and location(s).

   B. Security measures for potential spill sources.

   C. Secondary containment practices and structures for hazardous materials.

   D. Methods used to prevent stormwater from contacting hazardous materials.

   E. Site inspection procedures and frequency.

   F. Equipment and structure maintenance practices.

   G. Daily inspection and cleanup procedures that ensure all equipment used below the ordinary high water line is free of all external petroleum based products.

   H. Refueling procedures for equipment that cannot be moved from below the ordinary high water line.

8. Spill Response
Outline the response procedures the Contractor will follow for each scenario listed below. Include a description of the actions the Contractor shall take and the specific, on-site, spill response equipment that shall be used to assess the spill, secure the area, contain and eliminate the spill source, and clean up and dispose of spilled and contaminated material.
A spill of each type of hazardous material at each location identified in 4, above.

B. Stormwater that has come into contact with hazardous materials.

C. A release or spill of any pre-existing contamination and contaminant source described in 5, above.

D. A release or spill of any unknown pre-existing contamination and contaminant sources (such as buried pipes or tanks) encountered during project Work.

E. A spill occurring during Work with equipment used below the ordinary high water line.

If the Contractor will use a Subcontractor for spill response, provide contact information for the Subcontractor under item 1 (above), identify when the Subcontractor will be used, and describe actions the Contractor shall take while waiting for the Subcontractor to respond.

9. Project Site Map
Provide a map showing the following items:

A. Site location and boundaries.

B. Site access roads.

C. Drainage pathways from the site.

D. Nearby waterways and sensitive areas.

E. Hazardous materials, equipment, and decontamination areas identified in 4, above.

F. Pre-existing contamination or contaminant sources described in 5, above.

G. Spill prevention and response equipment described in 7 and 8, above.

10. Spill Report Forms
Provide a copy of the spill report form(s) that the Contractor will use in the event of a release or spill.

Payment
Payment will be made in accordance with Section 1-04.1 for the following Bid item when it is included in the Proposal:
“SPCC Plan”, lump sum.

When the written SPCC is accepted by WSDOT, the Contractor shall receive 50-percent of the lump sum Contract price for the plan.

The remaining 50-percent of the lump sum price will be paid after the materials and equipment called for in the plan are mobilized to the project.

The lump sum payment for “SPCC Plan” shall be full pay for:

1. All costs associated with creating the accepted SPCC Plan.

2. All costs associated with providing and maintaining the on-site spill prevention equipment described in the accepted SPCC Plan.

3. All costs associated with providing and maintaining the on-site standby spill response equipment and materials described in the accepted SPCC Plan.

4. All costs associated with implementing the spill prevention measures identified in the accepted SPCC Plan.

5. All costs associated with updating the SPCC Plan as required by this Specification.

As to other costs associated with releases or spills, the Contractor may request payment as provided for in the Contract. No payment shall be made if the release or spill was caused by or resulted from the Contractor’s operations, negligence, or omissions.

1-07.16(4) Archaeological and Historical Objects

This section is supplemented with the following new sub-section:

1-07.16(4)A Inadvertent Discovery of Human Skeletal Remains

If human skeletal remains are encountered by the Contractor, they shall not be further disturbed. The Contractor shall immediately notify the Engineer of any such finds, and shall cease all work adjacent to the discovery, in an area adequate to provide for the total security and protection of the integrity of the skeletal remains. The Engineer may require the Contractor to suspend Work in the vicinity of the discovery until final determinations and removal of the skeletal remains is completed.

If the Engineer finds that the suspension of Work in the vicinity of the discovery increases or decreases the cost or time required for performance of any part of the Work under this Contract, the Engineer will make an adjustment in payment or the time required for the performance of the Work in accordance with Sections 1-04.4 and 1-08.8.

1-07.17(2) Utility Construction, Removal or Relocation by Others

The first sentence in the second paragraph is revised to read:
If the Contract provides notice that utility work (including furnishing, adjusting, relocating, replacing, or constructing utilities) will be performed by others during the prosecution of the Work, the Special Provisions will establish the utility owners anticipated completion.

The first sentence in the third paragraph is revised to read:

When others delay the Work through late performance of utility work, the Contractor shall adhere to the requirements of Section 1-04.5.

1-07.23 Public Convenience and Safety

This section is revised to read:

The Contractor shall be responsible for providing adequate safeguards, safety devices, protective equipment, and any other needed actions to protect the life, health, and safety of the public, and to protect property in connection with the performance of the Work covered by the Contract. The Contractor shall perform any measures or actions the Engineer may deem necessary to protect the public and property. The responsibility and expense to provide this protection shall be the Contractor's except that which is to be furnished by the Contracting Agency as specified in other sections of these Specifications. Nothing contained in this Contract is intended to create any third-party beneficiary rights in favor of the public or any individual utilizing the Highway facilities being constructed or improved under this Contract.

1-07.23(1) Construction Under Traffic

The second sentence in the second paragraph is revised to read:

The Contractor shall maintain existing roads, streets, sidewalks, and paths within the project limits, keeping them open, and in good, clean, safe condition at all times.

The fifth sentence in the second paragraph is revised to read:

The Contractor shall also maintain roads, streets, sidewalks, and paths adjacent to the project limits when affected by the Contractor’s operations.

The final paragraph in this section is deleted.

1-07.23(2) Construction and Maintenance of Detours

Number 1. under the first paragraph is revised to read:

Detours and detour bridges that will accommodate traffic diverted from the Roadway, bridge, sidewalk or path during construction,
SECTION 1-08, PROSECUTION AND PROGRESS
August 4, 2008

1-08.1 Subcontracting
Item (2) in the first sentence of the seventh paragraph is revised to read:

(2) Delivery of these materials to the Work site in vehicles owned or operated by such
plants or by recognized independent or commercial hauling companies hired by those
commercial plants.

1-08.3(2)A Type A Progress Schedule
This section is revised to read:

The Contractor shall submit five copies of a Type A Progress Schedule no later than 10 days
after the date the contract is executed, or some other mutually agreed upon submittal time.
The schedule may be a critical path method (CPM) schedule, bar chart, or other standard
schedule format. Regardless of which format used, the schedule shall identify the critical
path. The Engineer will evaluate the Type A Progress Schedule and approve or return the
schedule for corrections within 15 calendar days of receiving the submittal.

1-08.5 Time for Completion
The third sentence in the first paragraph is revised to read:

A nonworking day is defined as a Saturday, a Sunday, a whole or half day on which the
Contract specifically prohibits Work on the critical path of the Contractor’s approved
progress schedule, or one of these holidays: January 1, the third Monday of January, the
third Monday of February, Memorial Day, July 4, Labor Day, November 11, Thanksgiving
Day, the day after Thanksgiving, and Christmas Day.

1-08.6 Suspension of Work
The first paragraph is revised to read:

The Engineer may order suspension of all or any part of the Work if:

1. Unsuitable weather that prevents satisfactory and timely performance of the Work;
or

2. The Contractor does not comply with the Contract: or

3. It is in the public interest.

1-08.7 Maintenance During Suspension
The first sentence in the fourth paragraph is revised to read:

If the Engineer determines that the Contractor has pursued the Work diligently before the
suspension, then the Contracting Agency will maintain the temporary Roadway (and bear its
cost).
The fifth paragraph is revised to read:

The Contractor shall protect and maintain all other Work in areas not used by traffic. All costs associated with protecting and maintaining such Work shall be the responsibility of the Contractor, except those costs associated with implementing the TESC Plan according to Section 8-01.

The seventh paragraph is revised to read:

After any suspension, the Contractor shall resume all responsibilities the Contract assigns for the Work.

SECTION 1-09, MEASUREMENT AND PAYMENT
April 7, 2008

1-09.9 Payments
The first paragraph is supplemented with the following:

For items Bid as lump sum, the Contractor shall submit a breakdown of their lump sum price in sufficient detail for the Project Engineer to determine the value of the Work performed on a monthly basis. Lump sum breakdowns shall be provided to the Project Engineer no later than the date of the preconstruction meeting.

The second sentence in the third paragraph is revised to read:

Unless otherwise provided in the payment clause of the applicable Specifications, partial payment for lump sum Bid items will be a percentage of the price in the Proposal based on the Project Engineer’s determination of the amount of Work performed, with consideration given to but not exclusively based on the Contractors lump sum breakdown.

The third paragraph is supplemented with the following:

The determination of payments under the contract will be final in accordance with Section 1-05.1.

1-09.9(1) Retainage
In the fourth paragraph, number 1, the reference to $20,000 is revised to read $35,000.

SECTION 1-10, TEMPORARY TRAFFIC CONTROL
April 6, 2009

1-10.1(2) Description
The following new paragraph is inserted after the second paragraph:
Unless otherwise permitted by the Contract or approved by the Project Engineer, the Contractor shall keep all existing pedestrian routes and access points (including sidewalks, paths and crosswalks) open and clear at all times.

The second and third sentences in the third paragraph are revised to read:

The Contractor shall erect and maintain all construction signs, warning signs, detour signs, and other traffic control devices necessary to warn and protect the public at all times from injury or damage as a result of the Contractor’s operations which may occur on or adjacent to Highways, roads, streets, sidewalks or paths. No Work shall be done on or adjacent to any Traveled Way until all necessary signs and traffic control devices are in place.

1-10.2(1) General

The second sentence in the third paragraph is revised to read:

Possession of a current TCS card and flagging card by the primary and alternate TCS is mandatory.

1-10.2(2) Traffic Control Plans

The first sentence in the first paragraph is revised to read:

The traffic control plan or plans appearing in the Contract documents show a method of handling vehicle, bicycle and pedestrian traffic.

In the third sentence of the second paragraph, the reference to "MUTCD, Part VI" is revised to "MUTCD, Part 6".

1-10.3(2)B Rolling Slowdown

The first two paragraphs are deleted and replaced with the following:

Rolling slowdown traffic control operations are not to be used for routine work that can be addressed by standard lane or shoulder closure traffic control. When a short-term roadway closure is needed for an infrequent, non-repetitive work operation such as a sign bridge removal, or utility wire crossing, the Contractor may implement a rolling slowdown on a multi-lane roadway, as part of an approved traffic control plan.

The Contractor shall submit for approval a traffic control plan detailing the expected delay time, interchange ramp control and rolling slowdown distance. A portable changeable message sign shall be placed ahead of the starting point of the traffic control to warn traffic of the slowdown. The sign shall be placed far enough ahead of the Work to avoid any expected backup of vehicles.

A rolling slowdown shall use traffic control vehicles with flashing amber lights. At least one traffic control vehicle will be used for every two lanes to be slowed, plus a control vehicle will serve as a following (chase) vehicle for traffic ahead of the blockade. The traffic control vehicles shall enter the roadway and form a moving blockade to reduce traffic
speeds and create a clear area ahead of the blockade in which to accomplish the work without a total stoppage of traffic.

1-10.3(3)A Construction Signs
The fifth paragraph is revised to read:

Where it is necessary to add weight to signs for stability, sand bags or other similar ballast may be used but the height shall not be more than 4-inches above the roadway surface, and shall not interfere with the breakaway features of the device. The Contractor shall follow the manufacturer’s recommendations for sign ballasting.

1-10.3(3)D Barricades
The second paragraph is revised to read:

Where it is necessary to add weight to barricades for stability, sand bags or other similar ballast may be used but the height shall not be more than 4-inches above the roadway surface and shall not interfere with the breakaway features of the device. The Contractor shall follow the manufacturer’s recommendation for sign ballasting.

1-10.3(3)G Traffic Cones
This section including title is revised to read:

1-10.3(3)G Traffic Cones and Tall Channelizing Devices
Where shown on an approved traffic control plan or where ordered by the Engineer, the Contractor shall provide, install and maintain traffic cones or tall channelizing devices. Cones and tall channelizing devices shall be kept in good repair and shall be removed immediately when directed by the Engineer. Where wind or moving traffic frequently displaces cones, an effective method of stabilizing them, such as stacking two together at each location, shall be employed.

1-10.3(3)K Portable Temporary Traffic Control Signal
The first paragraph is revised to read:

Where shown on an approved traffic control plan, the Contractor shall provide, operate, maintain and remove a portable temporary traffic control signal system to provide alternating one-lane traffic operations on a two-way facility. A portable temporary traffic control signal system shall be defined as two traffic control units that operate together. The system shall be trailer mounted, fully self-contained and designed so that it can be easily transported and deployed at different locations.

The third sentence in the second paragraph is deleted.

The following is inserted in front of the sixth paragraph:
The Traffic Control Supervisor shall monitor and insure that the Portable Temporary Traffic Control Signal is fully operational and maintained as specified by the manufacturer. This Work may include cleaning and replacing lamps and other routine maintenance as needed.

1-10.4(2) Item Bids with Lump Sum for Incidentals

The unit of measurement statement for "Portable Temporary Traffic Control Signal" is revised to read:

No specific unit of measurement will apply to the lump sum item of "Portable Temporary Traffic Control Signal".

1-10.5(1) Lump Sum Bid for Project (No Unit Items)

This section is revised to read:

"Project Temporary Traffic Control", lump sum.

The lump sum Contract payment shall be full compensation for all costs incurred by the Contractor in performing the Contract Work defined in Section 1-10, except for costs compensated by Bid Proposal items inserted through Contract Provisions as described in Section 1-10.4(3).

1-10.5(2) Item Bids with Lump Sum for Incidentals

The unit of measure for the bid item "Portable Temporary Traffic Control Signal," is revised to lump sum.

The paragraph following "Portable Temporary Traffic Control Signal," is revised to read:

The lump sum Contract price shall be full compensation for all costs of labor, materials and equipment incurred by the Contractor in performing the Contract Work as described in Section 1-10.3(3)K, including all costs for traffic control during manual control, adjustment, malfunction, or failure of the portable traffic control signals and during replacement of failed or malfunctioning signals.

DIVISION 2
EARTHWORK

SECTION 2-01, CLEARING, GRUBBING, AND ROADSIDE CLEANUP
April 7, 2008

2-01.3(1) Clearing

Item 3 is deleted.

The first sentence in Item 4. is revised to read:
Follow these requirements for all stumps that will be buried deeper than 5-feet from the top, side, or end surface of the embankment or any structure:

2-01.3(2) Grubbing
Item 2. e, is revised to read:

Upon which embankments will be placed except stumps may be close-cut or trimmed as allowed in Section 2-01.3(1) item 4.

SECTION 2-02, REMOVAL OF STRUCTURES AND OBSTRUCTIONS
April 7, 2008

2-02.3(3) Removal of Pavement, Sidewalks, Curbs, and Gutters
The first sentence in 3. is supplemented with the following:

For removal of bituminous pavement, asphalt planing equipment may be used in lieu of sawcutting provided that a clean vertical edge remains.

SECTION 2-03, ROADWAY EXCAVATION AND EMBANKMENT
January 7, 2008

2-03.1 Description
The first sentence in the first paragraph is revised to read:

The Work described in this section, regardless of the nature or type of the materials encountered, includes excavating and grading the Roadway, excavating in borrow pits, excavating below grade, excavating channels and ditches, removing slide material, and disposing of all excavated material.

2-03.3(3) Excavation Below Grade
The section title is revised to read:

2-03.3(3) Excavation Below Subgrade
The first sentence in the fifth paragraph is revised to read:

Compaction. If the density of the natural earth under any area of the Roadway is less than that required in Section 2-03.3(14)C, Method B, the Engineer may order the Contractor to perform any or all of the following:

2-03.3(14)M Excavation of Channels
This section including title is revised to read:
2-03.3(14)M Excavation of Channels and Ditches
Channel Excavation: Open excavations 8-feet or more wide at the bottom, but excludes channels that are part of the Roadway.

Ditch Excavation: Open excavations less than 8-feet wide at the bottom, but excludes ditches that are part of the Roadway.

Before excavating channels or ditches, the Contractor shall clear and grub the area in accordance with Section 2-01.

2-03.4 Measurement
The first sentence in the first paragraph is revised to read:

Roadway excavation, channel excavation, ditch excavation, unsuitable foundation excavation, and common borrow items will be measured by the cubic yard.

The fourth sentence in the first paragraph is revised to read:

For Roadway excavation, channel excavation and ditch excavation items, the original ground will be compared with the planned finished section shown in the Plans.

2-03.5 Payment
The first paragraph is supplemented with the following:

“Channel Excavation”, per cubic yard.
“Channel Excavation Incl. Haul”, per cubic yard.
“Ditch Excavation”, per cubic yard.
"Ditch Excavation Incl. Haul", per cubic yard.

The first sentence in the second paragraph is revised to read:


The second paragraph is supplemented with the following:

When a bid item is not included in the proposal for channel excavation or ditch excavation all costs shall be included in roadway excavation.

The third paragraph is revised to read:
When the Engineer orders Work according to Section 2-03.3(3), unit Contract prices shall apply, unless the Work differs materially from the excavation above Subgrade, then payment will be in accordance with Section 1-04.4.

SECTION 2-10, DITCH EXCAVATION
January 7, 2008

This section is deleted in its entirety. The section title is revised to read:

DIVISION 5
SURFACE TREATMENTS AND PAVEMENTS

SECTION 5-04, HOT MIX ASPHALT
December 1, 2008

5-04.3(9) Spreading and Finishing
The nominal compacted depth for HMA Class 3/4" and HMA Class 1/2" listed under the first paragraph is revised to read:

HMA Class 3/4" and HMA Class 1/2"
  wearing course 0.30-feet
  other courses 0.35-feet

5-04.3(12)B Longitudinal Joints
The first two paragraphs are revised to read:

The longitudinal joint in any 1 course shall be offset from the course immediately below by not more than 6-inches nor less than 2-inches. All longitudinal joints constructed in the wearing course shall be located at a lane line or an edge line of the Traveled Way.

On one-lane ramps a longitudinal joint may be constructed at the center of the traffic lane, subject to approval by the Project Engineer, if:

1. The ramp must remain open to traffic, or
2. The ramp is closed to traffic and a hot-lap joint is constructed.

   a. If a hot-lap joint is allowed at the center of the traffic lane, 2 paving machines shall be used; a minimum compacted density in accordance with Section 5-04.3(10)B shall be achieved throughout the traffic lane; and construction equipment other than rollers shall not operate on any uncompacted mix.
The reference to Standard Plan A-1 in the third paragraph is revised to read "Standard Plan A40.10-00."

5-04.3(16) Weather Limitations
The chart for Surface Temperature Limitation is revised to read:

<table>
<thead>
<tr>
<th>Compacted Thickness (Feet)</th>
<th>Wearing Course</th>
<th>Other Courses</th>
</tr>
</thead>
<tbody>
<tr>
<td>Less than 0.10</td>
<td>55°F</td>
<td>45°F</td>
</tr>
<tr>
<td>0.10 to 0.20</td>
<td>45°F</td>
<td>35°F</td>
</tr>
<tr>
<td>More than 0.20</td>
<td>35°F</td>
<td>35°F</td>
</tr>
</tbody>
</table>

5-04.3(21) Asphalt Binder Revision
This section is revised to read:

When the Contracting Agency provides a source of aggregate, the expected percentage content of asphalt binder in the resulting mix will be identified in the Contract documents.

Should the percentage of asphalt binder shown in the job mix formula for Hot Mix Asphalt produced with Agency-provided aggregate vary by more than plus or minus 0.3-percent from the amount shown in the Contract documents, an adjustment in payment will be made. The adjustment in payment (plus or minus) will be based on the invoice unit cost, including shipping cost, without any markups. The quantity subject to an adjustment shall be the difference between the JMF asphalt binder percentage and the contract document asphalt binder percentage except that the first 0.3% of this difference shall not apply. No adjustment will be made when the Contractor elects not to use a Contracting Agency-provided source, or when no source is made available by the Contracting Agency.

SECTION 6-02, CONCRETE STRUCTURES
April 6, 2009

6-02.2 Materials
This section is supplemented with the following:

Pigmented Sealer Materials for Coating of Concrete Surfaces 9-08.2(1)

6-02.3(2)A Contractor Mix Design
The third sentence in the fourth paragraph is revised to read:

The nominal maximum size aggregate for Class 4000P shall be 3/8-inch.

The fourth sentence in the fourth paragraph is revised to read:
The nominal maximum size aggregate for Class 4000D shall be 1-inch.

6-02.3(2)B Commercial Concrete
The second paragraph is revised to read:

Where concrete Class 3000 is specified for items such as, culvert headwalls, plugging culverts, concrete pipe collars, pipe anchors, monument cases, light standard foundations, pedestals, cabinet bases, guardrail anchors, sign post foundations, fence post footings, sidewalks, curbs, and gutters, the Contractor may use commercial concrete. If commercial concrete is used for sidewalks, curbs, and gutters, it shall have a minimum cementitious material content of 564-pounds per cubic yard of concrete, shall be air entrained, and the tolerances of Section 6-02.3(5)C shall apply. Commercial concrete shall not be used for items such as, bridges, retaining walls, box culverts, or foundations for high mast luminaires, mast arm traffic signals, cantilever signs, and sign bridges. The Engineer may approve the use of commercial concrete for other applications not listed above.

6-02.3(6)D Protection Against Vibration
The second sentence in the second paragraph is revised to read:

These requirements for the protection of freshly placed concrete against vibration shall not apply for plant cast concrete, nor shall they apply to the vibrations caused by the traveling public.

The third sentence in the second paragraph is deleted.

Item (2) under the third paragraph is revised to read:

(2) Equipment Class L (Low Vibration) shall include tracked dozers under 85,000-pounds, track vehicles, trucks (unless excluded above), hand operated jack hammers, cranes, auger drill rig, caisson drilling, vibratory roller compactors under 30,000-pounds, and grab-hammers.

Item (3) under the third paragraph is revised to read:

(3) Equipment Class H (High Vibration) shall include pile drivers, vibratory hammers, machine operated impact tools, pavement breakers, and other large pieces of equipment.

6-02.3(10) Roadway Slabs and Bridge Approach Slabs
This section’s content is deleted. This section's title is revised to read:

6-02.3(10) Bridge Decks and Bridge Approach Slabs

This section is supplemented with the following new sub-sections:
6-02.3(10)A Preconstruction Meeting
A pre-concreting conference shall be held 5 to 10-working days before placing concrete to
discuss construction procedures, personnel, and equipment to be used. Those attending shall
include:

1. (representing the Contractor) The superintendent and all foremen in charge of
placing the concrete, finishing it; and

2. (representing the State) The Project Engineer, key inspection assistants, and the
State Construction Office.

If the project includes more than 1 deck or slab, and if the Contractor’s key personnel
change between concreting operations, or at request of the Engineer, an additional
conference shall be held just before each deck or slab is placed.

The Contractor shall not place bridge decks until the Engineer agrees that:

1. Concrete producing and placement rates will be high enough to meet placing and
finishing deadlines;

2. Finishers with enough experience have been employed;

3. Adequate finishing tools and equipment are at the site, and

4. Curing procedures consistent with the Specification requirements are employed.

6-02.3(10)B Screed Rail Supports
The Contractor shall place screed rails outside the finishing area. When screed rails cannot
be placed outside the finishing area as determined by the Engineer, they shall rest on
adjustable supports that can be removed with the least possible disturbance to the screeded
concrete. The supports shall rest on structural members or on forms rigid enough to resist
deflection. Supports shall be removable to at least 2-inches below the finished surface. For
staged constructed bridge decks, the finishing machine screed rails shall not be supported on
the completed portion of deck and shall deflect with the portion of structure under
construction.

Screed rails (with their supports) shall be strong enough and stiff enough to permit the
finishing machine to operate effectively on them. All screed rails shall be placed and secured
for the full length of the deck/slab before the concreting begins. If the Engineer approves in
advance, the Contractor may move rails ahead onto previously set supports while concreting
progresses. But such movable rails and their supports shall not change the set elevation of
the screed.

On steel truss and girder spans, screed rails and bulkheads may be placed directly on
transverse steel floorbeams, with the strike-board moving at right angles to the centerline of
the Roadway.
6-02.3(10)C Finishing Equipment
The finishing machine shall be self-propelled and be capable of forward and reverse
movement under positive control. The finishing machine shall be equipped with a rotating
cylindrical single or double drum screed not exceeding 60-inches in length. The finishing
machine shall have the necessary adjustments to produce the required cross-section, line,
and grade. Provisions shall be made for the raising and lowering of all screeds under
positive control. The upper vertical limit of screed travel shall permit the screed to clear the
finished concrete surface.

For bridge deck widening of 20-feet or less, and for bridge approach slabs, or where jobsite
conditions do not allow the use of conventional configuration finishing machines described
above, the Contractor may propose the use of a hand operated motorized power screed such
as a “Texas” or “Bunyan” screed. This screed shall be capable of finishing the bridge deck
and bridge approach slab to the same standards as the finishing machine. The Contractor
shall not begin placing bridge deck or bridge approach slab concrete until receiving the
Engineer’s approval of this screed and the placing procedures.

On bridge decks the Contractor may use hand-operated strike-boards only when the
Engineer approves for special conditions where self propelled or motorized hand operated
screeds cannot be employed. These boards shall be sturdy and able to strike off the full
placement width without intermediate supports. Strike-boards, screed rails, and any
specially made auxiliary equipment shall receive the Engineer’s approval before use. All
finishing requirements in these Specifications apply to hand-operated finishing equipment.

6-02.3(10)D Concrete Placement, Finishing, and Texturing
Before any concrete is placed, the finishing machine shall be operated over the entire length
of the deck/slab to check screed deflection. Concrete placement may begin only if the
Engineer approves after this test.

Immediately before placing concrete, the Contractor shall check (and adjust if necessary) all
falsework and wedges to minimize settlement and deflection from the added mass of the
concrete deck/slab. The Contractor shall also install devices, such as telltales, by which the
Engineer can readily measure settlement and deflection.

The Contractor shall schedule the concrete placement so that it can be completely finished
during daylight. After dark finishing is permitted if the Engineer approves and if the
Contractor provides adequate lighting.

The placement operation shall cover the full width of the Roadway or the full width between
construction joints. The Contractor shall locate any construction joint over a beam or web
that can support the deck/slab on either side of the joint. The joint shall not occur over a pier
unless the Plans permit. Each joint shall be formed vertically and in true alignment. The
Contractor shall not release falsework or wedges supporting pours on either side of a joint
until each side has aged as these Specifications require.
Placement of concrete for bridge decks and bridge approach slabs shall comply with Section 6-02.3(6). The Engineer shall approve the placement method. In placing the concrete, the Contractor shall:

1. Place it (without segregation) against concrete placed earlier, as near as possible to its final position, approximately to grade, and in shallow, closely spaced piles;

2. Consolidate it around reinforcing steel by using vibrators before strike-off by the finishing machine;

3. Not use vibrators to move concrete;

4. Not revibrate any concrete surface areas where workers have stopped prior to screeding;

5. Remove any concrete splashed onto reinforcing steel in adjacent segments before concreting them;

6. Tamp and strike off the concrete with a template or strike board moving slowly forward at an even speed;

7. Maintain a slight excess of concrete in front of the cutting edge across the entire width of the placement operation;

8. Make enough passes with the strike-board (without overfinishing and bringing excessive amounts of mortar to the surface) to create a surface that is true and ready for final finish; and

9. Leave a thin, even film of mortar on the concrete surface after the last pass of the strike-board.

Workers shall complete all post screeding operations without walking on the concrete. This may require work bridges spanning the full width of the slab.

After removing the screed supports, the Contractor shall fill the voids with concrete (not mortar).

If necessary, as determined by the Engineer, the Contractor shall float the surface left by the finishing machine to remove roughness, minor irregularities, and seal the surface of the concrete. Floating shall leave a smooth and even surface. Float finishing shall be kept to a minimum number of passes so air bubbles in the concrete are not released. The floats shall be at least 4-feet long. Each transverse pass of the float shall overlap the previous pass by at least half the length of the float. The first floating shall be at right angles to the strike-off. The second floating shall be at right angles to the centerline of the span. A smooth riding surface shall be maintained across construction joints.
Expansion joints shall be finished with a 1/2-inch radius edger.

After floating, but while the concrete remains plastic, the Contractor shall test the entire deck/slab for flatness (allowing for crown, camber, and vertical curvature). The testing shall be done with a 10-foot straightedge held on the surface. The straightedge shall be advanced in successive positions parallel to the centerline, moving not more than 1/2 the length of the straightedge each time it advances. This procedure shall be repeated with the straightedge held perpendicular to the centerline. An acceptable surface shall be one free from deviations of more than 1/8-inch under the 10-foot straightedge.

If the test reveals depressions, the Contractor shall fill them with freshly mixed concrete, strike off, consolidate, and refinish them. High areas shall be cut down and refinished. Retesting and refinishig shall continue until an acceptable, deviation free surface is produced. The hardened concrete shall meet all smoothness requirements of these Specifications even though the tests require corrective Work.

The Contractor shall texture the bridge deck and bridge approach slab by combing the final surface perpendicular to the centerline. Made of a single row of metal tines, the comb shall leave striations in the fresh concrete approximately 3/16-inch deep by 1/8-inch wide and spaced approximately 1/2-inch apart. The Engineer will decide actual depths at the site. (If the comb has not been approved, the Contractor shall obtain the Engineer’s approval by demonstrating it on a test section.)

The Contractor may operate the combs manually or mechanically, either singly or with several placed end to end. The timing and method used shall produce the required texture without displacing larger particles of aggregate. Texturing shall end 2-feet from curb lines. This 2-foot untextured strip shall be hand finished with a steel trowel.

If the Plans call for an overlay (to be constructed under the same Contract), such as hot mix asphalt, latex modified concrete, epoxy concrete, or similar, the Contractor shall produce the final finish by dragging a strip of damp, seamless burlap lengthwise over the full width of the deck/slab or by brooming it lightly. A burlap drag shall equal the deck/slab in width. Approximately 3-feet of the drag shall contact the surface, with the least possible bow in its leading edge. It shall be kept wet and free of hardened lumps of concrete. When it fails to produce the required finish, the Contractor shall replace it. When not in use, it shall be lifted clear of the slab.

After the deck/slab has cured, the surface shall not vary more than 1/8-inch under a 10-foot straightedge placed parallel and perpendicular to the centerline.

The Contractor shall cut high spots down with a diamond faced, saw-type cutting machine. This machine shall cut through mortar and aggregate without breaking or dislodging the aggregate or causing spalls.
Low spots shall be built up utilizing a grout or concrete with a strength equal to or greater than the required 28-day strength of the deck/slab. The method of build-up shall be submitted to the Engineer for approval.

The surface texture on any area cut down or built up shall match closely that of the surrounding bridge deck or bridge approach slab area. The entire bridge deck and bridge approach slab shall provide a smooth riding surface.

6-02.3(10)E Sidewalk
Concrete for sidewalk shall be well compacted, struck off with a strike-board, and floated with a wooden float to achieve a surface that does not vary more than 1/8-inch under a 10-foot straightedge. An edging tool shall be used to finish all sidewalk edges and expansion joints. The final surface shall have a granular texture that will not turn slick when wet.

6-02.3(10)F Bridge Approach Slab Orientation and Anchors
Bridge approach slabs shall be constructed full bridge deck width from outside usable Shoulder to outside usable Shoulder at an elevation to match the Structure. The bridge approach slabs shall be modified as shown in the Plans to accommodate the grate inlets at the bridge ends if the grate inlets are required.

Bridge approach slab anchors shall be installed as detailed in the Plans and the anchor rods, couplers, and nuts shall conform to Section 9-06.5(1). The steel plates shall conform to ASTM A 36. All metal parts shall receive 1 coat of formula A-11-99 paint meeting the requirements of Section 9-08.2. The pipe shall be any non-perforated PE or PVC pipe of the diameter specified in the Plans. Polystyrene shall conform to Section 9-04.6. The anchors shall be installed parallel both to profile grade and center line of Roadway. The Contractor shall secure the anchors to ensure that they will not be misaligned during concrete placement. For Method B anchors installations, the epoxy bonding agent used to install the anchors shall be Type IV conforming to Section 9-26.1. The compression seal shall be as noted in the Contract documents. Dowel bars shall be installed in the bridge approach slabs in accordance with the requirements of the Standard Plans and Section 5-05.3(10).

After curing bridge approach slabs in accordance with Section 6-02.3(11), the bridge approach slabs may be opened to traffic when a minimum compressive strength of 2,500 psi is achieved.

6-02.3(12) Construction Joints
The third sentence in the second paragraph is deleted.

6-02.3(14) Finishing Concrete Surfaces
The following new sub-section is inserted after Section 6-02.3(14)B:

6-02.3(14)C Pigmented Sealer for Concrete Surfaces
All surfaces specified in the Plans to receive pigmented sealer shall receive a Class 2 surface finish, (except that concrete barrier surfaces shall be finished in accordance with Section 6-02.3(11)A) and shall receive a light brush sandblasting in order that complete neutralization
of the surface and subsequent penetration of the pigmented sealer is achieved. All curing
agents and form release agents shall be removed. The surface shall be dry, clean and
prepared in accordance with the manufacturer's written instructions. The Contractor shall
submit four copies of the manufacturer's written instructions.

The Contractor shall not apply pigmented sealer from a batch greater than twelve months
past the initial date of color sample approval of that batch by the Engineer.
The pigmented sealer color or colors for specific concrete surfaces shall be as specified in
the Special Provisions.

The pigmented sealer shall be spray applied in accordance with the manufacturer's written
instructions for application, air temperature required for sealer application and curing,
qualification of applicator, rate of application, and number of coats to apply. Pigmented
sealer shall not be applied until the concrete has cured for at least 28 days. Pigmented sealer
shall not be applied upon damp surfaces, nor shall it be applied when the air is misty, or
otherwise unsatisfactory for the work, in the opinion of the manufacturer or the Engineer.
The final appearance shall have an even and uniform color acceptable to the Engineer.

For concrete surfaces such as columns, retaining walls, pier walls, abutments, concrete
fascia panels, and noise barrier wall panels, the pigmented sealer shall extend to one foot
below the finish ground line, unless otherwise shown in the Plans.

6-02.3(17)N Removal of Falsework and Forms
The fifth paragraph, beginning with “The Contractor may remove side forms, traffic barrier
form, and pedestrian barrier forms” etc, is deleted.

6-02.3(17)O Early Concrete Test Cylinder Breaks
The third paragraph is revised to read:

The cylinders shall be cured in accordance with WSDOT FOP for AASHTO T 23.

6-02.3(20) Grout for Anchor Bolts and Bridge Bearings
This section's title is revised to read:

6-02.3(20) Grout for Anchor Bolts and Bridge Bearings

6-02.3(25) Prestressed Concrete Girders
In the fourth paragraph, the second sentence in Prestressed Concrete Wide Flange I Girder is
revised to read:

WSDOT standard girders in this category include Series WF42G, WF50G, WF58G, WF66G,
WF74G, WF83G, WF95G and WF100G.

In the fourth paragraph, the seventh sentence in Spliced Prestressed Concrete Girder is revised to
read:
WSDOT standard girders in this category include Series WF66PTG, WF74PTG, WF83PTG, WF95PTG and WF100PTG.

6-02.3(25)B Casting
The reference to Section 9-23.7 in the second sentence of the third paragraph is deleted.

6-02.3(25)C Prestressing
The fifth paragraph is revised to read:

From manufacture to encasement in concrete, prestressing strand shall be protected against dirt, oil, grease, damage, and all corrosives. Strand shall be stored in a dry covered area and shall be kept in the manufacturer’s original packaging until placement in the forms. If prestressing strand has been damaged or pitted, it will be rejected. Prestressing strand with rust shall be spot cleaned with a non-metallic pad to inspect for any sign of pitting or section loss.

6-02.3(25)J Horizontal Alignment
The first paragraph is revised to read:

The Contractor shall check and record the horizontal alignment of the top and bottom flanges of each girder at the following times:

1. Initial - upon removal of the girder from the casting bed;
2. Final - within 2-weeks, but not less than 3-days prior to shipment; and
3. Storage - between 115 to 125-days after casting, if the girder remains in storage for a period exceeding 120-days.

Each check shall be made by measuring the distance between each flange and a chord that extends the full length of the girder. The Contractor shall perform and record each check at a time when the alignment of the girder is not influenced by temporary differences in surface temperature. Records for the Initial check shall be included in the Contractor’s Prestressed Concrete Certificate of Compliance. Records for the Final and Storage checks shall be provided to the Engineer for approval.

The first sentence in the fifth paragraph is deleted.

6-02.3(25)K Girder Deflection
The first paragraph is revised to read:

The Contractor shall check and record the vertical deflection (camber) of each girder at the following times:

1. Initial - upon removal of the girder from the casting bed; and
2. Storage - within 2-weeks, but not less than 3-days prior to shipment, if the girder remains in storage for a period exceeding 120-days.

The Contractor shall perform and record each check at a time when the alignment of the girder is not influenced by temporary differences in surface temperature. These records shall be available for the Engineer's inspection, and in the case of girders older than 120-days, shall be transmitted to the Engineer as soon as practical for evaluation of the effect of long-term storage on the “D” dimension. Records for the Initial check shall be included in the Contractor's Prestressed Concrete Certificate of Compliance. Records for the Storage check shall be provided to the Engineer for approval.

6-02.3(25)L Handling and Storage
The fifth sentence in the third paragraph is deleted.

6-02.3(25)N Prestressed Concrete Girder Erection
The fourth paragraph is revised to read:

When prestressed girders arrive on the project, the Project Engineer will confirm that they are stamped “Approved for Shipment”, that the final horizontal alignment and deflection (camber) check records have been approved, and that they have not been damaged in shipment, before accepting them.

6-02.3(26)E Ducts
The first six paragraphs under the heading Ducts for Internal Embedded Installation are revised to read:

Ducts, including their splices, shall be semi-rigid, air and mortar tight, corrugated plastic ducts of virgin polyethylene or polypropylene materials, free of water soluble chlorides or other chemicals reactive with concrete or post-tensioning reinforcement. Ducts, including their splices, shall either have a white coating on the outside or shall be of a white material with ultraviolet stabilizers added. Ducts, including their splices, shall be capable of withstanding concrete pressures without deforming or permitting the intrusion of cement paste during placement of concrete. All fasteners shall be appropriate for use with plastic ducts, and all clamps shall be of an approved plastic material.

Polyethylene ducts shall conform to ASTM D 3350 with a cell classification of 345464A. Polypropylene ducts shall conform to ASTM D 4101 with a cell classification of either PP0340B14541 or PP0340B67884. Resins used for duct fabrication shall have a minimum oxidation induction time of 20 minutes, in accordance with ASTM D 3895, based on tests performed by the duct fabricator on samples taken from the lot of finished product. The duct thickness shall be as specified in Section 10.8.3 of the AASHTO LRFD Bridge Construction Specifications, latest edition and current interims.

Each duct shall maintain the required profile within a placement tolerance of plus or minus \(\frac{1}{4}\) -inch for longitudinal tendons and plus or minus \(\frac{1}{8}\)-inch for transverse slab tendons during all phases of the work. The minimum acceptable radius of curvature shall be as
recommended by the duct manufacturer and as supported by documented industry standard
testing. The ducts shall be completely sealed to keep out all mortar.

Each duct shall be located to place the tendon at the center of gravity alignment shown in
the Plans. To keep friction losses to a minimum, the Contractor shall install ducts to the
exact lines and grades shown in the Plans. Once in place, the ducts shall be tied firmly in
position before they are covered with concrete. During concrete placement, the Contractor
shall not displace or damage the ducts.

The ends of the ducts shall:

1. Permit free movement of anchorage devices, and
2. Remain covered after installation in the forms to keep out all water or debris.

Immediately after any concrete placement, the Contractor shall force blasts of oil-free,
compressed air through the ducts to break up and remove any mortar inside before it
hardens. Before deck concrete is placed, the Contractor shall satisfy the Engineer that ducts
are unobstructed and contain nothing that could interfere with tendon installation,
tensioning, or grouting. If the tendons are in place, the Contractor shall show that they are
free in the duct.

Ducts shall be capped and sealed at all times until the completion of grouting to prevent the
intrusion of water.

The last paragraph under the heading **Ducts for Internal Embedded Installation** is revised to
read:

When the duct must be curved in a tight radius, more flexible duct may be used, subject to
the Engineer’s approval.

The first paragraph under the heading **Ducts for External Exposed Installation** is revised to
read:

Duct shall be high-density polyethylene (HDPE) conforming to ASTM D 3350. The cell
classification for each property listed in Table 1 shall be as follows:

This section is supplemented with the following:

**Vents, Grout Injection Ports, Drains and Caps**
The Contractor shall install vents at high points and drains at low points of the tendon
profile (and at other places if the Plans require). Vents at high points shall consist of a set of
three vents - one to be installed at the high point of the duct, and flanking vents to be
installed on either side of the high point vent at locations where the duct profile is 8 to 12
inches below the elevation of the high point vent. Vents shall include grout injection ports.
Vents and drains shall have a minimum inside diameter of 3/4 inches, and shall be of either stainless steel, nylon, or polyolefin materials, free of water soluble chlorides or other chemicals reactive with concrete or post-tensioning reinforcement. Stainless steel vents and drains shall conform to ASTM A 240 Type 316. Nylon vents and drains shall conform to cell classification S-PA0141 (weather resistant). Polyolefin vents and drains shall contain an antioxidant with a minimum oxidation induction time of 20 minutes in accordance with ASTM D 3895. Polyolefin vents and drains shall also have a stress crack resistance of three hours minimum when tested at an applied stress of 350 psi in accordance with ASTM F 2136.

All fasteners shall be appropriate for use with plastic ducts, and all clamps shall be of an approved plastic material. Taping of connections is not allowed. Valves shall be positive mechanical shut-off valves. Valves, and associated caps, shall have a minimum pressure rating of 100 psi.

Vents shall point upward and remain closed until grouting begins. Drains shall point downward and remain open until grouting begins. Ends of stainless steel vents and drains shall be removed 1-inch inside the concrete surface after grouting has been completed. Ends of nylon or polyolefin vents and drains may be left flush to the surface unless otherwise specified by the Engineer. Vents, except for grout injection, are not required for transverse post-tensioning ducts in the roadway slab unless specified in the Plans.

Caps shall be made of either stainless steel or fiber reinforced polymer (FRP). Stainless steel caps shall conform to ASTM A 240 Type 316L. The resin for FRP caps shall be either nylon, polyester, or acrylonitrile butadiene styrene (ABS). Nylon shall conform to cell classification S-PA0141 (weather resistant). Caps shall be sealed with "O" ring seals or precision fitted flat gaskets placed against the bearing plate. Caps shall be fastened to the anchorage with stainless steel bolts conforming to ASTM A 240 Type 316L.

**Leak Tightness Testing**

The Contractor shall test each completed duct assembly for leak tightness, prior to casting concrete and placing post-tensioning reinforcement. The Contractor shall submit the equipment used to conduct the leak tightness testing and to monitor and record the pressure maintained in and lost from the closed assembly, and the process to be followed in conducting the leak tightness testing, to the Engineer for approval along with the post-tensioning system shop drawings in accordance with Section 6-02.3(26)A.

Prior to testing, all vents, grout injection ports, and drains shall either be capped or have their shut-off valves closed. The Contractor shall pressurize the completed duct assembly to an initial air pressure of 50 psi. This pressure shall be held for five minutes to allow for internal adjustments within the assembly. After five minutes, the air supply valve shall be closed. The Contractor shall monitor and measure the pressure maintained within the closed assembly, and any subsequent loss of pressure, over a period of one minute following the closure of the air supply valve. Locations of leakage shall be identified, repaired or reconstructed, and the repaired reassembled duct system retested. The cycle of testing, repair and retesting of each completed duct assembly shall continue until the completed duct...
assembly completes a test with pressure loss within the specified amount. The maximum pressure loss for duct assemblies equal to or less than 150 feet in length shall be 25 psig. The maximum pressure loss for duct assemblies greater than 150 feet in length shall be 15 psig.

6-02.3(26)F Prestressing Reinforcement

The fourth paragraph is revised to read:

From manufacture to encasement in concrete or grout, prestressing strand shall be protected against dirt, oil, grease, damage, and all corrosives. Strand shall be stored in a dry covered area and shall be kept in the manufacturer’s original packaging. If prestressing strand has been damaged or pitted, it will be rejected. Prestressing strand with rust shall be spot cleaned with a non-metallic pad to inspect for any sign of pitting or section loss. If the prestressing reinforcement will not be stressed and grouted for more than seven calendar days after it is placed in the ducts, the Contractor shall place an approved corrosion inhibitor conforming to Federal Specification MIL-P-3420F-87 in the ducts.

6-02.3(26)H Grouting

The following is inserted in front of the first paragraph of this section:

Grout for post-tensioning reinforcement shall be a Class C pre-packaged, pumpable, non-segregating, non-shrink, high-strength grout conforming to the requirements specified in Section 10.9.3 of the AASHTO LRFD Bridge Construction Specifications, latest edition and current interims. Pre-packaged components of the grout mix shall be used within six months or less from date of manufacture to date of usage. Grout for post-tensioning reinforcement will be accepted based on manufacturer's certificate of compliance in accordance with Section I-06.3, except that the water-cementitious material ratio of 0.45 maximum shall be field verified.

All grout produced for any single structure shall be furnished by one supplier.

All grouting operations shall be conducted by ASBI certified grout technicians.

The Contractor shall submit a grouting operation plan to the Engineer for approval in accordance with Section 6-01.9. The grouting operation plan shall include, but not be limited to, the following:

1. Names of the grout technicians, accompanied by documentation of their ASBI certification.

2. Type, quantity and brand of materials used in the grouting operations, including all manufacturer's certificates of compliance.

3. Type of equipment to be used, including meters and measuring devices used to positively measure the quantity of materials used to mix the post-tensioning grout,
the equipment capacity in relation to demand and working conditions, and all back-
up equipment and spare parts.

4. General grouting procedure.

5. Duct leak tightness testing and repair procedures as specified in Section 6-
02.3(26E).

6. Methods used to control the rate of grout flow within the ducts.

7. Theoretical grout volume calculations, and target flow rates recommended by the
gROUT manufacturer as a function of the mixer equipment and the expected range of
ambient temperatures.

8. Grout mixing and pumping procedures.

9. Direction of grouting.

10. Sequence of use of the grout injection ports, vents and drains.

11. Procedures for handling blockages.


The Contractor shall not begin grouting operations until receiving the Engineer's approval of
the grouting operation plan.

Post-tensioning grout shall be mixed in accordance with the pre-packaged grout
manufacturer's recommendations using high-shear colloidal mixers. Mechanical paddle
mixers will not be allowed. The grout produced for filling post-tensioning ducts shall be
free of lumps and undispersed cement. All equipment used to mix each batch of post-
tensioning grout shall be equipped with appropriate meters and measuring devices to
positively measure all quantities of all materials used to produce the mixed grout. The field
test for water-cementitious materials ratio shall be performed prior to beginning the grout
injection process. Grouting shall not begin until the material properties of each batch of
gROUT have been confirmed as acceptable.

The fourth paragraph is deleted.

The fifth paragraph is deleted.

The sixth paragraph is deleted

6-02.5 Payment
The paragraph following bid item "Commercial Concrete" is supplemented with the following:
All costs in connection with furnishing and applying pigmented sealer to concrete surfaces as specified shall be included in the unit contract price per cubic yard for "Conc. Class ____". If the concrete is to be paid for other than by class of concrete then the costs shall be included in the applicable adjacent item of work.

SECTION 6-03, STEEL STRUCTURES
April 6, 2009

6-03.3(33) Bolted Connections
The second paragraph is revised to read:

All bolted connections are slip critical. Painted structures require either Type 1 or Type 3 bolts. Unpainted structures require Type 3 bolts. AASHTO M 253 bolts shall not be galvanized or be used in contact with galvanized metal.

6-03.3(33)A Pre-Erection Testing
The first sentence in the first paragraph is revised to read:

High strength bolt assemblies (bolt, nut, and washer), black and galvanized, shall be subjected to a field rotational capacity test, as outlined below, prior to any erection activity.

6-03.3(38) Placing Superstructure
This section is revised to read:

The concrete in piers and crossbeams shall reach at least 80-percent of design strength before girders are placed on them.

6-03.4 Measurement
The second paragraph is revised to read:

Cast or forged metal (kind) shown in the Plans will be measured by the pound or will be paid for on a lump sum basis, whichever is shown on the Proposal.

SECTION 6-07, PAINTING
April 6, 2009

6-07.3(1) Painting New Steel Structures
The third paragraph is revised to read:

The primer coat, the second coat and the third coat shall all be selected from the same manufacturer and shall be from one of the approved paint systems listed in the Qualified Products List. Once a paint system has been selected, that system shall be used throughout the Structure.

The ninth paragraph is deleted.
6-07.3(2)G Painting Steel Surfaces
The first sentence in the first paragraph is revised to read:

The coating system for all steel surfaces shall incorporate 3 single component moisture-cured polyurethane coats from the same manufacturer and shall be from one of the approved paint systems listed in the Qualified Products List.

6-07.3(4) Painting Galvanized Surfaces
This section is revised and renumbered as follows:

6-07.3(4)A Painting of Galvanized Surfaces
All galvanized surfaces receiving paint shall be prepared for painting in accordance with the ASTM D 6386. The method of preparation shall be as agreed upon by the paint manufacturer and the galvanizer. The Contractor shall not begin painting until receiving the Engineer’s approval of the prepared galvanized surface.

Environmental Conditions
Steel surfaces shall be:

- Greater than 35°F and
- Less than 115°F

or in accordance with the manufacturer’s recommendations, whichever is more stringent.

The Contractor shall paint the dry surface as follows:

<table>
<thead>
<tr>
<th>Paint Formulas</th>
<th>Type</th>
</tr>
</thead>
<tbody>
<tr>
<td>First Coat</td>
<td>MIL-P-24441 Epoxy polyamide</td>
</tr>
<tr>
<td>Second Coat</td>
<td>C-11-99 Moisture Cured Aliphatic Polyurethane</td>
</tr>
</tbody>
</table>

Each coat shall be dry before the next coat is applied. All coats applied in the shop shall be dried hard before shipment.

The following new section is inserted before Section 6-07.3(4)A:

6-07.3(4) Painting or Powder Coating of Galvanized Surfaces
Galvanized surfaces specified to be coated after galvanizing shall receive either paint in accordance with Section 6-07.3(4)A or powder coating in accordance with Section 6-07.3(4)B. The color of the finish coat shall be as specified in the Special Provisions.

The following new sub-section is inserted after Section 6-07.3(4)A:

6-07.3(4)B Powder Coating of Galvanized Surfaces
Powder coating of galvanized surfaces shall conform to the following requirements:
Submittals
The Contractor shall submit the following information to the Engineer for approval:

1. The name, location, and contact information (mail address, phone, and e-mail) for the firm performing the powder coating operation.

2. Quality control (QC) programs established and followed by the firm performing the powder coating operation. Forms to document inspection and testing of coatings as part of the QC program shall be included in the submittal.

3. Project specific powder coating plan, including identification of the powder coating materials used (and manufacturer), and specific cleaning, surface preparation, pre-heating, powder coating application, curing, shop and field coating repair, handling, and storage processes to be taken for the assemblies being coated for this project.

4. Product data and MSDS sheets for all powder coating and coating repair materials.

Galvanizing
Prior to the galvanizing operation, the Contractor shall identify to the galvanizer the specific assemblies and surfaces receiving the powder coating after galvanizing, to ensure that the galvanizing method used on these assemblies is compatible with subsequent application of a powder coating system. Specifically, such assemblies shall neither be water-quenched, nor receive a chromate conversion coating, as part of the galvanizing operation.

Galvanized Surface Cleaning and Preparation
Galvanized surfaces receiving the powder coating shall be cleaned and prepared for coating in accordance with ASTM D 6386, and the project specific powder coating plan as approved by the Engineer.

Assemblies conforming to the ASTM D 6386 definition for newly galvanized steel shall receive surface smoothing and surface cleaning in accordance with ASTM D 6386 Section 5, and surface preparation in accordance with ASTM D 6386 Section 5.4.1.

Assemblies conforming to the ASTM D 6386 definition for partially weathered galvanized steel shall be checked and prepared in accordance with ASTM D 6386 Section 6, before then receiving surface smoothing and surface cleaning in accordance with ASTM D 6386 Section 5, and surface preparation in accordance with ASTM D 6386 Section 5.4.1.

Assemblies conforming to the ASTM D 6386 definition for weathered galvanized steel shall be prepared in accordance with ASTM D 6386 Section 7, before then receiving
surface smoothing and surface cleaning in accordance with ASTM D 6386 Section 5
and surface preparation in accordance with ASTM D 6386 Section 5.4.1.

The Contractor shall notify the Engineer of all surface cleaning and preparation
activities, and shall provide the Engineer opportunity to perform quality assurance
inspection, in accordance with Section 1-05.6, at the completion of surface cleaning and
preparation activities prior to beginning powder coating application.

**Powder Coating Application and Curing**

After surface preparation, the two component powder coating shall be applied in
accordance with the powder coating manufacturer’s recommendations, the project
specific powder coating plan as approved by the Engineer, and as follows:

1. Pre-heat. The pre-heat shall be sufficient to prevent pin holes from forming in
the finished coating system.

2. Apply the epoxy primer coat, followed by a partial cure.

3. Apply the polyester finish coat, followed by the finish cure.

**Testing**

The firm performing the powder coating operation shall conduct, or make arrangements
for, QC testing on all assemblies receiving powder coating for this project, in
accordance with the powder coating firm’s QC program as documented in item 2 of the
Submittal subsection above. Testing may be performed on coated surfaces of
production fabricated items, or on a representative test panel coated alongside the
production fabricated items being coated. There shall be a minimum of one set of tests
representing each cycle of production fabricated items coated and cured. Additional
tests shall be performed at the request of the Engineer. Repair of damaged coatings on
production fabricated items shall be the responsibility of the firm applying the powder
coating, and shall be in accordance with the project specific powder coating plan as
approved by the Engineer. At a minimum, the QC testing shall test for the following
requirements:

1. Visual inspection for the presence of coating holidays, and other unacceptable
surface imperfections.

2. Coating thickness measurement in accordance with Section 6-07.3(5). The
minimum thickness of the epoxy primer coating and polyester finish coating
shall be 3 mils each.

3. Hardness testing in accordance with ASTM D 3363, with the finish coat
providing a minimum hardness value of H.

4. Adhesion testing in accordance with ASTM D 4541 for 400 psi minimum
adhesion for the complete two component coating system.
5. Powder Coating Institute (PCI) #8 recommended procedure for solvent cure test.

The results of the QC testing shall be documented in a QC report, and submitted to the Engineer for approval.

The Engineer shall be provided notice and access to all assemblies at the powder coating facility for the purposes of Contracting Agency acceptance inspection, including notice and access to witness all hardness and adhesion testing performed by the firm conducting the QC testing, in accordance with Section 1-05.6.

Assemblies not meeting the above requirements will be subject to rejection by the Engineer. Rejected assemblies shall be repaired or re-coated by the Contractor, at no additional expense to the Contracting Agency, in accordance with the project specific powder coating plan as approved by the Engineer until the assemblies satisfy the acceptance testing requirements.

Assemblies shall not be shipped from the powder coating firm’s facility to the project site until the Contractor receives the Engineer’s approval of the QC Report and assembly inspection performed by the Engineer.

Coating Protection For Shipping, Storage, and Field Erection

After curing and acceptance, the Contractor shall protect the coated assemblies with multiple layers of bubble wrap, or other protective wrapping materials specified in the project specific powder coating plan as approved by the Engineer.

During storage and shipping, each assembly shall be separated from other assemblies by expanded polystyrene spacers and other spacing materials specified in the project specific powder coating plan as approved by the Engineer.

After erection, all coating damage due to the Contractor’s shipping, storage, handling, and erection operations shall be repaired by the Contractor, at no additional expense to the Contracting Agency, in accordance with the project specific powder coating plan as approved by the Engineer. The Contractor shall provide the Engineer access to all locations of all powder coated members for verification of coating conditions prior to and following all coating repairs.

6-07.3(5) Paint - Film Thickness

The second sentence in the first paragraph is deleted and replaced with the following:

The dry film thickness of the primer coat on faying surfaces shall not be less than 2.5 mils nor greater than the paint manufacturer’s maximum recommended thickness. The primer coat shall meet the coefficient of friction requirements of Section 6-07.3(1) and 6-07.3(2)E Item 2. Top flange surfaces to be embedded in concrete shall receive a mist coat. Welded
shear connectors, if installed in the shop, shall not receive paint except for incidental overspray.

DIVISION 7
DRAINAGE STRUCTURES, STORM SEWERS,
SANITARY SEWERS, WATER MAINS, AND CONDUITS

SECTION 7-02, CULVERTS
December 1, 2008

7-02.2 Materials
The third paragraph is revised to read:

Thermoplastic culvert pipe includes solid wall PVC culvert pipe, profile wall PVC culvert pipe, and corrugated polyethylene culvert pipe. Solid wall PVC culvert pipe, profile wall PVC culvert pipe, and corrugated polyethylene culvert pipe are acceptable alternates for Schedule A or B culvert pipe.

In the chart for Culvert Pipe Schedules, for Schedule B, 15' – 25', the references in the column for Thermoplastic PE or PVC for "PVC" are revised to "PE or PVC".

DIVISION 8
MISCELLANEOUS CONSTRUCTION

SECTION 8-01, EROSION CONTROL AND WATER POLLUTION CONTROL
April 6, 2009

8-01.3(1) General
The first sentence in the eighth paragraph is revised to read:

Erodible earth not being worked, whether at final grade or not, shall be covered within the following time period, using an approved soil covering practice:

The ninth paragraph is revised to read:

If the Engineer, under Section 1-08.6, orders the Work suspended, the Contractor shall continue to control erosion, pollution, and runoff during the shutdown.

8-01.3(1)C Water Management
Item 2. "Process Water" is supplemented with the following new first paragraph:

High pH process water or wastewater (non-stormwater) that is generated on-site, including water generated during concrete grinding, rubblizing, washout, and hydrodemolition activities, shall not be discharged to waters of the state. Water may be infiltrated upon the
approval of the Engineer. Off-site disposal of concrete process water shall be in accordance with Standard Specification 5-01.3(11).

8-01.3(2)D Mulching
The second paragraph is supplemented with the following:

Wood strand mulch shall be applied by hand or by straw blower.

8-01.3(2)E Tackling Agent and Soil Binders
The second sentence in the fourth paragraph is revised to read:

Pam may be reapplied on actively worked areas within a 48-hour period.

8-01.3(6)D Wattle Check Dam
The reference to Section 8-01.3(10) is revised to Section 9-14.5(5).

8-01.3(12) Compost Sock
The last paragraph is deleted.

8-01.3(13) Temporary Curb
The first paragraph is revised to read:

Temporary curbs may consist of asphalt, concrete, sand bags, compost socks, wattles, or geotextile/plastic encased berms of sand or gravel, or as approved by the Engineer.

SECTION 8-15, RIPRAP
April 7, 2008

8-15.3(1) Excavation for Riprap
The second sentence of the first paragraph is revised to read:

Excavation below the level of the intersection of the slope to be protected and the adjacent original ground or the channel floor or slope shall be classified, measured, and paid for as channel excavation or ditch excavation in accordance with Section 2-03.

8-15.4 Measurement
The following new paragraph is inserted to follow the fifth paragraph.

Channel excavation will be measured by the cubic yard as specified in Section 2-03.

The sixth paragraph is revised to read:

Ditch excavation will be measured by the cubic yard as specified in Section 2-03.

The reference to Section 2-10 in the seventh paragraph is revised to Section 2-03.
8-15.5 Payment
The bid item “Filter Blanket” is supplemented with the following:

The unit price for “Filter Blanket” shall be full payment for all costs incurred to perform the work in Section 8-15.3(7).

This section is supplemented with the following:

“Channel Excavation”, per cubic yard.
“Channel Excavation Incl. Haul”, per cubic yard.
Payment for “Channel Excavation”, “Channel Excavation Incl. Haul”, “Ditch Excavation” and “Ditch Excavation Incl. Haul” is described in Section 2-03.5.

DIVISION 9
MATERIALS

SECTION 9-03, AGGREGATES
April 6, 2009

9-03.1(1) General Requirements
The reference to ASTM C-1260 in the third, fifth, and sixth paragraphs is deleted.

The following new paragraph is inserted after the sixth paragraph:

The use of fly ash that does not meet the requirements of Table 2 of AASHTO M295 may be approved for use for aggregates with expansions greater than or equal to 0.21 percent. The Contractor shall submit test results according to ASTM C 1567 through the Project Engineer to the State Materials Laboratory that demonstrate that the proposed fly ash when used with the proposed aggregates and portland cement will control the potential expansion to 0.20 percent or less before the fly ash and aggregate sources may be used in concrete. The Contracting Agency may test the proposed ASR mitigation measure to verify its effectiveness. In the event of a dispute, the Contracting Agency’s results will prevail.

9-03.8(7) HMA Tolerances and Adjustments
The third sentence in the second paragraph under (1.), (Beginning with: The tolerance limits on sieves...) is deleted.

9-03.17 Foundation Material Class A and Class B
This section is revised to read:

Foundation material Class A and Class B shall conform to the following gradations:
<table>
<thead>
<tr>
<th>Sieve Size</th>
<th>Class A</th>
<th>Class B</th>
</tr>
</thead>
<tbody>
<tr>
<td>2½&quot; square</td>
<td>98-100</td>
<td>95-100</td>
</tr>
<tr>
<td>2&quot; square</td>
<td>92-100</td>
<td>75-100</td>
</tr>
<tr>
<td>1½&quot; square</td>
<td>72-87</td>
<td>30-60</td>
</tr>
<tr>
<td>¾&quot; square</td>
<td>27-47</td>
<td>0-5</td>
</tr>
<tr>
<td>¼&quot; square</td>
<td>3-14</td>
<td>---</td>
</tr>
<tr>
<td>U.S. No. 4</td>
<td>0-5</td>
<td>---</td>
</tr>
</tbody>
</table>

All percentages are by mass.

SECTION 9-04, JOINT AND CRACK SEALING MATERIALS
December 1, 2008

9-04.1(2) Premolded Joint Filler for Expansion Joints
This section is revised to read:
Premolded joint filler for use in expansion (through) joints shall conform to either AASHTO M 213 Specifications for “Preformed Expansion Joint Fillers for Concrete Paving and Structural Construction” except the requirement for water absorption is deleted, or ASTM D 7174 Specifications for “Preformed Closed-Cell Polyolefin Expansion Joint Fillers for Concrete Paving and Structural Construction.”

9-04.2(1) Hot Poured Joint Sealants
This section is revised to read:
Hot poured joint sealants shall meet the requirements of AASHTO M 324 Type IV except that the Cone Penetration at 25°C shall be 130 max. Hot poured joint sealants shall be sampled in accordance with ASTM D 5167 and tested in accordance with ASTM D 5329. The Hot poured joint sealant shall have a minimum Cleveland Open Cup Flash Point of 205°C in accordance with AASHTO T 48

SECTION 9-06, STRUCTURAL STEEL AND RELATED MATERIALS
April 6, 2009

9-06.5(3) High Strength Bolts
Paragraphs one through four are revised to read as follows:
High-strength bolts for structural steel joints shall conform to either AASHTO M 164 Type 1 or 3, or AASHTO M 253 Type 1 or 3, as specified in the Plans or Special Provisions.
Galvanized AASHTO M 164 Type 1 bolts with an ultimate tensile strength above 145 ksi shall be tested for embrittlement. Embrittlement testing shall be conducted after galvanization in accordance with ASTM F 606, Section 7. The Manufacturer's Certificate of Compliance for the lot provided shall show the ultimate tensile strength test results.

Bolts conforming to AASHTO M 253 shall not be galvanized. AASHTO M 253 Type 1 bolts shall be painted with two coats of zinc rich paint, formula A-9-73, consisting of a minimum dry film thickness of 2 mils per coat, when specified in the Plans or Special Provisions.

Bolts for unpainted and nongalvanized structures shall conform to either AASHTO M 164 Type 3 or AASHTO M 253 Type 3, as specified in the Plans or Special Provisions.

Nuts for high strength bolts shall meet the following requirements:

AASHTO M 164 Bolts
- Black Type 1
- Black weathering Type 3
- Galvanized Type 1

AASHTO M 253 Bolts
- Black Type 1
- Black weathering Type 3

9-06.13 Copper Seals
This section including title is revised to read:

9-06.13 Vacant

9-06.16 Roadside Sign Structures
This section is revised to read:

All bolts, nuts, washers, cap screws, and coupling bolts shall conform to AASHTO M 164 and Section 9-06.5(3). All connecting hardware shall be galvanized after fabrication in accordance with AASHTO M 232.

Posts for single post sign structures shall meet the requirements of ASTM A 500 Grade B or ASTM A 53 Grade B, Type E or S.

Posts for perforated square steel posts shall meet the requirements of ASTM A 653 Grade 50. Perforated square steel posts shall be finished in accordance ASTM A 653 G90 Structural Quality Grade 50 or ASTM A 653 G140.
Slip bases (SB1, SB2, and SB3) for perforated square steel posts shall conform to the following:

- Plates: ASTM A 572
- Casting (SB3): ASTM A 536 Grade 65-45-12 and ASTM A 153
- Tubing: ASTM A 500 Grade B
- Angle Iron (SB1): ASTM A 36

Except as noted otherwise, the slip bases (SB1, SB2, and SB3) for perforated square steel posts shall be hot dipped galvanized.

The heavy duty anchor used for perforated square steel posts (ST-4) shall meet the requirements of ASTM A 500 Grade B and shall be hot dipped galvanized.

Wide flange steel or solid square steel posts for multiple post sign structures shall conform to either ASTM A 36 or ASTM A 992. Posts conforming to either ASTM A 588 or ASTM A 572 Grade 50 may be used as an acceptable alternate to the ASTM A 36 and ASTM A 992 posts. All steel not otherwise specified shall conform to either ASTM A 36 or ASTM A 992.

Except as noted otherwise all steel, including posts, base plates, and base stiffeners, shall be galvanized after fabrication in accordance with AASHTO M111.

Base connectors for multiple directional steel breakaway posts shall conform to the following:

- Brackets: Aluminum Alloy 6061 T-6
- Bosses for Type TPB Brackets: ASTM A 582
- Anchor Ferrules: Type 304 stainless steel for threaded portion. AISI 1045 steel rod and AISI 1008 coil for cage portion.

Anchor couplings for multiple directional steel breakaway posts shall conform to AMS 6378D with a tensile breaking strength range as follows:

- Type TPA: 17,000 to 21,000 lb.
- Type TPB: 47,000 to 57,000 lb.

For multi-directional breakaway base connectors, shims shall conform to ASTM A 653, SS Grade 33, Coating Designation G 165.

SECTION 9-07, REINFORCING STEEL
April 6, 2009

9-07.3 Epoxy Coated Steel Reinforcing Bars
The reference to ASTM A 06 in number 1. of the first paragraph is revised to ASTM A 706.
9-07.10 Prestressing Reinforcement Strand
The first sentence in the fourth paragraph is revised to read:

For every 5 reels furnished, one sample, not less than 5½-feet long, shall be sent to the Engineer for testing.

9-07.11 Prestressing Reinforcement Bar
The fifth and sixth paragraphs are revised to read:

The Contractor shall supply a Manufacturer’s Certificate of Compliance in accordance with Section 1-06.3 for each bar. The Contractor shall supply a Manufacturer’s Certificate of Compliance in accordance with Section 1-06.3 for all nuts and couplers confirming compliance with the specified strength requirement.

For each heat of steel for high-strength steel bar, the Contractor shall submit two samples, each not less than 5½ -feet long, to the Engineer for testing.

SECTION 9-08, PAINTS
April 6, 2009

9-08.2 Paint Formulas – General
The following new sub-sections are inserted after this section.

9-08.2(1) Pigmented Sealer Materials for Coating of Concrete Surfaces
The pigmented sealer shall be a semi-opaque colored toner containing only methyl methacrylate-ethyl acrylate copolymer resins, toning pigments suspended in solution at all times by a chemical suspension agent, and solvent. Toning pigments shall be laminar silicates, titanium dioxide and inorganic oxides only. There shall be no settling or color variation. Use of vegetable or marine oils, paraffin materials, stearates or organic pigments in any part of coating formulation will not be permitted. The Contractor shall submit a one-quart wet sample, a draw down color sample and spectrophotometer or colorimeter readings, taken in accordance with ASTM D 2244, for each batch. The calculated Delta E shall not exceed 1.0 deviation from the Commission Internationale de l’Eclairage color measurement analysis method (CIELAB) for each pigmented sealer color.

For the respective color, pigmented sealer shall conform to the following CIELAB analysis:

<table>
<thead>
<tr>
<th>Color</th>
<th>III/Obs</th>
<th>L*</th>
<th>a*</th>
<th>b*</th>
</tr>
</thead>
<tbody>
<tr>
<td>Washington Gray</td>
<td>D65/10, degrees</td>
<td>62.59</td>
<td>0.98</td>
<td>5.23</td>
</tr>
<tr>
<td></td>
<td>A/10, degrees</td>
<td>63.06</td>
<td>1.80</td>
<td>5.70</td>
</tr>
<tr>
<td></td>
<td>CWF/10, degrees</td>
<td>63.02</td>
<td>0.73</td>
<td>6.08</td>
</tr>
<tr>
<td>Cascade Green</td>
<td>D65/10, degrees</td>
<td>36.62</td>
<td>-0.53</td>
<td>-0.89</td>
</tr>
<tr>
<td></td>
<td>A/10, degrees</td>
<td>35.82</td>
<td>-7.15</td>
<td>-2.53</td>
</tr>
<tr>
<td></td>
<td>CWF/10, degrees</td>
<td>36.34</td>
<td>-5.09</td>
<td>-1.18</td>
</tr>
</tbody>
</table>
Mt. Baker Gray  
D65/10, degrees  45.94  1.38  4.46
A/10, degrees  46.40  1.70  5.05
CWF/10, degrees  46.46  1.07  5.48

Mt. St. Helens Gray  
D65/10, degrees  56.07  2.15  6.68
A/10, degrees  56.76  3.08  7.52
CWF/10, degrees  56.67  1.64  7.85

The one-quart wet sample shall be submitted in the manufacturer's labeled container with product number, batch number and size of batch. The companion draw down color sample shall be labeled with the product number, batch number and size of batch. The Contractor shall submit the specified samples and readings to the Engineer at least 14 calendar days prior to the scheduled application of the sealer. The Contractor shall not begin applying pigmented sealer until receiving the Engineer's approval of the pigmented sealer color samples.

9-08.2(2) Powder Coating Materials for Coating Galvanized Surfaces

The powder coating system shall consist of two components, an epoxy primer coat and a polyester finish coat. The epoxy primer coat and the polyester finish coat materials shall be from the same manufacturer.

The epoxy primer coat shall be an epoxy powder primer conforming to the following requirements:

<table>
<thead>
<tr>
<th>Property</th>
<th>Specification</th>
<th>Performance Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adhesion</td>
<td>ASTM D 3359 Method B</td>
<td>5B (no failure)</td>
</tr>
<tr>
<td>Flexibility</td>
<td>ASTM D 522 Method B</td>
<td>Pass 1/8&quot; mandrel bend</td>
</tr>
<tr>
<td>Pencil Hardness</td>
<td>ASTM D 3363</td>
<td>H Plus</td>
</tr>
<tr>
<td>Specific Gravity</td>
<td>ASTM D 792</td>
<td>1.25 minimum</td>
</tr>
</tbody>
</table>

The polyester finish coat shall conform to American Architectural Manufacturers Association (AAMA) Specification 2604.

Degassing additives may be added as necessary to prevent pin holes in the finish coat. The degassing additives shall be added in accordance with manufacturer's recommendations.

The color of the powder coating system polyester finish coat shall be as specified in the Plans or Special Provisions.

Repair materials shall be selected from one of the approved products listed in the current Qualified Products List and specified in the Contractor's powder coating plan as approved by the Engineer.
SECTION 9-09, TIMBER AND LUMBER
January 7, 2008

9-09.1 General Requirements
This section is revised to read:

All timber and lumber shall be sized as indicated in the Plans.

All timber and lumber to be painted shall be surfaced on all sides. All timber and lumber to be painted shall be thoroughly air or kiln dried to an equilibrium moisture content and shall be stored in such a manner as to remain in a thoroughly dry condition until placed into the work.

9-09.2 Grade Requirements
This section is revised to read:

Timber and lumber shall conform to the grades and usage listed below.

Timber and lumber shall be marked with a certified lumber grade stamp provided by one of the following agencies:

West Coast Lumber Inspection Bureau (WCLIB)
Western Wood Products Association (WWPA)
Pacific Lumber Inspection Bureau (PLIB)
Any lumber grading bureau certified by the American Lumber Standards Committee

For structures, all material delivered to the project shall bear a grade stamp and have a grading certificate. The grade stamp and grading certificate will not constitute final acceptance of the material. The Engineer may reject any or all of the timber or lumber that does not comply with the specifications or has been damaged during shipping or upon delivery. The grading certificate shall be issued by either the grading bureau whose stamp is shown on the material, or by the lumber mill, which shall be under the supervision of one of the grading bureaus listed above. The certificate shall include the following:

Name of the mill performing the grading
The grading rules being used
Name of the person doing the grading with current certification
Signature of a responsible mill official
Date the lumber was graded at the mill.
Grade, dimensions, and quantity of the timber or lumber

For Guardrail Posts and Blocks, Sign Posts, Mileposts, Sawed Fence Posts, and Mailbox Posts, the material delivered to the project shall either bear a grade stamp on each piece or have a grading certificate as defined above. The grade stamp or grading certificate shall not constitute final acceptance of the material. The Engineer may reject any or all of the timber or lumber that does not comply with the specifications or has been damaged during shipping or upon delivery.
9-09.2(1) Surfacing and Seasoning

This section including title is revised to read:

9-09.2(1) Structures

All timber and lumber for structures shall be Douglas Fir-Larch unless specified otherwise in the contract, and shall conform to the following:

| Materials 2” to 4” nominal thick, 5” nominal and wider (Structural Joists and Planks) | No. 1 and better, grade (Section 123-b of WCLIB) or (Section 62.11 of WWPA) |
| Materials 5” nominal and thicker (Beams and Stringers) | No. 1 and better, grade (Section 130-b of WCLIB) or (Section 70.11 of WWPA) |

Timber lagging for soldier pile walls shall be Douglas Fir-Larch, grade No. 2 or better or Hem-Fir No. 1.

When the material is delivered to the project, the Engineer will check the order for the appropriate grade stamp. The invoice and grading certificate accompanying the order must be accurate and complete with the information listed above. The grading certificate and grade markings shall not constitute final acceptance of the material. The Engineer may reject any or all of the timber or lumber that does not comply with the specifications or has been damaged during shipping or upon delivery.

9-09.2(2) Vacant

This section including title is revised to read:

9-09.2(2) Guardrail Posts and Blocks

Timber and lumber for guardrail posts and blocks (classified as Posts and Timbers) shall conform to the species and grades listed below.

| Douglas Fir | No. 1 and better, grade (Section 131-b WCLIB) or (Section 80.11 WWPA) |
| Hem Fir | Select Structural, grade (Section 131-a WCLIB) or (Section 80.10 WWPA) |
| Southern Yellow Pine | No. 1 and better, grade (Southern Pine Inspection Bureau) |

When the material is delivered to the project, the Engineer will check the order for the appropriate grade stamp. The grade markings shall not constitute final acceptance of the material. The Engineer may reject any or all of the timber or lumber that does not comply with the specifications or has been damaged during shipping or upon delivery.

9-09.2(3) Inspection

This section including title is revised to read:
9-09.2(3) Sign Posts, Mileposts, Sawed Fence Posts, and Mailbox Posts
The allowable species of timber and lumber for signposts, and mileposts shall be Douglas Fir-Larch or Hem Fir. Timber and lumber for sawed fence posts and mailbox posts shall be Western Red Cedar, Douglas Fir-Larch, or Hem Fir.

Sign posts, mileposts, sawed fence posts, and mailbox posts shall conform to the grades shown below.

<table>
<thead>
<tr>
<th>Size</th>
<th>Grade Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>4&quot; x 4&quot;</td>
<td>Construction grade (Light Framing, Section 122-b WCLIB) or (Section 40.11 WWPA)</td>
</tr>
<tr>
<td>4&quot; x 6&quot;</td>
<td>No. 1 and better, grade (Structural Joists and Planks, Section 123-b WCLIB) or</td>
</tr>
<tr>
<td></td>
<td>(Section 62.11 WWPA)</td>
</tr>
<tr>
<td>6&quot; x 6&quot;, 6&quot; x 8&quot;, 8&quot; x 10&quot;</td>
<td>No. 1 and better, grade (Posts and Timbers, Section 131-b WCLIB) or (Section</td>
</tr>
<tr>
<td></td>
<td>80.11 WWPA)</td>
</tr>
<tr>
<td>6&quot; x 10&quot;, 6&quot; x 12&quot;</td>
<td>No. 1 and better, grade (Beams and Stringers, Section 130-b WCLIB) or (Section</td>
</tr>
<tr>
<td></td>
<td>70.11 WWPA)</td>
</tr>
</tbody>
</table>

SECTION 9-14, EROSION CONTROL AND ROADSIDE PLANTING
April 6, 2009

9-14.4(4) Vacant
This section including title is revised to read:

9-14.4(4) Wood Strand Mulch
Wood strand mulch shall be a blend of loose, long, thin wood pieces derived from native conifer or deciduous trees with high length-to-width ratio. A minimum of 95% of the wood strand shall have lengths between 2 and 10-inches, with a width and thickness between 1/16 and 3/8-inches.

The mulch shall not contain resin, tannin, or other compounds in quantities that would be detrimental to plant life. Sawdust or wood shavings shall not be used as mulch.

9-14.4(8) Compost
This section is revised to read:

Compost products shall be the result of the biological degradation and transformation of plant-derived materials under controlled conditions designed to promote aerobic decomposition. Compost shall be stable with regard to oxygen consumption and carbon dioxide generation. Compost shall be mature with regard to its suitability for serving as a soil amendment or an erosion control BMP as defined below. The compost shall have a moisture content that has no visible free water or dust produced when handling the material.

Compost production and quality shall comply with Chapter 173-350 WAC.
Compost products shall meet the following physical criteria:
1. Compost material shall be tested in accordance with U.S. Composting Council Testing Methods for the Examination of Compost and Composting (TMECC) 02.02-B, “Sample Sieving for Aggregate Size Classification”.

Fine Compost shall meet the following:

<table>
<thead>
<tr>
<th></th>
<th>Min.</th>
<th>Max.</th>
</tr>
</thead>
<tbody>
<tr>
<td>Percent passing 2&quot;</td>
<td>100%</td>
<td></td>
</tr>
<tr>
<td>Percent passing 1&quot;</td>
<td>95%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 5/8&quot;</td>
<td>90%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 1/4&quot;</td>
<td>75%</td>
<td>100%</td>
</tr>
<tr>
<td>Maximum particle length of 6 inches</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Coarse Compost shall meet the following:

<table>
<thead>
<tr>
<th></th>
<th>Min.</th>
<th>Max.</th>
</tr>
</thead>
<tbody>
<tr>
<td>Percent passing 3&quot;</td>
<td>100%</td>
<td></td>
</tr>
<tr>
<td>Percent passing 1&quot;</td>
<td>90%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 3/4&quot;</td>
<td>70%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 1/2&quot;</td>
<td>40%</td>
<td>60%</td>
</tr>
<tr>
<td>Maximum particle length of 6 inches</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

2. The pH shall be between 6.0 and 8.5 when tested in accordance with U.S. Composting Council TMECC 04.11-A, “1:5 Slurry pH”.

3. Manufactured inert material (plastic, concrete, ceramics, metal, etc.) shall be less than 1.0 percent by weight as determined by U.S. Composting Council TMECC 03.08-A “Classification of Inerts by Sieve Size”.

4. Minimum organic matter shall be 40 percent by dry weight basis as determined by U.S. Composting Council TMECC 05.07A “Loss-On-Ignition Organic Matter Method (LOI)”.

5. Soluble salt contents shall be less than 4.0 mmhos/cm when tested in accordance with U.S. Composting Council TMECC 04.10 “Electrical Conductivity”.

6. Maturity shall be greater than 80% in accordance with U.S. Composting Council TMECC 05.05-A, “Germination and Root Elongation”.

7. Stability shall be 7 mg CO₂-C/g OM/day or below in accordance with U.S. Composting Council TMECC 05.08-B “Carbon Dioxide Evolution Rate”.

8. The compost product must originate a minimum of 65 percent by volume from recycled plant waste as defined in WAC 173-350 as “Type 1 Feedstocks.” A maximum of 35 percent by volume of “Type 2 Feedstocks,” source-separated food
waste, and/or biosolids may be substituted for recycled plant waste. The manufacturer shall provide a list of feedstock sources by percentage in the final compost product.

9. The Engineer may also evaluate compost for maturity using U.S. Composting Council TMECC 05.08-E “Solvita® Maturity Index”. Fine Compost shall score a number 6 or above on the Solvita® Compost Maturity Test. Coarse Compost shall score a 5 or above on the Solvita® Compost Maturity Test.

This section is supplemented with the following new sub-sections:

9-14.4(8)A Compost Approval
The Contractor shall either select a compost manufacturer from the Qualified Products List, or submit the following information to the Engineer for approval:

1. A Request for Approval of Material Source.

2. A copy of the Solid Waste Handling Permit issued to the manufacturer by the Jurisdictional Health Department as per WAC 173-350 (Minimum Functional Standards for Solid Waste Handling).

3. The manufacturer shall verify in writing, and provide lab analyses that the material complies with the processes, testing, and standards specified in WAC 173-350 and these specifications. An independent Seal of Testing Assurance (STA) Program certified laboratory shall perform the analysis.

4. A copy of the manufacturer’s Seal of Testing Assurance STA certification as issued by the U.S. Composting Council.

9-14.4(8)B Compost Acceptance
Seven days prior to initial application of any compost the Contractor shall submit a compost sample, a STA test report dated within 90 calendar days, and the list of feedstocks by volume for each compost type to the Engineer for review.

The Contractor shall use only compost that has been tested within 90 calendar days of application and meets the requirements in section 9-14.4(8). Compost not conforming to the above requirements or taken from a source other than those tested and accepted shall be immediately removed from the project and replaced at no cost to the Contracting Agency.

9-14.5(1) Polyacrylamide (PAM)
The second sentence is revised to read:

PAM shall be anionic and shall be linear, and not cross-linked.

9-14.5(3) Clear Plastic Covering
This section is revised to read:
Clear plastic covering shall conform to the requirements of ASTM D 4397, for polyethylene sheeting having a minimum thickness of 6 mils.

9-14.5(7) Coir Log
The reference to Standard Plans in the second sentence of the first paragraph is revised to read Plans.

SECTION 9-23, CONCRETE CURING MATERIALS AND ADMIXTURES
April 6, 2009

9-23.6 Admixture for Concrete
This section including title is revised to read:

9-23.6 Chemical Admixtures for Concrete
Acceptance of chemical admixtures will be based on Manufacturer’s Certificate of Compliance. If required by the Engineer, admixtures shall be sampled and tested before they are used. A one-pint (500 milliliter) sample of the admixture shall be submitted to the WSDOT Headquarters Materials Laboratory for testing 10 days prior to use. Chemical Admixtures shall contain less than one percent chloride ion (Cl-) by weight of admixture.

This section is supplemented with the following new sub-sections.

9-23.6(1) Air Entraining Admixtures
Air Entraining Admixtures shall meet the requirements of AASHTO M 154 or ASTM C 260.

9-23.6(2) Type A Water-Reducing Admixtures
Type A Water-Reducing admixtures shall conform to the requirements of AASHTO M 194 Type A or ASTM C 494 Type A.

9-23.6(3) Type B Retarding Admixtures
Type B Retarding admixtures shall conform to the requirements of AASHTO M 194 Type B or ASTM C 494 Type B.

9-23.6(4) Type C Accelerating Admixtures
Type C Accelerating admixtures shall conform to the requirements of AASHTO M 194 Type C or ASTM C 494 Type C and only non-chloride accelerating admixtures shall be used.

9-23.6(5) Type D Water-Reducing and Retarding Admixtures
Type D Water-Reducing and Retarding admixtures shall conform to the requirements of AASHTO M 194 Type D or ASTM C 494 Type D.
9-23.6(6) Type E Water-Reducing and Accelerating Admixtures
Type E Water-Reducing and Accelerating admixtures shall conform to the requirements of AASHTO M 194 Type E or ASTM C 494 Type E and only non-chloride accelerating admixtures shall be used.

9-23.6(7) Type F Water-Reducing, High Range Admixtures
Type F Water-Reducing, High Range admixtures shall conform to the requirements of AASHTO M 194 Type F or ASTM C 494 Type F.

9-23.6(8) Type G Water-Reducing, High Range and Retarding Admixtures
Type G Water-Reducing, High Range and Retarding admixtures shall conform to the requirements of AASHTO M 194 Type G or ASTM C 494 Type G.

9-23.6(9) Type S Specific Performance Admixtures
Type S Specific Performance Admixtures shall conform to the requirements of ASTM C 494 Type S. When a Type S admixture is used a report on the performance characteristics of the Type S admixture shall be submitted along with the WSDOT concrete mix design (WSDOT Form 350-040). The report shall describe the performance characteristics and provide data substantiating the specific characteristics of the Type S admixture in accordance with ASTM C 494.

9-23.7 Air Entraining and Chemical Admixtures for Precast Prestressed Concrete
This section including title is revised to read:

9-23.7 Vacant

9-23.9 Fly Ash
This section is supplemented with the following:

Fly ash that exceeds the available alkalies limits set in AASHTO M 295 Table 2 may be used if they meet the tests requirements of Section 9-03.1(1). The optional chemical limits in AASHTO M 295 Table 2 do not apply to fly ash used in Controlled Density Fill.

SECTION 9-29, ILLUMINATION, SIGNAL, ELECTRICAL
April 6, 2009

9-29.1 Conduit, Innerduct, and Outerduct
This section's content is deleted. This section is supplemented with the following:

Conduit shall be free from defects, including out of round, and foreign inclusions. Conduit shall be uniform in color, density, and physical properties. The inside shall be smooth and free from burrs which could damage cable during installation. Conduit ends shall be cut square to the inside diameter, and supplied with thread protectors. All conduit, conduit fittings, and associated hardware/appurtenances shall be listed by a Nationally Recognized Testing Laboratory.
9-29.1(1) Rigid Metal Conduit, Galvanized Steel Outerduct, and Fittings
Rigid metal conduit, shall be straight, and be rigid galvanized steel, or stainless steel, as
required and bear the mark of a Nationally Recognized Testing Laboratory. Exterior and
interior surfaces of the galvanized steel conduit, except threaded ends, shall be uniformly
and adequately zinc coated by a hot-dip galvanizing process. The average of the zinc coating
shall comply with Federal Specification WW-C-581d.

9-29.1(2) Rigid Metal Conduit Fittings and Appurtenances
Couplings for rigid metal type conduits may be either hot-dip or electroplated galvanized.

Conduit bodies and fittings for rigid steel conduit systems shall be listed by Nationally
Recognized Testing Laboratory listed for wet locations, and shall be hot-dip galvanized
malleable iron, or bronze. Conduit bodies shall have tapered threads, and include a bolt on
cover with stainless steel screws and a neoprene gasket seal.

Grounding end bushings shall be bronze or galvanized malleable iron with copper, tinned
copper, stainless steel, or integral lug with stainless steel clamping screw, mounting screw
and set screw.

Conduit clamps and straps shall be type 304 or type 316 stainless steel or hot-dip
galvanized. Two-hole type straps shall span the entire width of the support channel and
attach to the supports on both sides of the conduit with bolts and associated hardware. Two
piece conduit clamps shall interlock with the support channel with a single bolt.

Conduit supports for surface mounted conduit shall be hot-dip galvanized or type 304 or
type 316 stainless steel channel using type 304 or type 316 stainless steel bolts and spring
nuts.

9-29.1(2A) Expansion Fittings, Deflection Fittings, and Combination
Expansion/Deflection Fittings
Expansion fittings for rigid galvanized steel conduit shall be weather tight, with hot-dip
galvanized malleable or ductile iron end couplings and body and shall allow for 4-inches of
movement minimum (2-inches in each direction). Expansion fittings for rigid galvanized
steel conduit shall have an external tinned copper bonding jumper or an internal tinned
copper bonding jumper. The internal tinned copper bonding jumper shall not reduce the
conduit conductor capacity.

Deflection fittings for rigid galvanized steel conduit shall be weather tight, with hot-dip
galvanized ductile iron or bronze end couplings, with molded neoprene sleeve, stainless
steel bands and internal tinned copper bonding jumper. Deflection fittings shall provide for
conduit movement of ¼-inch in all directions and angular movement of 30 degree in any
direction.

A combination of a deflection and an expansion fitting for rigid galvanized steel conduit
shall be assembled from a deflection fitting and an expansion fitting as defined above.
The bonding jumper used for expansion fittings and combination expansion deflection fittings shall be a tinned copper braid attached to the conduit with a galvanized “U” bolt type connection designed for the application.

9-29.1(3) **Flexible Metal Conduit**
Liquidtight flexible metal conduit shall consist of a single strip of continuous flexible interlocked steel galvanized inside and out, forming a smooth internal wiring channel with a liquid tight covering of sunlight resistant flexible PVC conforming to NEC Article 350.

9-29.1(3)A **Flexible Metal Conduit Appurtenances**
Liquidtight connectors shall be the insulated throat type, conforming to NEC Article 350, and listed for wet locations.

9-29.1(4) **Non-Metallic Conduit**

9-29.1(4)A **Rigid PVC Conduit**
Rigid PVC conduit shall conform to NEMA TC 2 and ASTM F 2136, and UL 651. Fittings shall conform to NEMA TC-3, and be UL 514C and UL 651.

PVC solvent cement shall meet ASTM D 2564 including note 8 (label to show pipe sizes for which the cement is recommended).

9-29.1(4)B **HDPE Conduit**
HDPE conduit shall be listed by a Nationally Recognized Testing Laboratory. Couplings for HDPE shall be mechanical and listed for use with HDPE.

Aluminum mechanical couplings are prohibited.

9-29.1(5) **Innerduct and Outerduct**
The innerduct system shall be factory-installed and shall be designed so that expansion and contraction of the innerducts takes place in the coupling body to eliminate compatibility problems. The conduit coupling body shall have a factory-assembled gasket that is multi-stage and anti-reversing, sealing both the outerduct and innerducts. A secondary mid-body O-ring gasket shall be seated into the coupling body and shall hold the coupling body firmly in the outerduct.

All fittings, adapters, and bends (sweeps) shall be provided and shall be manufactured from the same materials and manufacturing process as the conduit, except as specified otherwise. The conduit system shall be a complete system with the following accessories:

- Manhole Terminator Kits
- Deflection Fittings
- Offset Fittings
- Expansion/Contraction Fittings
- Repair Kits
- Conduit and Innerduct Plugs
Pull string
Pull rope
Conduit spacers
Split Plugs

9-29.1(5)A Rigid Galvanized Steel Outerduct with PVC or PE Innerduct
Each section of steel outerduct shall be supplied with one reversing spin coupling that allows straight sections and fittings to be joined without spinning the conduit. The reversing coupling shall be galvanized and have three setscrews or a lock nut ring to lock the coupling in place. Setscrews or lock nut ring shall be galvanized or stainless steel and insure continuous electrical ground. The couplings shall be galvanized steel with the same material properties as the conduit.

The conduit system shall be designed so that assembly of components can be accomplished in the following steps:

1. Loosen setscrews or lock nut ring on coupling and spin back to allow for insertion.
2. Spin coupling mating sections forward to bottom.
3. Tighten setscrews on lock nut ring.

9-29.1(5)B Rigid PVC Outerduct with PVC or PE Innerduct
Protective outerduct for schedule 40 PVC and schedule 80 PVC conduit outerduct shall be 4-inch with a minimum 5-inch extended integral “bell end” and shall be gray in color. The outerduct minimum wall thickness shall be 0.23-inch for Schedule 40 PVC and 0.32-inch for Schedule 80 PVC.

Conduit and fittings for PVC outerduct shall be manufactured with an ultraviolet inhibitor.

The coupling body for PVC outerduct shall include a factory-assembled, multi-stage gasket that is anti-reversing, sealing both the outer and innerducts. A secondary mid-body gasket shall be seated at the shoulder of the bell to assure air and water integrity of the system. The bell end and the coupling body assembly shall accept a minimum of 5-inches of the spigot end.

The conduit system shall be designed so that straight sections and fittings will assemble without the need for lubricants or cement.

PVC outerduct shall have a longitudinal print-line that denotes “Install This Side Up” for proper innerduct alignment. PVC outer-ducts shall have a circumferential ring on the spigot end of the duct to provide a reference point for ensuring the proper insertion depth when connecting conduit ends. The line shall be a minimum of 5-inches from the end of the conduit.
9-29.1(5)C Innerduct for Straight Sections of Galvanized Steel Outerduct or PVC
Outerduct
The innerducts shall have a minimum outside diameter of 1.25-inch, and a minimum inside diameter of 1.2-inch. Larger diameter innerducts may be provided if the wall thickness and diameter tolerances are met. The tolerance for inside and outside diameters shall be 0.005-inch. The innerducts shall have a minimum wall thickness of 0.060-inch. Innerduct shall be color coded and shall index a minimum of one innerduct with a different color. Alternate color codes are permitted as long as the color codes are contiguous between adjacent junction boxes. The innerducts shall be factory installed in the outerduct.

Dynamic coefficient of friction of innerducts shall be tested in accordance with Telcordia GR-356-CORE procedure. The coefficient of friction shall be less than 0.30 between medium density polyethylene jacketed fiber optic cable and the prelubricated innerduct. The coefficient of friction shall be less than 0.10 between the 1/4-inch diameter polypropylene rope (suitable for fiber optic cable pulling) and the prelubricated innerduct. Pull rope used for testing (meeting the 0.10 coefficient of friction requirement) shall be the same type as the pull rope used for cable installation. The Contractor shall provide as part of the conduit submittals a certificate of compliance with these coefficient of friction requirements.

The innerduct shall have a smooth, non-ribbed interior surface, with a factory prelubricated coating. The coating shall provide the required dynamic coefficient of friction.

Innerduct shall be extruded polyvinyl chloride (PVC) or polyethylene (PE).

The coupling body for the innerduct shall be factory assembled in the bell end of the outerduct and shall be manufactured from a high impact engineered thermoplastic. The coupling body face shall be supplied with lead-ins to facilitate assembly.

All outerduct shall be marked with data traceable to plant location.

9-29.1(5)D Conduit with Innerducts Fittings and Appurtenances
Duct plugs shall be polypropylene and be equipped with a neoprene or polyurethane gasket. Plugs shall be equipped with an attachment to secure the pull rope in the innerduct. The plug shall withstand 5 psi.

9-29.1(5)D1 Bends for 4-inch PVC Conduit with Innerducts or Galvanized Steel Conduit with Innerducts
All bend radii shall be 36-inches or greater. The conduit system shall provide a complete line of fixed and flexible sweeps with system compatible bell and spigot or threaded ends. The bends shall contain high-temperature burn-through-resistant innerducts manufactured from PVC, PE, or Nylon-66. The innerducts shall meet all other requirements for innerduct In Sections 9-29.1(1) and 9-29.1(5)A.
9-29.1(5)D2 Prefabricated Fixed and Flexible Bends (for Innerducts)
The prefabricated standard fixed PVC bends shall have a radius between 4-feet and 9-feet
and sweep angles of 11.25-degree, 22.5-degree, 45-degree, or 90-degree.

Flexible bends shall be prefabricated. These conduits may be field bent to a uniform radius
no less than 4-feet. The field bend shall be no greater than 90-degrees. Grounding shall be
continuous in flexible bends. Outerduct for flexible ends shall be manufactured from
reinforced PVC. Expansion and Deflection fittings for rigid galvanized steel conduit with
innerduct shall be provided in accordance with 9-29.1(2)A.

9-29.1(6) Detectable Underground Warning Tape
Detectable Underground Warning tape shall be Orange imprinted in black lettering with the
message; “FIBER OPTIC CABLE BURIED BELOW” or equal. The warning tape shall be
polyethylene with a metallic backing. The polyethylene shall be a minimum 4-mils thick and
3-inches wide.

9-29.1(7) Steel Casings
Steel casing material shall conform to ASTM A 252 Grade 2 or 3 or casing as approved by
the Engineer. The Contractor shall furnish pipe of adequate thickness to withstand the forces
exerted by the boring operation as well as those forces exerted by the earth during
installation and shall be a minimum of ¾-inch thick. All joints shall be welded by a welder
qualified in accordance with AWS D1.1 structural welding code, section 3.

9-29.1(8) Drilling Fluid
Drilling fluid used for directional boring shall be an inert mixture of water and bentonite
clay, conforming to the drilling equipment manufacturers recommendations.

9-29.3 Conductors, Cable
This section’s content is deleted. This section’s title is revised to read:

9-29.3 Fiber Optic Cable, Electrical Conductors, and Cable

9-29.3(1)A Singlemode Fiber Optic Cable
This section is revised to read:

Singlemode fibers utilized in the cables specified herein shall be fabricated from 100 kpsi
proof stress glass and primarily composed of silica which shall provide a matched clad
index of refraction (n) profile and the following physical and performance characteristics:

1. Maximum Attenuation: 0.4/0.3 dB/km at 1310/1550 nanometers, respectively;
2. Typical Core Diameter: 8.3 microns;
3. Cladding Diameter: 125 micron;
4. Core-to-Cladding Offset (Defined as the distance between the core center and the cladding center: < 0.8 microns;

5. Cladding Non-Circularity (Defined as \[ \frac{[1-(\text{minimum cladding diameter} - \text{maximum cladding diameter})] \times 100.}{\text{maximum cladding diameter}} \text{ < 2.0\%;} \]

6. Coating Diameter of 250 microns ± 15 microns with a minimum coating thickness at any point of not less than 50 microns;

7. The coating shall be a dual-layered, UV-cured acrylate applied by the fiber manufacturer; and,

8. The coating shall be mechanically or chemically strippable without damaging the fiber.

9-29.3(2) Twisted-Pair (TWP) Copper Cable

This section's content is deleted. This section's title is revised to read:

9-29.3(2) Electrical Conductors and Cable

This section is supplemented with the following new sub-sections:

9-29.3(2)A Single Conductor

9-29.3(2)A1 Single Conductor Current Carrying

All current carrying single conductors shall be stranded copper conforming to ASTM B3 and B8. Insulation shall be chemically XLP (cross-linked polyethylene) or EPR (Ethylene Propylene Rubber) Type USE rated for 600 volt.

9-29.3(2)A2 Grounding Electrode Conductor

Grounding electrode conductor shall be bare or insulated stranded copper. The insulation shall be green or green with a yellow tracer.

9-29.3(2)A3 Equipment Grounding and Bonding Conductors

Equipment grounding and bonding jumper conductors shall be bare or green insulated, stranded copper with cross-linked polyethylene insulation rated USE and 600 volts, with the exception that the equipment grounding and bonding jumper conductors installed between junction box, pull box, or cable vault frame and lids shall be tinned, braided copper.

9-29.3(2)A4 Location Wire

Location wire shall be a single stranded copper size AWG 14 insulated conductor. The insulation shall be type USE Orange in color.

9-29.3(2)B Multi-Conductor Cable

Two conductor through 10 conductor unshielded signal control cable shall conform to International Municipal Signal Association (IMSA) signal cable Specification 20-1.
9-29.3(2)C Aluminum Cable Steel Reinforced

Triplex or Quadruplex type ACSR neutral self-supporting aerial conductors of the appropriate size for aluminum conductors shall be used where required in the Contract. The neutral conductor shall be the same size as the insulated conductor. All conductors shall be stranded.

9-29.3(2)D Pole and Bracket

Pole and bracket cable shall be a two-conductor cable rated for 600 volts. The individual conductors shall be one red and one black 19-strand No. 10 AWG copper, assembled parallel. The conductor insulation shall be 45-mil polyvinyl chloride or a 600 volt rated cross-linked polyethylene. The Jacketing shall be polyethylene or polyvinyl chloride not less than 45-mils thick. If luminaires with remote ballasts are specified in the Contract, this same cable shall be used between luminaire and ballast for both timber and ornamental pole construction. If the luminaire requires fixture wire temperatures greater than 75°C, the outer jacket shall be stripped for that portion of the cable inside the luminaire. The single conductors shall then be sheathed with braided fiberglass sleeving of the temperature rating recommended by the luminaire manufacturer.

9-29.3(2)E Two-Conductor Shielded

Two conductor shielded (2CS) cable shall have 14 AWG (minimum) conductors and shall conform to IMSA Specification No. 50-2.

9-29.3(2)F Detector Loop Wire

Detector loop wire may be 12 or 14 AWG stranded copper wire, IMSA 51-3

9-29.3(2)G Four-Conductor Shielded Cable

Four conductor shielded cable (4CS) shall consist of a cable with four 18 AWG conductors with polypropylene insulation, an aluminized polyester shield, water blocking material in the cable interstices, and a 26-mil minimum outer jacket of polyethylene. The four-conductor assembly shall be twisted 6 turns per foot. Each conductor shall have a different insulation color. Overall cable diameter shall be 0.25-inch maximum. Capacitance between adjacent pairs shall be 18 pF per foot and 15 pF per foot between diagonal pairs. The capacitances shall not vary more than 10 percent after a 10-day immersion test with ends exposed in a saturated brine solution.

9-29.3(2)H Three-Conductor Shielded Cable

Three-conductor shielded cable (3CS) for the detector circuit for optical fire preemption receivers shall consist of three 20 AWG conductors with aluminized mylar shield and one No. 20 drain wire, all enclosed with an outer jacket. All wires shall be 7 X 28 stranded tinned copper material. Conductor insulation shall be rated 75°C, 600 volt. The drain wire shall be uninsulated. Conductor color coding shall be yellow, blue, and orange. DC resistance of any conductor or drain wire shall not exceed 11 ohms per 1,000-feet. Capacitance from one conductor to the other two conductors and shield shall not exceed 48 pF per foot. The jacket shall be rated 80 degree C, 600 volt, with a minimum average wall thickness of 0.045-inch. The finished outside diameter of the cable shall be 0.3-inch maximum.
9-29.3(2) Twisted Pair Communications Cable
Twisted Pair Communications Cable shall meet RUS Specification 1755.390 and shall be AWG22 conductor. The cable shall have a petroleum compound completely filling the inside of the cable and rated for OSP (Outside Plant) applications.

9-29.6 Light and Signal Standards
This section is supplemented with the following:

Materials for steel light and signal standards, and associated anchorage and fastening hardware, shall conform to Sections 9-29.6(1), 9-29.6(2) and 9-29.6(5) unless otherwise specified in one of the following documents:

1. The steel light and signal standard fabricator's pre-approved plan as approved by the Washington State Department of Transportation and as identified in the Special Provisions.

2. The steel light and signal standard fabricator's shop drawing submittal, including supporting design calculations, as submitted in accordance with Sections 6-01.9 and 8-20.2(1) and the Special Provisions, and as approved by the Engineer.

SECTION 9-30, WATER DISTRIBUTION MATERIALS
December 1, 2008

9-30.3(1) Gate Valves (3-inches to 16-inches)
The second paragraph is revised to read:

The Contractor shall provide an affidavit of compliance stating that the valve furnished fully complies with AWWA C509 or AWWA C515.
SPECIAL PROVISIONS
CHEYNE LANDFILL SEPTAGE LAGOONS

SP 3265

YAKIMA COUNTY, WASHINGTON

INTRODUCTION TO THE SPECIAL PROVISIONS

(July 31, 2007 APWA GSP)

The work on this project shall be accomplished in accordance with the Standard Specifications for Road, Bridge and Municipal Construction, 2008 edition, as issued by the Washington State Department of Transportation (WSDOT) and the American Public Works Association (APWA), Washington State Chapter (hereafter “Standard Specifications”). The Standard Specifications, as modified or supplemented by the Amendments to the Standard Specifications and these Special Provisions, all of which are made a part of the Contract Documents, shall govern all of the Work.

These Special Provisions are made up of both General Special Provisions (GSPs) from various sources, which may have project-specific fill-ins; and project-specific Special Provisions. Each Provision either supplements, modifies, or replaces the comparable Standard Specification, or is a new Provision. The deletion, amendment, alteration, or addition to any subsection or portion of the Standard Specifications is meant to pertain only to that particular portion of the section, and in no way should it be interpreted that the balance of the section does not apply.

The project-specific Special Provisions are not labeled as such. The GSPs are labeled under the headers of each GSP, with the date of the GSP and its source, as follows:

(May 18, 2007 APWA GSP)
(August 7, 2006 WSDOT GSP)

Also incorporated into the Contract Documents by reference are:

- Manual on Uniform Traffic Control Devices for Streets and Highways, currently adopted edition, with Washington State modifications, if any
- Standard Plans for Road, Bridge and Municipal Construction, WSDOT/APWA, current edition

Contractor shall obtain copies of these publications, at Contractor’s own expense.
SPECIAL PROVISIONS

The following Special Provisions are made a part of this contract and supersede any conflicting provisions of the 2008 Standard Specifications for Road, Bridge and Municipal Construction, and the foregoing Amendments to the Standard Specifications.

Several types of Special Provisions are included in this contract; General, Region, Bridges and Structures, and Project Specific. Special Provisions types are differentiated as follows:

(date) General Special Provision
(******) Notes a revision to a General Special Provision
and also notes a Project Specific Special Provision.
(Regions\(^1\) date) Region Special Provision
(BSP date) Bridges and Structures Special Provision

General Special Provisions are similar to Standard Specifications in that they typically apply to many projects, usually in more than one Region. Usually, the only difference from one project to another is the inclusion of variable project data, inserted as a “fill-in”.

Region Special Provisions are commonly applicable within the designated Region. Region designations are as follows:

<table>
<thead>
<tr>
<th>Regions(^1)</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>ER</td>
<td>Eastern Region</td>
</tr>
<tr>
<td>NCR</td>
<td>North Central Region</td>
</tr>
<tr>
<td>NWR</td>
<td>Northwest Region</td>
</tr>
<tr>
<td>OR</td>
<td>Olympic Region</td>
</tr>
<tr>
<td>SCR</td>
<td>South Central Region</td>
</tr>
<tr>
<td>SWR</td>
<td>Southwest Region</td>
</tr>
<tr>
<td>WSF</td>
<td>Washington State Ferries Division</td>
</tr>
</tbody>
</table>

Bridges and Structures Special Provisions are similar to Standard Specifications in that they typically apply to many projects, usually in more than one Region. Usually, the only difference from one project to another is the inclusion of variable project data, inserted as a “fill-in”.

Project Specific Special Provisions normally appear only in the contract for which they were developed.
DIVISION 1
GENERAL REQUIREMENTS

DESCRIPTION OF WORK

(******)

This contract provides for construction of septage lagoons for Cheyne Landfill, including site grading, piping, electrical, concrete discharge station, two (2) 0.76 acre solids lagoons, two (2) 1.47 acre liquid lagoons with 60 mil HDPE primary liner, geocomposite drainage system, 60 mil HDPE secondary liner, and other appurtenances as shown in the plans.

This contract also provides for decommissioning an existing well, which consists of perforating the existing well casing, grouting, cutting, and capping the existing onsite well by a licensed well driller.

The following general sequence of work is contemplated:

1. Mobilize all required materials and equipment to the job site.
2. Decommission existing well to within 20 feet of existing surface.
3. Excavate lagoons to lines and grades shown on the Drawings and as described in these specifications. Excess material from the excavation operations shall be stockpiled on-site at the locations shown on the plans, and/or at the locations staked by the Engineer.
4. Perform all piping, electrical, and concrete work as shown on the Drawings, including walkway structures.
5. Install geomembrane liner and finish or restore all surfaces as shown on the Drawings.
6. Construct access roads.

Construction activities are not likely to bring workers in close contact with chemically hazardous substances derived from the landfill. However, landfill gas may be encountered during construction. Therefore, the Contractor shall have hazardous waste safety training.
SECTION 1-01 DEFINITIONS AND TERMS

1-01.3 Definitions

(September 12, 2008 APWA GSP)

This Section is supplemented with the following:

All references in the Standard Specifications to the terms “State”, “Department of Transportation”, “Washington State Transportation Commission”, “Commission”, “Secretary of Transportation”, “Secretary”, “Headquarters”, and “State Treasurer” shall be revised to read “Contracting Agency”.

All references to “State Materials Laboratory” shall be revised to read “Contracting Agency designated location”.

The venue of all causes of action arising from the advertisement, award, execution, and performance of the contract shall be in the Superior Court of the County where the Contracting Agency’s headquarters are located.

Additive
A supplemental unit of work or group of bid items, identified separately in the proposal, which may, at the discretion of the Contracting Agency, be awarded in addition to the base bid.

Alternate
One of two or more units of work or groups of bid items, identified separately in the proposal, from which the Contracting Agency may make a choice between different methods or material of construction for performing the same work.

Contract Documents
See definition for “Contract”.

Contract Time
The period of time established by the terms and conditions of the contract within which the work must be physically completed.

Dates

*Bid Opening Date*
The date on which the Contracting Agency publicly opens and reads the bids.

*Award Date*
The date of the formal decision of the Contracting Agency to accept the lowest responsible and responsive bidder for the work.

*Contract Execution Date*
The date the Contracting Agency officially binds the agency to the contract.
Notice to Proceed Date
The date stated in the Notice to Proceed on which the contract time begins.

Substantial Completion Date
The day the Engineer determines the Contracting Agency has full and unrestricted use and benefit of the facilities, both from the operational and safety standpoint, and only minor incidental work, replacement of temporary substitute facilities, or correction or repair remains for the physical completion of the total contract.

Physical Completion Date
The day all of the work is physically completed on the project. All documentation required by the contract and required by law does not necessarily need to be furnished by the Contractor by this date.

Completion Date
The day all the work specified in the contract is completed and all the obligations of the Contractor under the contract are fulfilled by the Contractor. All documentation required by the contract and required by law must be furnished by the Contractor before establishment of this date.

Final Acceptance Date
The date on which the Contracting Agency accepts the work as complete.

Notice of Award
The written notice from the Contracting Agency to the successful bidder signifying the Contracting Agency’s acceptance of the bid.

Notice to Proceed
The written notice from the Contracting Agency or Engineer to the Contractor authorizing and directing the Contractor to proceed with the work and establishing the date on which the contract time begins.

Traffic
Both vehicular and non-vehicular traffic, such as pedestrians, bicyclists, wheelchairs, and equestrian traffic.
SECTION 1-02 BID PROCEDURES AND CONDITIONS

1-02.1 Prequalification of Bidders

Delete this Section and replace it with the following:

1-02.1 Qualifications of Bidder
(March 25, 2009 APWA GSP)

Bidders must meet the minimum qualifications of RCW 39.04.350(1), as amended:

"Before award of a public works contract, a bidder must meet the following responsibility criteria to be considered a responsible bidder and qualified to be awarded a public works project. The bidder must:

(a) At the time of bid submittal, have a certificate of registration in compliance with chapter 18.27 RCW;
(b) Have a current state unified business identifier number;
(c) If applicable, have industrial insurance coverage for the bidder's employees working in Washington as required in Title 51 RCW; an employment security department number as required in Title 50 RCW; and a state excise tax registration number as required in Title 82 RCW; and
(d) Not be disqualified from bidding on any public works contract under RCW 39.06.010 or 39.12.065(3)."

Section 1-02.1 is supplemented with the following:

(*****)
Qualifications of Bidder
Bidders shall be qualified by experience, financing, equipment, and organization to do the work called for in the Contract Documents. The Contracting Agency reserves the right to take whatever action it deems necessary to ascertain the ability of the bidder to perform the work satisfactorily.

In addition the Contractor shall submit the following information within the bid package:

1. Contact person and proposed well driller to decommission existing well.
2. Company and driller’s experience.
3. Demonstration of company and driller’s past experience.
4. Proposed equipment to complete the job including excavation equipment, drill equipment, drill tools, and dimensions.
5. Availability.

1-02.2 Plans and Specifications
(October 1, 2005 APWA GSP)

Delete this section and replace it with the following:

Information as to where Bid Documents can be obtained or reviewed will be found in the
Call for Bids (Advertisement for Bids) for the work.

After award of the contract, plans and specifications will be issued to the Contractor at no
cost as detailed below:

To Prime Contractor  No. of Sets  Basis of Distribution

Reduced plans (11" x 17")  10  Furnished automatically
and Contract Provisions  upon award.

Large plans (22" x 34")  3  Furnished only upon
and Contract Provisions  request.

Additional plans and Contract Provisions may be purchased by the Contractor by payment of
the cost stated in the Call for Bids.

1-02.5 Proposal Forms
(October 1, 2005 APWA GSP)

Delete this section and replace it with the following:

At the request of a bidder, the Contracting Agency will provide a proposal form for any
project on which the bidder is eligible to bid.

The proposal form will identify the project and its location and describe the work. It will also
list estimated quantities, units of measurement, the items of work, and the materials to be
furnished at the unit bid prices. The bidder shall complete spaces on the proposal form that
call for, but are not limited to, unit prices; extensions; summations; the total bid amount;
signatures; date; and, where applicable, retail sales taxes and acknowledgment of addenda;
the bidder's name, address, telephone number, and signature; the bidder's D/M/WBE
commitment, if applicable; a State of Washington Contractor's Registration Number; and a
Business License Number, if applicable. Bids shall be completed by typing or shall be
printed in ink by hand, preferably in black ink. The required certifications are included as
part of the proposal form.

The Contracting Agency reserves the right to arrange the proposal forms with alternates and
additives, if such be to the advantage of the Contracting Agency. The bidder shall bid on all
alternates and additives set forth in the proposal forms unless otherwise specified.
Any correction to a bid made by interlineation, alteration, or erasure, shall be initialed by the
signer of the bid. The bidder shall make no stipulation on the Bid Form, nor qualify the bid in
any manner.

A bid by a corporation shall be executed in the corporate name, by the president or a vice
president (or other corporate officer accompanied by evidence of authority to sign).

A bid by a partnership shall be executed in the partnership name, and signed by a partner. A
copy of the partnership agreement shall be submitted with the Bid Form if any D/M/WBE
requirements are to be satisfied through such an agreement.

A bid by a joint venture shall be executed in the joint venture name and signed by a member
of the joint venture. A copy of the joint venture agreement shall be submitted with the Bid
Form if any D/W/MBE requirements are to be satisfied through such an agreement.

1-02.6 Preparation of Proposal

(October 10, 2008 APWA GSP)

Supplement the second paragraph with the following:

4. If a minimum bid amount has been established for any item, the unit or lump sum price
must equal or exceed the minimum amount stated.

1-02.7 Bid Deposit

(October 1, 2005 APWA GSP)

Supplement this section with the following:

Bid bonds shall contain the following:
1. Contracting Agency-assigned number for the project;
2. Name of the project;
3. The Contracting Agency named as obligee;
4. The amount of the bid bond stated either as a dollar figure or as a percentage which
represents five percent of the maximum bid amount that could be awarded;
5. Signature of the bidder's officer empowered to sign official statements. The signature of
the person authorized to submit the bid should agree with the signature on the bond, and the
title of the person must accompany the said signature;
6. The signature of the surety's officer empowered to sign the bond and the power of
attorney.

If so stated in the Contract Provisions, bidder must use the bond form included in the
1-02.9 Delivery of Proposal
(October 1, 2005 APWA GSP)

Revise the first paragraph to read:

Each proposal shall be submitted in a sealed envelope, with the Project Name and Project Number as stated in the Advertisement for Bids clearly marked on the outside of the envelope, or as otherwise stated in the Bid Documents, to ensure proper handling and delivery.

1-02.12 Public Opening of Proposal

Section 1-02.12 is supplemented with the following:

(*****)
Date of Opening Bids
Sealed bids are to be received at the following location prior to the time specified:

Yakima County Public Services, 4th floor Yakima County Courthouse, 128 N. 2nd Street, Yakima Washington, 98901, until 2:00 P.M. of the bid opening date.

The bid opening date for this project is August 19, 2009. Bids received will be publicly opened and read after 2:00 P.M. on this date.

1-02.13 Irregular Proposals
(March 25, 2009 APWA GSP)

Revise item 1 to read:

1. A proposal will be considered irregular and will be rejected if:
   a. The Bidder is not prequalified when so required;
   b. The authorized proposal form furnished by the Contracting Agency is not used or is altered;
   c. The completed proposal form contains any unauthorized additions, deletions, alternate Bids, or conditions;
   d. The Bidder adds provisions reserving the right to reject or accept the award, or enter into the Contract;
   e. A price per unit cannot be determined from the Bid Proposal;
   f. The Proposal form is not properly executed;
   g. The Bidder fails to submit or properly complete a Subcontractor list, if applicable, as required in Section 1-02.6;
   h. The Bidder fails to submit or properly complete a Disadvantaged, Minority or Women’s Business Enterprise Certification, if applicable, as required in Section 1-02.6;
   i. The Bid Proposal does not constitute a definite and unqualified offer to meet the material terms of the Bid invitation; or
j. More than one proposal is submitted for the same project from a Bidder under the same or different names.

1-02.14 Disqualification of Bidders
(March 25, 2009 APWA GSP, Option B)

Delete this Section and replace it with the following:

A Bidder will be deemed not responsible if:
1. the Bidder does not meet the mandatory bidder responsibility criteria in RCW 39.04.350(1), as amended; or
2. evidence of collusion exists with any other Bidder or potential Bidder. Participants in collusion will be restricted from submitting further bids; or
3. the Bidder, in the opinion of the Contracting Agency, is not qualified for the work or to the full extent of the bid, or to the extent that the bid exceeds the authorized prequalification amount as may have been determined by a prequalification of the Bidder; or
4. an unsatisfactory performance record exists based on past or current Contracting Agency work or for work done for others, as judged from the standpoint of conduct of the work; workmanship; or progress; affirmative action; equal employment opportunity practices; termination for cause; or Disadvantaged Business Enterprise, Minority Business Enterprise, or Women’s Business Enterprise utilization; or
5. there is uncompleted work (Contracting Agency or otherwise), which in the opinion of the Contracting Agency might hinder or prevent the prompt completion of the work bid upon; or
6. the Bidder failed to settle bills for labor or materials on past or current contracts, unless there are extenuating circumstances acceptable to the Contracting Agency; or
7. the Bidder has failed to complete a written public contract or has been convicted of a crime arising from a previous public contract, unless there are extenuating circumstances acceptable to the Contracting Agency; or
8. the Bidder is unable, financially or otherwise, to perform the work, in the opinion of the Contracting Agency; or
9. there are any other reasons deemed proper by the Contracting Agency.

As evidence that the Bidder meets the bidder responsibility criteria above, the apparent two lowest Bidders must submit to the Contracting Agency within 24 hours of the bid submittal deadline, documentation (sufficient in the sole judgment of the Contracting Agency) demonstrating compliance with all applicable responsibility criteria, including all documentation specifically listed in the supplemental criteria. The Contracting Agency reserves the right to request such documentation from other Bidders as well, and to request further documentation as needed to assess bidder responsibility.

The basis for evaluation of Bidder compliance with these supplemental criteria shall be any documents or facts obtained by Contracting Agency (whether from the Bidder or third
parties) which any reasonable owner would rely on for determining such compliance, including but not limited to: (i) financial, historical, or operational data from the Bidder; (ii) information obtained directly by the Contracting Agency from owners for whom the Bidder has worked, or other public agencies or private enterprises; and (iii) any additional information obtained by the Contracting Agency which is believed to be relevant to the matter.

If the Contracting Agency determines the Bidder does not meet the bidder responsibility criteria above and is therefore not a responsible Bidder, the Contracting Agency shall notify the Bidder in writing, with the reasons for its determination. If the Bidder disagrees with this determination, it may appeal the determination within 24 hours of receipt of the Contracting Agency’s determination by presenting its appeal to the Contracting Agency. The Contracting Agency will consider the appeal before issuing its final determination. If the final determination affirms that the Bidder is not responsible, the Contracting Agency will not execute a contract with any other Bidder until at least two business days after the Bidder determined to be not responsible has received the final determination.

1-02.15 Pre-Award Information
(October 1, 2005 APWA GSP)

Revise Section 1-02.15 to read:

Before awarding any contract, the Contracting Agency may require one or more of these items or actions of the apparent lowest responsible bidder:

1. A complete statement of the origin, composition, and manufacture of any or all materials to be used,

2. Samples of these materials for quality and fitness tests,

3. A progress schedule (in a form the Contracting Agency requires) showing the order of and time required for the various phases of the work,

4. A breakdown of costs assigned to any bid item,

5. Attendance at a conference with the Engineer or representatives of the Engineer,

6. Obtain, and furnish a copy of, a business license to do business in the city or county where the work is located.

7. A copy of State of Washington Contractor’s Registration, or

8. Any other information or action taken that is deemed necessary to ensure that the bidder is the lowest responsible bidder.
SECTION 1-03 AWARD AND EXECUTION OF CONTRACT

1-03.1 Consideration of Bids
(January 23, 2006 APWA GSP)

Revise the first paragraph to read:

After opening and reading proposals, the Contracting Agency will check them for correctness of extensions of the prices per unit and the total price. If a discrepancy exists between the price per unit and the extended amount of any bid item, the price per unit will control. If a minimum bid amount has been established for any item and the bidder's unit or lump sum price is less than the minimum specified amount, the Contracting Agency will unilaterally revise the unit or lump sum price to the minimum specified amount and recalculate the extension. The total of extensions, corrected where necessary, including sales taxes where applicable and such additives and/or alternates as selected by the Contracting Agency, will be used by the Contracting Agency for award purposes and to fix the Awarded Contract Price amount and the amount of the contract bond.

1-03.3 Execution of Contract
(October 1, 2005 APWA GSP)

Revise this section to read:

Copies of the Contract Provisions, including the unsigned Form of Contract, will be available for signature by the successful bidder on the first business day following award. The number of copies to be executed by the Contractor will be determined by the Contracting Agency.

Within 10 calendar days after the award date, the successful bidder shall return the signed Contracting Agency-prepared contract, an insurance certification as required by Section 1-07.18, and a satisfactory bond as required by law and Section 1-03.4. Before execution of the contract by the Contracting Agency, the successful bidder shall provide any pre-award information the Contracting Agency may require under Section 1-02.15.

Until the Contracting Agency executes a contract, no proposal shall bind the Contracting Agency nor shall any work begin within the project limits or within Contracting Agency-furnished sites. The Contractor shall bear all risks for any work begun outside such areas and for any materials ordered before the contract is executed by the Contracting Agency.
If the bidder experiences circumstances beyond their control that prevents return of the contract documents within the calendar days after the award date stated above, the Contracting Agency may grant up to a maximum of 10 additional calendar days for return of the documents, provided the Contracting Agency deems the circumstances warrant it.
1-03.4 Contract Bond
(October 1, 2005 APWA GSP)

Revise the first paragraph to read:

The successful bidder shall provide an executed contract bond for the full contract amount. This contract bond shall:

1. Be on a Contracting Agency-furnished form;
2. Be signed by an approved surety (or sureties) that:
   a. Is registered with the Washington State Insurance Commissioner, and
   b. Appears on the current Authorized Insurance List in the State of Washington published by the Office of the Insurance Commissioner,
3. Be conditioned upon the faithful performance of the contract by the Contractor within the prescribed time;
4. Guarantee that the surety shall indemnify, defend, and protect the Contracting Agency against any claim of direct or indirect loss resulting from the failure:
   a. Of the Contractor (or any of the employees, subcontractors, or lower tier subcontractors of the Contractor) to faithfully perform the contract, or
   b. Of the Contractor (or the subcontractors or lower tier subcontractors of the Contractor) to pay all laborers, mechanics, subcontractors, lower tier subcontractors, materialperson, or any other person who provides supplies or provisions for carrying out the work;
5. Be accompanied by a power of attorney for the Surety’s officer empowered to sign the bond; and
6. Be signed by an officer of the Contractor empowered to sign official statements (sole proprietor or partner). If the Contractor is a corporation, the bond must be signed by the president or vice-president, unless accompanied by written proof of the authority of the individual signing the bond to bind the corporation (i.e., corporate resolution, power of attorney or a letter to such effect by the president or vice-president).

SECTION 1-04 SCOPE OF THE WORK

1-04.2 Coordination of Contract Documents, Plans, Special Provisions, Specifications, and Addenda
(October 1, 2005 APWA GSP)

Revise the second paragraph to read:

Any inconsistency in the parts of the contract shall be resolved by following this order of precedence (e.g., 1 presiding over 2, 2 over 3, 3 over 4, and so forth):

1. Addenda,
2. Proposal Form,
3. Special Provisions, including APWA General Special Provisions, if they are included,
4. Contract Plans,
5. Amendments to the Standard Specifications,
6. WSDOT/APWA Standard Specifications for Road, Bridge and Municipal Construction,
7. Contracting Agency’s Standard Plans (if any), and
8. WSDOT/APWA Standard Plans for Road, Bridge, and Municipal Construction.

1-04.6 Variation in Estimated Quantities
(May 25, 2006 APWA GSP, Option B; may not be used on FHWA-funded projects)

Delete the first paragraph, and replace it with the following:

Payment to the Contractor will be made only for the actual quantities of work performed and
accepted in conformance with the contract. When the accepted quantity of work performed
under a unit item varies from the original proposal quantity, payment will be at the unit
contract price for all work unless the total accepted quantity of any contract item, adjusted to
exclude added or deleted amounts included in change orders accepted by both parties,
increases or decreases by more than 25 percent from the original proposal quantity, and if the
total extended bid price for that item at time of award is equal to or greater than 10 percent of
the total contract price at time of award. In that case, payment for contract work may be
adjusted as described herein:

SECTION 1-05 CONTROL OF WORK

1-05.4 Conformity With and Deviations From Plans and Stakes

Section 1-05.4 is supplemented with the following:

(******)

Contractor Surveying

Yakima County has established primary survey control data for this project. Copies of
the Contracting Agency provided primary survey control data are available for the
bidder's inspection at the office of the Project Engineer.

The Contractor shall be responsible for setting, maintaining, and resetting all alignment
stakes, slope stakes, and grades necessary for the construction. Except for the survey
control data to be furnished by the Contracting Agency, calculations, surveying, and
measuring required for setting and maintaining the necessary lines and grades shall be the
Contractor's responsibility.

Detailed survey records shall be maintained, including a description of the work
performed on each shift, the methods utilized, and the control points used. The record
shall be adequate to allow the survey to be reproduced. A copy of each day's record shall
be provided to the Engineer within three working days after the end of the shift.

The meaning of words and terms used in this provision shall be as listed in "Definitions
of Surveying and Associated Terms" current edition, published by the American Congress
on Surveying and Mapping and the American Society of Civil Engineers.
The survey work shall include but not be limited to the following:

1. Verify the primary horizontal and vertical control furnished by the Contracting Agency, and expand into secondary control by adding stakes and hubs as well as additional survey control needed for the project. Provide descriptions of secondary control to the Contracting Agency. The description shall include coordinates and elevations of all secondary control points.

2. Establish, the centerlines of all alignments, by placing hubs, stakes, or marks on centerline or on offsets to centerline at all curve points (PCs, PTs, and PIs) and at points on the alignments spaced no further than 50 feet.

3. Establish clearing limits, placing stakes at all angle points and at intermediate points not more than 50 feet apart. The clearing and grubbing limits shall be 5 feet beyond the toe of a fill and 10 feet beyond the top of a cut unless otherwise shown in the Plans.

4. Establish grading limits, placing slope stakes at centerline increments not more than 50 feet apart. Establish offset reference to all slope stakes.

5. Establish the horizontal and vertical location of all drainage features, placing offset stakes to all drainage structures and to pipes at a horizontal interval not greater than 25 feet.

6. Establish roadbed and surfacing elevations by placing stakes at the top of subgrade and at the top of each course of surfacing. Subgrade and surfacing stakes shall be set at horizontal intervals not greater than 50 feet in tangent sections, 25 feet in curve sections with a radius less than 300 feet, and at 10-foot intervals in intersection radii with a radius less than 10 feet. Transversely, stakes shall be placed at all locations where the slope changes and at additional points such that the transverse spacing of stakes is not more than 12 feet.

7. Establish intermediate elevation benchmarks as needed to check work throughout the project.

8. For all other types of construction included in this provision, provide staking and layout as necessary to adequately locate, construct, and check the specific construction activity.

The Contractor shall provide the Contracting Agency copies of any calculations and staking data when requested by the Engineer.

To facilitate the establishment of these lines and elevations, the Contracting Agency will provide the Contractor with primary survey control information consisting of descriptions of two primary control points used for the horizontal and vertical control. Primary
control points will be described by reference to the project alignment and the coordinate system and elevation datum utilized by the project.

The Contractor shall ensure a surveying accuracy within the following tolerances:

<table>
<thead>
<tr>
<th></th>
<th>Vertical</th>
<th>Horizontal</th>
</tr>
</thead>
<tbody>
<tr>
<td>Slope stakes</td>
<td>±0.10 feet</td>
<td>±0.10 feet</td>
</tr>
<tr>
<td>Subgrade grade stakes set 0.04 feet below grade</td>
<td>±0.01 feet</td>
<td>±0.5 feet (parallel to alignment) ±0.1 feet (normal to alignment)</td>
</tr>
<tr>
<td>Stationing on roadway</td>
<td>N/A</td>
<td>±0.1 feet</td>
</tr>
<tr>
<td>Alignment on roadway</td>
<td>N/A</td>
<td>±0.04 feet</td>
</tr>
<tr>
<td>Surfacing grade stakes</td>
<td>±0.01 feet</td>
<td>±0.5 feet (parallel to alignment) ±0.1 feet (normal to alignment)</td>
</tr>
<tr>
<td>Roadway paving pins for surfacing or paving</td>
<td>±0.01 feet</td>
<td>±0.2 feet (parallel to alignment) ±0.1 feet (normal to alignment)</td>
</tr>
</tbody>
</table>

The Contracting Agency may spot-check the Contractor's surveying. These spot-checks will not change the requirements for normal checking by the Contractor.

Contract work to be performed using contractor-provided stakes shall not begin until the stakes are approved by the Contracting Agency. Such approval shall not relieve the Contractor of responsibility for the accuracy of the stakes.

Stakes shall be marked in accordance with Standard Plan A-10.10-00. When stakes are needed that are not described in the Plans, then those stakes shall be marked, at no additional cost to the Contracting Agency as ordered by the Engineer.

Payment
No separate payment will be made for surveying. Costs for surveying shall be considered incidental to other bid items.
1-05.7 Removal of Defective and Unauthorized Work

(October 1, 2005 APWA GSP)

Section 1-05.7 is supplemented with the following:

If the Contractor fails to remedy defective or unauthorized work within the time specified in a written notice from the Engineer, or fails to perform any part of the work required by the Contract Documents, the Engineer may correct and remedy such work as may be identified in the written notice, with Contracting Agency forces or by such other means as the Contracting Agency may deem necessary.

If the Contractor fails to comply with a written order to remedy what the Engineer determines to be an emergency situation, the Engineer may have the defective and unauthorized work corrected immediately, have the rejected work removed and replaced, or have work the Contractor refuses to perform completed by using Contracting Agency or other forces. An emergency situation is any situation when, in the opinion of the Engineer, a delay in its remedy could be potentially unsafe, or might cause serious risk of loss or damage to the public.

Direct or indirect costs incurred by the Contracting Agency attributable to correcting and remediating defective or unauthorized work, or work the Contractor failed or refused to perform, shall be paid by the Contractor. Payment will be deducted by the Engineer from monies due, or to become due, the Contractor. Such direct and indirect costs shall include in particular, but without limitation, compensation for additional professional services required, and costs for repair and replacement of work of others destroyed or damaged by correction, removal, or replacement of the Contractor’s unauthorized work.

No adjustment in contract time or compensation will be allowed because of the delay in the performance of the work attributable to the exercise of the Contracting Agency’s rights provided by this Section.

The rights exercised under the provisions of this section shall not diminish the Contracting Agency’s right to pursue any other avenue for additional remedy or damages with respect to the Contractor’s failure to perform the work as required.

1-05.11 Final Inspection

(October 1, 2005 APWA GSP)

Delete Section 1-05.11 and replace it with the following:

Final Inspections and Operational Testing

Substantial Completion Date

When the Contractor considers the work to be substantially complete, the Contractor shall so notify the Engineer and request the Engineer establish the Substantial Completion Date. The
Contractor’s request shall list the specific items of work that remain to be completed in order to reach physical completion. The Engineer will schedule an inspection of the work with the Contractor to determine the status of completion. The Engineer may also establish the Substantial Completion Date unilaterally.

If, after this inspection, the Engineer concurs with the Contractor that the work is substantially complete and ready for its intended use, the Engineer, by written notice to the Contractor, will set the Substantial Completion Date. If, after this inspection the Engineer does not consider the work substantially complete and ready for its intended use, the Engineer will, by written notice, so notify the Contractor giving the reasons therefore.

Upon receipt of written notice concurring in or denying substantial completion, whichever is applicable, the Contractor shall pursue vigorously, diligently and without unauthorized interruption, the work necessary to reach Substantial and Physical Completion. The Contractor shall provide the Engineer with a revised schedule indicating when the Contractor expects to reach substantial and physical completion of the work.

The above process shall be repeated until the Engineer establishes the Substantial Completion Date and the Contractor considers the work physically complete and ready for final inspection.

**Final Inspection and Physical Completion Date**

When the Contractor considers the work physically complete and ready for final inspection, the Contractor by written notice, shall request the Engineer to schedule a final inspection. The Engineer will set a date for final inspection. The Engineer and the Contractor will then make a final inspection and the Engineer will notify the Contractor in writing of all particulars in which the final inspection reveals the work incomplete or unacceptable. The Contractor shall immediately take such corrective measures as are necessary to remedy the listed deficiencies. Corrective work shall be pursued vigorously, diligently, and without interruption until physical completion of the listed deficiencies. This process will continue until the Engineer is satisfied the listed deficiencies have been corrected.

If action to correct the listed deficiencies is not initiated within 7 days after receipt of the written notice listing the deficiencies, the Engineer may, upon written notice to the Contractor, take whatever steps are necessary to correct those deficiencies pursuant to Section 1-05.7.

The Contractor will not be allowed an extension of contract time because of a delay in the performance of the work attributable to the exercise of the Engineer’s right hereunder.

Upon correction of all deficiencies, the Engineer will notify the Contractor and the Contracting Agency, in writing, of the date upon which the work was considered physically complete. That date shall constitute the Physical Completion Date of the contract, but shall not imply acceptance of the work or that all the obligations of the Contractor under the contract have been fulfilled.
Operational Testing

It is the intent of the Contracting Agency to have at the Physical Completion Date a complete and operable system. Therefore when the work involves the installation of machinery or other mechanical equipment; street lighting, electrical distribution or signal systems; irrigation systems; buildings; or other similar work it may be desirable for the Engineer to have the Contractor operate and test the work for a period of time after final inspection but prior to the physical completion date. Whenever items of work are listed in the Contract Provisions for operational testing they shall be fully tested under operating conditions for the time period specified to ensure their acceptability prior to the Physical Completion Date. During and following the test period, the Contractor shall correct any items of workmanship, materials, or equipment which prove faulty, or that are not in first class operating condition. Equipment, electrical controls, meters, or other devices and equipment to be tested during this period shall be tested under the observation of the Engineer, so that the Engineer may determine their suitability for the purpose for which they were installed. The Physical Completion Date cannot be established until testing and corrections have been completed to the satisfaction of the Engineer.

The costs for power, gas, labor, material, supplies, and everything else needed to successfully complete operational testing, shall be included in the unit contract prices related to the system being tested, unless specifically set forth otherwise in the proposal.

Operational and test periods, when required by the Engineer, shall not affect a manufacturer’s guaranties or warranties furnished under the terms of the contract.

1-05.13 Superintendents, Labor and Equipment of Contractor

(March 25, 2009 APWA GSP)

Revise the seventh paragraph to read:

Whenever the Contracting Agency evaluates the Contractor’s qualifications pursuant to Section 1-02.14, it will take these performance reports into account.

1-05.14 Cooperation With other Contractors

(March 13, 1995)

Cooperation With Other Contractors
Section 1-05.14 is supplemented with the following:

Other Contracts Or Other Work
It is anticipated that the following work adjacent to or within the limits of this project will be performed by others during the course of this project and will require coordination of the work:
1. Utility Work.

No additional payment will be made for this utility coordination work and all costs shall be incidental to the unit contract prices and no further payment shall be made.

Add the following new section:

1-05.16 Water and Power

(October 1, 2005 APWA GSP)

The Contractor shall make necessary arrangements, and shall bear the costs for power and water necessary for the performance of the work, unless the contract includes power and water as a pay item.

Add the following new section:

1-05.17 Oral Agreements

(October 1, 2005 AWPA GSP)

No oral agreement or conversation with any officer, agent, or employee of the Contracting Agency, either before or after execution of the contract, shall affect or modify any of the terms or obligations contained in any of the documents comprising the contract. Such oral agreement or conversation shall be considered as unofficial information and in no way binding upon the Contracting Agency, unless subsequently put in writing and signed by the Contracting Agency.

SECTION 1-06 CONTROL OF MATERIAL

1-06 Buy America

Section 1-06 is supplemented with the following:

(August 6, 2007)

The major quantities of steel and iron construction material that is permanently incorporated into the project shall consist of American-made materials only. Buy America does not apply to temporary steel items, e.g., temporary sheet piling, temporary bridges, steel scaffolding and falsework.

The Contractor may utilize minor amounts of foreign steel and iron in this project provided the cost of the foreign material used does not exceed one-tenth of one percent of the total contract cost or $2,500.00, whichever is greater.

American-made material is defined as material having all manufacturing processes occurring domestically. To further define the coverage, a domestic product is a
manufactured steel material that was produced in one of the 50 States, the District of Columbia, Puerto Rico, or in the territories and possessions of the United States. If domestically produced steel billets or iron ingots are exported outside of the area of coverage, as defined above, for any manufacturing process then the resulting product does not conform to the Buy America requirements. Additionally, products manufactured domestically from foreign source steel billets or iron ingots do not conform to the Buy America requirements because the initial melting and mixing of alloys to create the material occurred in a foreign country.

Manufacturing begins with the initial melting and mixing, and continues through the coating stage. Any process which modifies the chemical content, the physical size or shape, or the final finish is considered a manufacturing process. The processes include rolling, extruding, machining, bending, grinding, drilling, welding, and coating. The action of applying a coating to steel or iron is deemed a manufacturing process. Coating includes epoxy coating, galvanizing, aluminizing, painting, and any other coating that protects or enhances the value of steel or iron. Any process from the original reduction from ore to the finished product constitutes a manufacturing process for iron.

Due to a nationwide waiver, Buy America does not apply to raw materials (iron ore and alloys), scrap (recycled steel or iron), and pig iron or processed, pelletized, and reduced iron ore.

The following are considered to be steel manufacturing processes:

1. Production of steel by any of the following processes:
   a. Open hearth furnace.
   b. Basic oxygen.
   c. Electric furnace.
   d. Direct reduction.

2. Rolling, heat treating, and any other similar processing.

3. Fabrication of the products.
   a. Spinning wire into cable or strand.
   b. Corrugating and rolling into culverts.
   c. Shop fabrication.

A certification of materials origin will be required for any items comprised of, or containing, steel or iron construction materials prior to such items being incorporated into the permanent
work. The certification shall be on DOT Form 350-109EF provided by the Engineer, or such other form the Contractor chooses, provided it contains the same information as DOT Form 350-109EF.

1-06.2(2) Statistical Evaluation of Materials for Acceptance

(******)
Section 1-06.2(2) of the Standard Specifications is deleted.

SECTION 1-07 LEGAL RELATIONS AND RESPONSIBILITIES TO THE PUBLIC

1-07.1 Laws to be Observed
(October 1, 2005 APWA GSP)

Section 1-07.1 is supplemented with the following:

In cases of conflict between different safety regulations, the more stringent regulation shall apply.

The Washington State Department of Labor and Industries shall be the sole and paramount administrative agency responsible for the administration of the provisions of the Washington Industrial Safety and Health Act of 1973 (WISHA).

The Contractor shall maintain at the project site office, or other well known place at the project site, all articles necessary for providing first aid to the injured. The Contractor shall establish, publish, and make known to all employees, procedures for ensuring immediate removal to a hospital, or doctor’s care, persons, including employees, who may have been injured on the project site. Employees should not be permitted to work on the project site before the Contractor has established and made known procedures for removal of injured persons to a hospital or a doctor’s care.

The Contractor shall have sole responsibility for the safety, efficiency, and adequacy of the Contractor’s plant, appliances, and methods, and for any damage or injury resulting from their failure, or improper maintenance, use, or operation. The Contractor shall be solely and completely responsible for the conditions of the project site, including safety for all persons and property in the performance of the work. This requirement shall apply continuously, and not be limited to normal working hours. The required or implied duty of the Engineer to conduct construction review of the Contractor’s performance does not, and shall not, be intended to include review and adequacy of the Contractor’s safety measures in, on, or near the project site.
1-07.2 State Sales Tax

Delete Section 1-07.2, including its sub-sections, in its entirety and replace it with the following:

1-07.2 State Sales Tax
(October 1, 2005 APWA GSP)

1-07.2(1) General

The Washington State Department of Revenue has issued special rules on the State sales tax. Sections 1-07.2(1) through 1-07.2(4) are meant to clarify those rules. The Contractor should contact the Washington State Department of Revenue for answers to questions in this area. The Contracting Agency will not adjust its payment if the Contractor bases a bid on a misunderstood tax liability.

The Contractor shall include all Contractor-paid taxes in the unit bid prices or other contract amounts. In some cases, however, state retail sales tax will not be included. Section 1-07.2(3) describes this exception.

The Contracting Agency will pay the retained percentage only if the Contractor has obtained from the Washington State Department of Revenue a certificate showing that all contract-related taxes have been paid (RCW 60.28.050). The Contracting Agency may deduct from its payments to the Contractor any amount the Contractor may owe the Washington State Department of Revenue, whether the amount owed relates to this contract or not. Any amount so deducted will be paid into the proper State fund.

1-07.2(2) State Sales Tax — Rule 171

WAC 458-20-171, and its related rules, apply to building, repairing, or improving streets, roads, etc., which are owned by a municipal corporation, or political subdivision of the state, or by the United States, and which are used primarily for foot or vehicular traffic. This includes storm or combined sewer systems within and included as a part of the street or road drainage system and power lines when such are part of the roadway lighting system. For work performed in such cases, the Contractor shall include Washington State Retail Sales Taxes in the various unit bid item prices, or other contract amounts, including those that the Contractor pays on the purchase of the materials, equipment, or supplies used or consumed in doing the work.

1-07.2(3) State Sales Tax — Rule 170

WAC 458-20-170, and its related rules, apply to the constructing and repairing of new or existing buildings, or other structures, upon real property. This includes, but is not limited to, the construction of streets, roads, highways, etc., owned by the state of Washington; water mains and their appurtenances; sanitary sewers and sewage disposal systems unless such sewers and disposal systems are within, and a part of, a street or road drainage system; telephone, telegraph, electrical power distribution lines, or other conduits or lines in or above
streets or roads, unless such power lines become a part of a street or road lighting system; and installing or attaching of any article of tangible personal property in or to real property, whether or not such personal property becomes a part of the realty by virtue of installation.

For work performed in such cases, the Contractor shall collect from the Contracting Agency, retail sales tax on the full contract price. The Contracting Agency will automatically add this sales tax to each payment to the Contractor. For this reason, the Contractor shall not include the retail sales tax in the unit bid item prices, or in any other contract amount subject to Rule 170, with the following exception.

Exception: The Contracting Agency will not add in sales tax for a payment the Contractor or a subcontractor makes on the purchase or rental of tools, machinery, equipment, or consumable supplies not integrated into the project. Such sales taxes shall be included in the unit bid item prices or in any other contract amount.

1-07.2(4) Services

The Contractor shall not collect retail sales tax from the Contracting Agency on any contract wholly for professional or other services (as defined in Washington State Department of Revenue Rules 138 and 244).

1-07.6 Permits and Licenses

Section 1-07.6 is supplemented with the following:

(******)

Department of Ecology

1. The Contractor shall procure all permits, licenses, and certificates that may be required by law or regulation for the execution of the work described in this document. The Contractor shall comply with all federal, state and local laws, ordinances, rules, and regulations relating to the performance of the work described in this document.

2. The successful bidder shall submit start cards to the Department of Ecology prior to well decommission as specified in WAC 173-160-055 “Well Construction Notification (Start Card)”. At the completion of the project, the Contractor shall complete and submit to the Department of Ecology all necessary reports and forms relating to the decommissioning of the existing well (WAC 173-160-050 “Records”).

(******)

Department of Labor and Industries

1. The Contractor shall procure all permits, licenses, and certificates that may be required by law or regulation for the execution of the work described in this document. The Contractor shall comply with all federal, state and local laws,
ordinances, rules, and regulations relating to the performance of the work described in this document.

2. The successful bidder shall apply for an electrical permit with the Department of Labor and Industries and comply with all regulations associated there with.

1-07.13 Contractor’s Responsibility for Work

1-07.13(4) Repair of Damage

Section 1-07.6 is supplemented with the following:

(******)

1. The Contractor shall conduct all operations in such a way as to prevent destruction, scarring, or defacing of the work site. Every effort shall be made to present an orderly appearance at the work site during construction activities. If contaminated soil or refuse is encountered, it shall be wrapped in plastic sheeting or drummed for later disposal by the Owner. No additional compensation will be made for this work.

2. In the event that any damages to the property occur because of the Contractor’s activities, the Contractor shall, at his own expense, repair the damages or reclaim the land to the extent deemed suitable to the Owner.

1-07.17 Utilities and Similar Facilities

(April 2, 2007)

Section 1-07.17 is supplemented with the following:

Locations and dimensions shown in the Plans for existing facilities are in accordance with available information obtained without uncovering, measuring, or other verification.

Public and private utilities, or their Contractors, will furnish all work necessary to adjust, relocate, replace, or construct their facilities unless otherwise provided for in the Plans or these Special Provisions. Such adjustment, relocation, replacement, or construction will be done during the prosecution of the work for this project. It is anticipated that utility adjustment, relocation, replacement or construction within the project limits will be completed as follows:

The following addresses and telephone numbers of utility companies or their Contractors that will be adjusting, relocating, replacing or constructing utilities within the project limits are supplied for the Contractor’s use:

Call Before You Dig One Call Center 1-800-424-5555
Pacific Power & Light Co., 500 N. Keys Road, Yakima, WA 98901 (509) 575-3158
Embarq Telephone (509) 839-6660
Roza Irrigation, 125 S. Thirteenth St., Sunnyside, WA 98944 (509) 836-2248
1-07.18 Public Liability and Property Damage Insurance

Delete this section in its entirety, and replace it with the following:

1-07.18 Insurance

(May 10, 2006 APWA GSP)

1-07.18(1) General Requirements

A. The Contractor shall obtain the insurance described in this section from insurers approved by the State Insurance Commissioner pursuant to RCW Title 48. The insurance must be provided by an insurer with a rating of A-: VII or higher in the A.M. Best’s Key Rating Guide, which is licensed to do business in the state of Washington (or issued as a surplus line by a Washington Surplus lines broker). The Contracting Agency reserves the right to approve or reject the insurance provided, based on the insurer (including financial condition), terms and coverage, the Certificate of Insurance, and/or endorsements.

B. The Contractor shall keep this insurance in force during the term of the contract and for thirty (30) days after the Physical Completion date, unless otherwise indicated (see C. below).

C. If any insurance policy is written on a claims made form, its retroactive date, and that of all subsequent renewals, shall be no later than the effective date of this Contract. The policy shall state that coverage is claims made, and state the retroactive date. Claims-made form coverage shall be maintained by the Contractor for a minimum of 36 months following the Final Completion or earlier termination of this contract, and the Contractor shall annually provide the Contracting Agency with proof of renewal. If renewal of the claims made form of coverage becomes unavailable, or economically prohibitive, the Contractor shall purchase an extended reporting period (“tail”) or execute another form of guarantee acceptable to the Contracting Agency to assure financial responsibility for liability for services performed.

D. The insurance polices shall contain a “cross liability” provision.

E. The Contractor’s and all subcontractors’ insurance coverage shall be primary and non-contributory insurance as respects the Contracting Agency’s insurance, self-insurance, or insurance pool coverage.

F. All insurance policies and Certificates of Insurance shall include a requirement providing for a minimum of 30 days prior written notice to the Contracting Agency of any cancellation in any insurance policy.

G. Upon request, the Contractor shall forward to the Contracting Agency a full and certified copy of the insurance policy(s).

H. The Contractor shall not begin work under the contract until the required insurance has been obtained and approved by the Contracting Agency.
I. Failure on the part of the Contractor to maintain the insurance as required shall constitute a material breach of contract, upon which the Contracting Agency may, after giving five business days notice to the Contractor to correct the breach, immediately terminate the contract or, at its discretion, procure or renew such insurance and pay any and all premiums in connection therewith, with any sums so expended to be repaid to the Contracting Agency on demand, or at the sole discretion of the Contracting Agency, offset against funds due the Contractor from the Contracting Agency.

J. All costs for insurance shall be incidental to and included in the unit or lump sum prices of the contract and no additional payment will be made.

1-07.18(2) Additional Insured

All insurance policies, with the exception of Professional Liability and Workers Compensation, shall name the following listed entities as additional insured(s):

Yakima County and its officers, elected officials, employees, consultants, agents, and volunteers.

The above-listed entities shall be additional insured(s) for the full available limits of liability maintained by the Contractor, whether primary, excess, contingent or otherwise, irrespective of whether such limits maintained by the Contractor are greater than those required by this Contract, and irrespective of whether the Certificate of Insurance provided by the Contractor pursuant to 1-07.18(3) describes limits lower than those maintained by the Contractor.

1-07.18(3) Subcontractors

Contractor shall ensure that each subcontractor of every tier obtains and maintains at a minimum the insurance coverage’s listed in 1-07.18(5A) and 1-07.18(5B). Upon request of the Contracting Agency, the Contractor shall provide evidence of such insurance.

1-07.18(4) Evidence of Insurance

The Contractor shall deliver to the Contracting Agency a Certificate(s) of Insurance and endorsements for each policy of insurance meeting the requirements set forth herein when the Contractor delivers the signed Contract for the work. The certificate and endorsements must conform to the following requirements:

1. An ACORD certificate or a form determined by the Contracting Agency to be equivalent.

2. Copies of all endorsements naming Contracting Agency and all other entities listed in 1-07.18(2) as Additional Insured(s), showing the policy number. The Contractor may submit a copy of any blanket additional insured clause from its policies instead of a separate endorsement. A statement of additional insured status on an ACORD Certificate of Insurance shall not satisfy this requirement.

3. Any other amendatory endorsements to show the coverage required herein.
1-07.18(5) Coverages and Limits

The insurance shall provide the minimum coverages and limits set forth below. Providing coverage in these stated minimum limits shall not be construed to relieve the Contractor from liability in excess of such limits. All deductibles and self-insured retentions must be disclosed and are subject to approval by the Contracting Agency. The cost of any claim payments falling within the deductible shall be the responsibility of the Contractor.

1-07.18(5)A Commercial General Liability

A policy of Commercial General Liability Insurance, including:

- Per project aggregate
- Premises/Operations Liability
- Products/Completed Operations – for a period of one year following final acceptance of the work.
- Personal/Advertising Injury
- Contractual Liability
- Independent Contractors Liability
- Stop Gap / Employers’ Liability
- Explosion, Collapse, or Underground Property Damage (XCU)
- Blasting (only required when the Contractor’s work under this Contract includes exposures to which this specified coverage responds)

Such policy must provide the following minimum limits:

- $1,000,000 Each Occurrence
- $2,000,000 General Aggregate
- $1,000,000 Products & Completed Operations Aggregate
- $1,000,000 Personal & Advertising Injury, each offence

Stop Gap / Employers’ Liability

- $1,000,000 Each Accident
- $1,000,000 Disease - Policy Limit
- $1,000,000 Disease - Each Employee

1-07.18(5)B Automobile Liability

Automobile Liability for owned, non-owned, hired, and leased vehicles, with an MCS 90 endorsement and a CA 9948 endorsement attached if “pollutants” are to be transported. Such policy(ies) must provide the following minimum limit:

- $1,000,000 combined single limit
1-07.18(5)C Workers’ Compensation

The Contractor shall comply with Workers’ Compensation coverage as required by the Industrial Insurance laws of the state of Washington.

1-07.18(5)E All Risk Builder’s Risk

(May 10, 2006 APWA GSP)

Contractor shall purchase and maintain Builders Risk insurance covering interests of the Contracting Agency, the Contractor, Subcontractors, and Sub-subcontractors in the work. Builders Risk insurance shall be on a all-risk policy form and shall insure against the perils of fire and extended coverage and physical loss or damage including flood, earthquake, theft, vandalism, malicious mischief and collapse. The Builders Risk insurance shall include coverage for temporary buildings, debris removal, and damage to materials in transit or stored off-site. Such insurance shall cover “soft costs” including but not limited to design costs, licensing fees, and architect’s and engineer’s fees. Builders Risk insurance shall be written in the amount of the completed value of the project, with no coinsurance provisions.

The Builders Risk insurance covering the work shall have a deductible of $5,000 for each occurrence, which will be the responsibility of the Contractor. Higher deductibles for flood, earthquake and all other perils may be accepted by the Contracting Agency upon written request by the Contractor and written acceptance by the Contracting Agency. Any increased deductibles accepted by the Contracting Agency will remain the responsibility of the Contractor.

The Builders Risk insurance shall be maintained until final acceptance of the work by the Contracting Agency.

The Contractor and the Contracting Agency waive all rights against each other any of their Subcontractors, Sub-subcontractors, agents and employees, each of the other, for damages caused by fire or other perils to the extent covered by Builders Risk insurance or other property insurance applicable to the work. The policies shall provide such waivers by endorsement or otherwise.

1-07.18(5)G Pollution Liability

(May 10, 2006 APWA GSP)

The Contractor shall provide a Pollution Liability policy, providing coverage for claims involving bodily injury, property damage (including loss of use of tangible property that has not been physically injured), cleanup costs, remediation, disposal or other handling of pollutants, including costs and expenses incurred in the investigation, defense, or settlement of claims arising out of:

- Contractor’s operations related to this project; and/or
- Remediation, abatement, repair, maintenance or other work with lead-based paint or materials containing asbestos; and/or
- Transportation of hazardous materials away from any site related to this project.
Such Pollution Liability policy shall provide the following minimum coverage:

$1,000,000  each loss and annual aggregate

SECTION 1-08 PROSECUTION AND PROGRESS

1-08  Prosecution and Progress
Add the following new section:

1-08.0  Preliminary Matters
(May 25, 2006 APWA GSP)

Add the following new section:

1-08.0(1)  Preconstruction Conference
(October 10, 2008 APWA GSP)

Prior to the Contractor beginning the work, a preconstruction conference will be held
between the Contractor, the Engineer and such other interested parties as may be invited.
The purpose of the preconstruction conference will be:

1.  To review the initial progress schedule;
2.  To establish a working understanding among the various parties associated or affected by
the work;
3.  To establish and review procedures for progress payment, notifications, approvals,
  submittals, etc.;
4.  To establish normal working hours for the work;
5.  To review safety standards and traffic control; and
6.  To discuss such other related items as may be pertinent to the work.

The Contractor shall prepare and submit at the preconstruction conference the following:

1.  A breakdown of all lump sum items;
2.  A preliminary schedule of working drawing submittals; and
3.  A list of material sources for approval if applicable.

Add the following new section:

1-08.0(2)  Hours of Work
(May 25, 2006 APWA GSP)

Except in the case of emergency or unless otherwise approved by the Contracting Agency,
the normal straight time working hours for the contract shall be any consecutive 8-hour
period between 7:00 a.m. and 6:00 p.m. of a working day with a maximum 1-hour lunch
break and a 5-day work week. The normal straight time 8-hour working period for the
contract shall be established at the preconstruction conference or prior to the Contractor
commencing the work.
If a Contractor desires to perform work on holidays, Saturdays, Sundays, or before 7:00 a.m. or after 6:00 p.m. on any day, the Contractor shall apply in writing to the Engineer for permission to work such times. Permission to work longer than an 8-hour period between 7:00 a.m. and 6:00 p.m. is not required. Such requests shall be submitted to the Engineer no later than noon on the working day prior to the day for which the Contractor is requesting permission to work.

Permission to work between the hours of 10:00 p.m. and 7:00 a.m. during weekdays and between the hours of 10:00 p.m. and 9:00 a.m. on weekends or holidays may also be subject to noise control requirements. Approval to continue work during these hours may be revoked at any time the Contractor exceeds the Contracting Agency’s noise control regulations or complaints are received from the public or adjoining property owners regarding the noise from the Contractor’s operations. The Contractor shall have no claim for damages or delays should such permission be revoked for these reasons.

Permission to work Saturdays, Sundays, holidays or other than the agreed upon normal straight time working hours Monday through Friday may be given subject to certain other conditions set forth by the Contracting Agency or Engineer. These conditions may include but are not limited to: requiring the Engineer or such assistants as the Engineer may deem necessary to be present during the work; requiring the Contractor to reimburse the Contracting Agency for the costs in excess of straight-time costs for Contracting Agency employees who worked during such times, on non Federal aid projects; considering the work performed on Saturdays, Sundays, and holidays as working days with regards to the contract time; and considering multiple work shifts as multiple working days with respect to contract time even though the multiple shifts occur in a single 24-hour period. Assistants may include, but are not limited to, survey crews; personnel from the Contracting Agency’s material testing lab; inspectors; and other Contracting Agency employees when in the opinion of the Engineer, such work necessitates their presence.

1-08.4 Notice to Proceed and Prosecution of the Work
(October 1, 2005 APWA GSP)

Revise Section 1-08.4 to read:

Notice to Proceed will be given after the contract has been executed and the contract bond and evidence of insurance have been approved and filed by the Contracting Agency. The Contractor shall not commence with the work until the Notice to Proceed has been given by the Engineer. The Contractor shall commence construction activities on the project site within ten days of the Notice to Proceed Date, unless otherwise approved in writing. The Contractor shall diligently pursue the work to the physical completion date within the time specified in the contract. Voluntary shutdown or slowing of operations by the Contractor shall not relieve the Contractor of the responsibility to complete the work within the time(s) specified in the contract.
1-08.5 Time For Completion
(June 28, 2007 APWA GSP, Option A)

Revise the third and fourth paragraphs in Section 1-08.5 to read:

Contract time shall begin on the first working day following the Notice to Proceed Date.

Each working day shall be charged to the contract as it occurs, until the contract work is physically complete. If substantial completion has been granted and all the authorized working days have been used, charging of working days will cease. Each week the Engineer will provide the Contractor a statement that shows the number of working days: (1) charged to the contract the week before; (2) specified for the physical completion of the contract; and (3) remaining for the physical completion of the contract. The statement will also show the nonworking days and any partial or whole day the Engineer declares as unworkable. Within 10 calendar days after the date of each statement, the Contractor shall file a written protest of any alleged discrepancies in it. To be considered by the Engineer, the protest shall be in sufficient detail to enable the Engineer to ascertain the basis and amount of time disputed. By not filing such detailed protest in that period, the Contractor shall be deemed as having accepted the statement as correct. If the Contractor elects to work 10 hours a day and 4 days a week (a 4-10 schedule) and the fifth day of the week in which a 4-10 shift is worked would ordinarily be charged as a working day then the fifth day of that week will be charged as a working day whether or not the Contractor works on that day.

Revise the sixth paragraph to read:

The Engineer will give the Contractor written notice of the completion date of the contract after all the Contractor’s obligations under the contract have been performed by the Contractor. The following events must occur before the Completion Date can be established:

1. The physical work on the project must be complete; and
2. The Contractor must furnish all documentation required by the contract and required by law, to allow the Contracting Agency to process final acceptance of the contract. The following documents must be received by the Project Engineer prior to establishing a completion date:
   a. Certified Payrolls (Federal-aid Projects)
   b. Material Acceptance Certification Documents
   d. Final Contract Voucher Certification
   e. Property owner releases per Section 1-07.24
(March 13, 1995)

Section 1-08.5 is supplemented with the following:

This project shall be physically completed within 90 working days.

SECTION 1-09 MEASUREMENT AND PAYMENT

1-09.2 Weighing Equipment

1-09.2(1) General Requirements for Weighing Equipment
(August 6, 2001)

Section 1-09.2(1) is revised to read as follows:

Any construction materials to be proportioned or measured and paid for by weight, shall be weighed on scales. These materials include natural, manufactured or processed materials obtained from natural deposits, stockpiles, bunkers, or mixing plants. The Contractor shall provide, set up, and maintain the scales necessary to perform the weighing or shall designate permanently installed, certified commercial scales for the purpose. Each truck to be weighed shall bear a unique identification number. This number shall be legible and in plain view of both the scale operator and the person receiving the material at the jobsite. Scales provided or designated by the Contractor shall be accurate to within one-half of one percent throughout the range of use.

An agent of the scale manufacturer shall test and service any scale before its use at each new site and then at 6-month intervals. The Contractor shall provide the Engineer a copy of the final results after each test.

All initial weighing at the dispatch site or at another site approved by the Engineer shall be performed by a Contractor employee or by another person designated by the Contractor. The designated weigher shall prepare a weigh or load ticket to accompany each load. Each ticket shall contain the truck identification number, the date and time of weighing the load, a description of the material being weighed and the signature or initials of the weigher.

Each weigh or load ticket shall also contain a determination of the net weight of the load. This shall be a reading from any device which weighs as material is loaded or a calculation including gross weight and tare weight when the method of loading does not include weighing. It shall also identify the weighed material. When used, tare weights shall be taken of each hauling vehicle at least twice a day. The ticket shall be provided to the inspector at the jobsite immediately after the material is delivered.

Except as noted below, all weighing shall be subject to confirmation testing through random checks made with a separate scale. The secondary scale shall be described in the contract provisions, either as a designated independent commercial scale or as a platform scale installed by the Contractor at a location named in the provisions. The
inspector will select loaded trucks at random and weigh them with the secondary scale. The same trucks will be weighed empty when the tested load has been delivered. The frequency of confirmation testing will be such that at least one test is performed for each contract item paid by weight for each $50,000 of payment for that item and at least one test weekly for each weighed contract item performed during that week.

Confirmation testing will not be routinely conducted for small quantities of weighed material. A small quantity shall be defined as one whose estimated proposal quantity, multiplied by its unit price, has a value of less than $20,000. The inspector may choose to apply confirmation testing to a minor quantity item if, in the inspector’s judgment, there is reason to suspect that the ticket weight might be incorrect.

1-09.6 Force Account
(October 10, 2008 APWA GSP)

Supplement this section with the following:

The Contracting Agency has estimated and included in the Proposal, dollar amounts for all items to be paid per force account, only to provide a common proposal for Bidders. All such dollar amounts are to become a part of Contractor’s total bid. However, the Contracting Agency does not warrant expressly or by implication, that the actual amount of work will correspond with those estimates. Payment will be made on the basis of the amount of work actually authorized by Engineer.

1-09.8 Payment For Material On Hand
(April 28, 1997)

The last paragraph of Section 1-09.8 is revised to read:

The Contracting Agency will not pay for any individual item on hand with a cost of less than $2,000. As materials are used in the work, credits equaling the partial payments for them will be taken on future estimates. Each month, no later than the estimate due date, the Contractor shall submit a letter to the Project Engineer that clearly states: 1) the amount originally paid on the invoice (or other record of production cost) for the items on hand, 2) the dollar amount of the material incorporated into each of the various work items for the month, and 3) the amount that should be retained in material on hand items. If work is performed on the items and the Contractor does not submit a letter, all of the previous material on hand payment will be deducted on the estimate. Partial payment for materials on hand shall not constitute acceptance. Any material will be rejected if found to be faulty even if partial payment for it has been made.
1-09.9 Payments
(October 10, 2008 APWA GSP)

Revise the first paragraph to read:

The basis of payment will be the actual quantities of Work performed according to the Contract and as specified for payment. For items Bid as lump sum, with a bid price of more than or equal to $20,000, the Contractor shall submit a breakdown of their lump sum price in sufficient detail for the Project Engineer to determine the value of the Work performed on a monthly basis. Lump sum breakdowns shall be provided to the Project Engineer no later than the date of the preconstruction conference.

Delete the third paragraph and replace it with the following:

Progress payments for completed work and material on hand will be based upon progress estimates prepared by the Engineer. A progress estimate cutoff date will be established at the preconstruction conference.

The initial progress estimate will be made not later than 30 days after the Contractor commences the work, and successive progress estimates will be made every month thereafter until the Completion Date. Progress estimates made during progress of the work are tentative, and made only for the purpose of determining progress payment. The progress estimates are subject to change at any time prior to the calculation of the Final Payment.

The value of the progress estimate will be the sum of the following:
1. Unit Price Items in the Bid Form — the approximate quantity of acceptable units of work completed multiplied by the unit price.
2. Lump Sum Items in the Bid Form — partial payment for lump sum Bid items will be a percentage of the price in the Proposal based on the Engineer’s determination of the amount of Work performed, with consideration given to, but not exclusively based on, the Contractor’s lump sum breakdown for that item.
3. Materials on Hand — 100 percent of invoiced cost of material delivered to Job site or other storage area approved by the Engineer.
4. Change Orders — entitlement for approved extra cost or completed extra work as determined by the Engineer.

Progress payments will be made in accordance with the progress estimate less:
1. Retainage per Section 1-09.9(1);
2. The amount of Progress Payments previously made; and
3. Funds withheld by the Contracting Agency for disbursement in accordance with the Contract Documents.

Progress payments for work performed shall not be evidence of acceptable performance or an admission by the Contracting Agency that any work has been satisfactorily completed. The determination of payments under the contract will be final in accordance with Section 1-05.1.
Payments will be made by warrants, issued by the Contracting Agency’s fiscal officer, against the appropriate fund source for the project. Payments received on account of work performed by a subcontractor are subject to the provisions of RCW 39.04.250.

1-09.13(3) Claims Resolution

1-09.13(3) Claims $250,000 or Less
(October 1, 2005 APWA GSP; may be used on FHWA-funded projects)

Delete this Section and replace it with the following:

The Contractor and the Contracting Agency mutually agree that those claims that total $250,000 or less, submitted in accordance with Section 1-09.11 and not resolved by nonbinding ADR processes, shall be resolved through litigation unless the parties mutually agree in writing to resolve the claim through binding arbitration.

1-09.13(3)A Administration of Arbitration
(October 1, 2005 APWA GSP)

Revise the third paragraph to read:

The Contracting Agency and the Contractor mutually agree to be bound by the decision of the arbitrator, and judgment upon the award rendered by the arbitrator may be entered in the Superior Court of the county in which the Contracting Agency’s headquarters are located. The decision of the arbitrator and the specific basis for the decision shall be in writing. The arbitrator shall use the contract as a basis for decisions.

DIVISION 2
EARTHWORK

SECTION 2-01 CLEARING, GRUBBING, AND ROADSIDE CLEANUP

2-01.1 Description

Section 2-01.1 is supplemented with the following:

(*******)
This work includes clearing the existing ground surface as necessary for excavation and subgrade preparation for septage solids and liquid lagoons, access road construction, and other items of work. Generally, there are no trees in areas to receive clearing. Clearing shall include removing and disposing of all unwanted material on or protruding from the existing ground surface.
2-01.3 Construction Requirements

Section 2-01.3 is supplemented with the following:

(******)
The Contractor shall clear areas:

a. To be excavated for lagoon construction.
b. To receive geomembrane liner.
c. To be excavated for any purpose.
d. Upon which embankment (roadways) will be placed.
e. Upon which drainage facilities will be placed or improved.

The Contractor shall remove and dispose of all unwanted material on or protruding from the ground surface, which will be a minimal requirement. This shall include, but not be limited to, vegetation, grass, sagebrush, and other objectionable material and obstructions interfering with the proposed work. Generally, all materials which protrude from the ground surface shall be cut off at the ground surface, such as vegetation, grass, etc., and disposed of at the landfill as designated by the Engineer.

2-01.4 Measurement

Section 2-01.4 is supplemented with the following:

(******)
Clearing and grubbing shall be incidental to the other items of work and shall not be measured for payment.

2-01.5 Payment

Section 2-01.5 is supplemented with the following:

(******)
No payment will be made for clearing and grubbing. This work shall be incidental to the other items of work for the project.

SECTION 2-02 REMOVAL OF STRUCTURES AND OBSTRUCTIONS

2-02.1 Description

Section 2-02.1 is supplemented with the following:

(******)
The work described in this section includes decommissioning the existing “DNR” well in accordance with WAC 173-160-381.

The existing DNR well has a depth of 1,355± feet, an open hole from 738± feet to 1,355± feet. Casing of various sizes exists from ground surface to 738± feet. The only above ground improvement is an 8 foot x 8 foot concrete pad which shall be removed and disposed on-site at the landfill at a location directed by the Engineer.

Section 2-02.2 is added

(******)

Materials

1. **General.** Materials and products that come into contact with drinking water supplied by public water systems or that come into contact with drinking water treatment chemicals used by public water systems shall meet the requirements of National Sanitation Foundation Standard 61 Drinking Water System Components – Health Effects (Revised October 1988) or equivalent. These materials and products include, but are not limited to, process media, protective materials, joining and sealing materials, pipes and related products, and mechanical devices used in treatment, transmission, and distribution systems.

2. **Cement Grout.** Cement grout shall conform to applicable State standards and requirements. Cement grout used to seal a well shall be composed of a uniformly mixed slurry of Portland cement or High Early Strength Type III Portland cement and potable water, or High-alumina cement and potable water mixed in the following proportions:

<table>
<thead>
<tr>
<th>Type of Cement</th>
<th>Gallons of Water per Sack of Dry Cement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Portland Cement</td>
<td>4 1/2 to 6</td>
</tr>
<tr>
<td>High Early Strength Type III Portland Cement</td>
<td>5 1/2 to 6 1/2</td>
</tr>
<tr>
<td>High-Alumina Cement</td>
<td>4 1/2 to 6</td>
</tr>
</tbody>
</table>

Additives to increase fluidity, reduce shrinkage, or control time of set may be used in a cement grout mixture. Expanding agents such as aluminum powder may be used at a rate not exceeding 0.075 ounce (1 level teaspoonful) per sack of dry cement. The powder shall not contain polishing agents. The addition of bentonite clay to a grout mixture is permissible but shall not exceed 5 percent by weight of dry cement. Sand shall not be added to grout seal mixtures. Calcium chloride may be added to a Portland cement grout to accelerate the set but shall not exceed 2 pounds per sack of dry cement. High-alumina cement and Portland cement of any type shall not be mixed together for use in a water well.
Cement types other than those set forth herein shall not be used as a scaling material in a well except upon written approval of the appropriate State agency and the Engineer.

**3. Bentonite Pellets.** Bentonite pellets shall be Environmental Plug Tablets as supplied by Mitchell, Lewis, and Staver, having a minimum diameter of 1/2 inch or approved equal.

2-02.3 Construction Requirements

Add the following subsection:

(******)

2-02.3(4) Well Decommissioning Work

The existing well data is shown on the Drawings. The abandonment of the DNR well shall conform to WSDOE “Minimum Standards for Construction and Maintenance of Wells”, WAC 173-160-415 and WAC 173-160-381 “What are Standards for Decommissioning a Well”. The specific requirements for the abandonment work is shown on the Drawings and outlined hereafter.

- Perform site work as required (i.e., remove concrete structures from the well). Use care not to allow any foreign material to enter the well casing.

- Sound the well to verify the well depth.

- Place the bentonite plugs or cement grout material at the locations shown on the Drawings to seal the open hole. The Contractor shall continuously sound the well to determine the location of the backfill material placed and to prevent any bridging of the material as it is placed in the well. If a bridge develops in the well, the Contractor shall be responsible for removing the bridge. The bentonite plug shall utilize bentonite pellets.

- Monitor water levels as bentonite plugs and cement grout are placed. If flowing conditions occur, provide flow control as needed.

- After the bentonite plugs are placed in the well, perforate the existing casing in accordance with State Standards, perforating the well from the bottom to the top of the casing, using four (4) cuts per row and one (1) row per foot. Each cut shall be at least 1 1/2 inches long. A variance has been requested from WSDOE to eliminate perforating of the 20-inch casing which has a clay and cement seal to the top of basalt.

- Place cement grout in the well at the location shown on the Drawings. The cement grout shall be placed from the bottom of the well upward in one continuous operation if well conditions allow. A tremie pipe shall be utilized. One exception to placing the grout in a continuous manner will

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be if the well formations take excessive amounts of grout while the grout is being placed. In this situation, the grouting operation may be stopped and the Contractor may allow the placed grout to set up before continuing the operation. If necessary, the Contractor may place a bentonite plug in zones that are taking excessive amounts of grout. The Contractor shall submit a grouting plan to the Engineer for review prior to the grouting operations being performed. The grouting plan shall outline methods, procedures, equipment, and materials to be used to complete the grout seal.

- The upper 20 to 30 feet of the well may be filled with granular bentonite pellets to facilitate cutting casing. Upon completion of the grouting operation, and allowing sufficient time for the grout to properly set, the Contractor shall cut and remove the upper portion of the well casing and weld a watertight steel cover on top of the casing at the elevation shown in the Drawings. The Contractor shall then backfill the hole with native soil in 12-inch lifts compacted to 90 percent of ASTM D1557 laboratory density. The top foot of the hole shall be soil prepared for HDPE liner.

- The Contractor shall document the quantities and materials placed in the well to be abandoned, and a proper report shall be filed with the WSDOE.

2-02.4 Measurement

Section 2-02.4 is supplemented with the following:

(***)

No specific unit of measurement shall apply to the lump sum Bid Item “Well Decommission”.

2-02.5 Payment

Section 2-02.5 is supplemented with the following:

(***)

Payment for well decommission will be on a lump sum basis. The lump sum contract price shall be full pay for all costs in connection with properly decommissioning the existing DNR well in accordance with State regulations. Payment will be made in accordance with Section 1-04.1. Well Decommission is one item and will include all materials, equipment and required regulatory requirements for completing the work.

SECTION 2-03 ROADWAY EXCAVATION AND EMBANKMENT
2-03.1 Description

Section 2-03.1 is supplemented with the following:
The work involved in this contract, regardless of the nature or type of the materials encountered, includes excavating, grading the septage solids and liquid lagoons, grading for access roadways, berm and road embankment, filling cracks, fissures, holes, overexcavated areas, and other void spaces during subgrade preparation, placement and compaction of lagoon and road embankments and subgrade materials, all haul, and the stockpiling of materials. These activities may be performed in making cuts, embankments, slopes, ditches, turn-around areas, performing grading, and in completing related work. The work shall conform as shown on the Plans and as directed by the Engineer.

All work described herein shall reasonably conform to the alignment, grade, and cross-sections shown in the Plans or established by the Engineer. The Contractor shall maintain a tolerance of plus or minus 0.5 foot for rough grading of cuts and embankments and plus or minus 0.1 foot for lagoon liner grading, protective cover soil, and roads as shown on the Plans. If the Contractor overexcavates beyond this tolerance the Engineer may require replacing in accordance with 2-03.3(14)C Compacting Earth Embankments Method B. On-site and off-site haul routes will be established, maintained, and restored by the Contractor.

Other work includes the surface water control ditch and erosion control features, and stockpiling excess material.

Add new subsection:

(*****)

2-03.2 Materials

Generally, excavated material shall be used for constructing embankments and for protective cover soil. Excess excavated material not used in the construction shall be stockpiled in the areas identified as Stockpile Area #1 and Stockpile Area #2 as shown on the Plans or as directed by the Engineer.

Protective cover soil is defined as on-site excavated soils that consist of primarily silty sand with a maximum particle size of 1/2-inch and free of sharp edges underlain by Type 3 geotextile. Embankment shall have a maximum particle size of 6 inches.

Other materials shall meet the requirements of the following sections:

<table>
<thead>
<tr>
<th>Crushed Surfacing Base Course</th>
<th>9-03.9(3)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Geotextile</td>
<td>9-33.1</td>
</tr>
</tbody>
</table>

2-03.3(7) Disposal of Surplus Material

2-03.3(7)A General

Delete Section 2-03.3(7)A and replace with the following:
Excavations and embankments shall be finished with all surfaces smooth and uniform. The tops and bottoms of slopes shall be rounded.

The Contractor shall be responsible for the stability of all embankments constructed under this contract and shall make repairs to any portion damaged or displaced at his own expense.

All compaction requirements shall be based on maximum densities as determined by the Modified Proctor method (ASTM D1557).

The embankment material shall be compacted to at least 95 percent of maximum density with compaction equipment approved by the Engineer.

All suitable excavated materials removed from site excavations shall be used as far as practicable in the construction of embankments, berms, slopes, and at such other places as may be directed by the Engineer. Suitable excavated material shall not be wasted or removed from the site. Material which is suitable as embankment or protective cover soils shall be placed directly in embankments or stockpiled at a convenient location approved by the Engineer. Stockpiled material shall be protected from erosion by wind and rain. Excess excavated material shall be stockpiled at the locations shown in the Plans, or as directed by the Engineer.

The lagoons shall be excavated, and berms constructed, to the lines and grades shown on the Plans.

Access roads and septage lagoon berms shall be constructed using embankment, as shown on the Plans. Embankment material determined by the Engineer to be unsuitable for this purpose shall be considered stockpile material.

The geotextile shall be placed as shown on the Plans and installed according to the manufacturer recommendations.

2-03.3(7)B Haul

Delete Section 2-03.3(7)B and replace with the following:

The Contractor shall haul and place excess excavated material in the locations as shown on the Plans. If excavation yields more material than can be accommodated in the designated areas for stockpiling, the Contractor shall dispose of the excess quantity in local locations as directed by the Engineer.

All haul costs associated with stockpiling excess excavated material shall be incidental to the Bid Item “Pond Excavation, Incl. Haul” per cubic yard. The Contractor shall be
responsible for any haul route construction, maintenance, and restoration necessary to
gain access to the stockpile areas.

2-03.3(10) Selected Material

Section 2-03.3(10) is supplemented with the following:

(******)
A protective cover soil layer shall be placed in the septage lagoons to the lines and
grades as shown on the Plans and as directed by the Engineer. The Contractor shall
take care backfilling protective cover soil on the geomembrane and non-woven
geotextile. The protective cover soil shall be carefully placed and spread with low
ground pressure equipment; direct dumping is prohibited. Material hauling
equipment shall require a minimum of 24 inches of protective cover soil while
operating over geosynthetic materials.

The protective cover soil layer may be placed in one lift such that the final thickness
meets the requirements of Plans after compacting. The Contractor shall moisture
condition the soil as necessary for placement and dust control. The protective cover
soil layer shall receive at least two passes with standard compaction equipment such
as a self-propelled smooth-drum roller. The Contractor shall take care compacting
and operating construction equipment near the lagoon side-slopes.

2-03.3(14) Embankment Construction

2-03.3(14)B Earth Embankment Construction

Section 2-03.3(14)B is supplemented with the following:

(******)
Excess excavation material shall be placed in stockpile at the locations shown on the
Plans. The material shall be placed in horizontal layers of uniform thickness. The
stockpiled material shall be compacted by spreading with tracked equipment and/or
by routing hauling equipment over the material using Method A Section 2-03.3(14)C
Compacting Earth Embankments unless otherwise specified herein. The stockpiled
material shall be placed to achieve a minimum of 80 percent of maximum density.
All side slopes shall be placed to achieve a maximum of 2H:1V and the tops shall be
sloped to drain to the perimeter a minimum of 2 percent unless otherwise directed by
the Engineer.

2-03.3(14)C Compacting Earth Embankments

Compacting embankments and excavations shall be by Method "C" as specified under
Section 2-03.3(14)C of the Standard Specifications.

2-03.4 Measurement
Section 2-03.4 of the Standard Specifications is supplemented with the following:

(******)
Only one determination of the original ground elevations shall be made on this project. Measurement for excavation and embankment shall be based on the original ground elevations recorded previous to the award of this Contract and the alignment, profile, grade, and sections as shown on the plans.

If discrepancies are discovered in the ground elevations which will materially effect the quantities of earthwork, the original computations of earthwork shall be adjusted accordingly.

Earthwork quantities shall be computed either manually or by means of electronic data processing equipment, by use of the average end area method.

Copies of the ground cross-section notes shall be available for the bidder's inspection, before the opening of bids, at the office of the County Engineer. Upon award of the Contract, copies of the original ground cross-sections shall be furnished to the successful bidder on request to the County Engineer.

Protective soil cover shall be measured per cubic yard, in place in the completed construction, measured to the nearest cubic yard, neat line within the limits shown on the Plans.

Pond excavation including haul shall be measured per cubic yard. All excavated material will be measured in the position it occupied before the excavation was performed.

2-03.5 Payment

Section 2-03.5 of the Standard Specifications is deleted and replaced with the following:

(******)
The contract unit price for "Pond Excavation, Incl. Haul," per cubic yard, shall be full compensation for all labor, equipment, tools, and materials necessary to excavate, load, haul, place, compact, stockpile and shape the materials, and any other work required to complete this item as specified and no further payment shall be made.

The contract unit price for "Embankment Compaction," per cubic yard, shall include all costs to place and compact the material as required, and no further payment shall be made.

The contract unit price for "Protective Cover Soil", per cubic yard, shall include all costs to place material as required, and no further payment shall be made.

SECTION 2-06 SUBGRADE PREPARATION
2-06.1 Description

Section 2-06.1 is supplemented with the following:

(*****)
The work consists of preparing the subgrade for the septage lagoon geomembrane liner, berm embankment, and access road embankments and crushed surfacing base course.

Add new subsection.

(*****)

2-06.4 Subgrade for Geomembrane Liners

Subgrade for Geomembrane Liners shall conform to the requirements of 2-06.3(1) except that:

1. Generally, the existing ground surface shall be prepared and excavated to the grades and lines as shown on the Plans or directed by the Engineer.

2. The Contractor shall over excavate any areas with rock or sharp edges and replace with soil free of rock or hard objects. The Contractor shall screen soil if needed to ensure no rocks or hard objects are in contact with the liner.

3. The Contractor shall grade and compact the excavated surface, after clearing and grubbing, to a smooth, firm, and non-yielding foundation suitable for placement of the overlying geomembrane liner.

4. The Contractor shall add water as needed for compaction and dust control.

5. The subgrade surface shall be proof-rolled to assure that it is firm and non-yielding. Proof-rolling on grades less than 10 percent shall be performed by running an empty 10 cubic yard dump truck, or other equipment acceptable to the Engineer, across the entire area. Proof-rolling on grades greater than 10 percent shall be performed by running or pulling approved construction equipment across the entire area. The Engineer shall observe all proof-rolling.

2-06.5 Measurement and Payment

Section 2-06.5 is supplemented with the following:

(*****)
Subgrade preparation shall be incidental to the other bid items of the Contract for constructing lagoons, access roads, ditches, and other elements of the work, and no further payment shall be made.
SECTION 2-07 WATERING

Section 2-07 is deleted and replaced with the following:

(******)
The Contractor shall be solely responsible for dust control on this project and shall protect the public, adjacent homes, orchards and crops from damage due to dust, by whatever means necessary. The Contractor shall be responsible for any claims for damages and shall protect the County from any and all such claims.

This work consists of furnishing, hauling, and applying water for compacting embankments, constructing subgrade, placing of crushed surfacing, and control of dust during construction activities. The Contractor shall use water for dust control, as needed, at all times during construction and in staging areas, and along haul roads within the Cheyne Road facility site. The construction area, including the final grade of protective cover soil, shall be watered down as necessary to encapsulate and prevent dust particles from becoming airborne.

When directed by the Engineer, the Contractor shall provide water for dust control within two hours of such order and have equipment and manpower available at all times including weekends and holidays to respond to orders for dust control measures.

If County forces are required to respond to a dust control problem, the Contractor shall be charged liquidated damages to offset County expenditures. For each time that the County is required to provide dust control measures, the Contractor shall be assessed damages in the amount of $500.00, which shall be deducted from any moneys due the Contractor under this contract.

Payment for water used for dust control, compaction, processing of base course and top course, and other work shall be included in the other Bid Items involved, and no further payment shall be made.

SECTION 2-09 STRUCTURE EXCAVATION

2-09.4 Measurement

Section 2-09.4 the second sentence of the second paragraph is revised to read:

(******)
Measurement will be made from existing ground line to the bottom of the excavation and for the length of the shoring or Extra Excavation Work actually performed.

Section 2-09.4 shall be supplemented with the following:

(******)
Structure Excavation Class A and B shall not be measured for payment.

2-09.5 Payment
Section 2-09.5 shall be supplemented with the following:

(******)
There shall be no separate payment for Structure Excavation Class A or B. All costs associated with excavation, backfill and compaction of pipe, and culvert trenches shall be included in the linear foot price of the pipe or culvert.

All costs associated with excavation and backfilling for footings shall be considered incidental to the other various bid items of the contract.

SECTION 2-10 DITCH EXCAVATION

2-10.4 Measurement

Section 2-10.4 shall be supplemented with the following:

(******)
The Bid Item “Surface Water Ditch” shall be measured per linear foot of ditch.

2-10.5 Payment

Section 2-10.5 shall be supplemented with the following:

(******)
Payment for the Bid Item “Surface Water Ditch” shall be per linear foot of ditch, and shall include all costs to excavate and shape the ditch, including haul, and no further payment shall be made.

DIVISION 6
STRUCTURES

SECTION 6-02 CONCRETE STRUCTURES

6-02.4 Measurement

Section 6-02.4 is supplemented with the following:

(******)
No specific unit of measurement shall apply to the lump sum Bid Item “Septage Discharge Station.”

Concrete walkway structural foundations and concrete stairways shall be measured in place by the cubic yard to the neat lines of the structure as shown on the Plans.

6-02.5 Payment
Section 6-02.5 shall be supplemented with the following:

(******)
Payment for the septage discharge station shall include all costs for supplying and installing all excavation, materials, including gravel base, cement concrete, reinforcing steel, quarry spalls, gravel base, hoses, fittings, hose racks, guard posts, and other incidentals required to construct the complete septage discharge station as shown on the Plans. Payment for the Bid Item "Septage Discharge Station", per Lump sum shall be for all required for a complete installation, and no further payment shall be made.

Payment for the walkway concrete foundation and concrete stairway shall be paid for per Bid Item “Conc. Class 4000”, per cubic yard, and shall include all costs to excavate for the footings and stairs, supply and install class 4000 concrete and all steel reinforcing bars as shown on the Plans, and all other incidental items necessary for a complete installation, and no further payment shall be made.

SECTION 6-03 STEEL STRUCTURES

6-03.3(7) Shop Plans

Section 6-03.3(7) is revised to read:

(******)
The Contractor shall submit to the Engineer for review and approval all shop detail plans for fabricating the steel. No material shall be fabricated until approved by the Engineer. The Engineer shall have ten (10) working days to review and return the shop drawings. The Engineer shall have an additional five (5) working days to review revised or corrected shop plans.

6-03.4 Measurement

Section 6-03.4 is supplemented with the following:

(******)
Measurement for “Walkway and Decant Valve Structure” shall be made per each.

6-03.5 Payment

Section 6-03.5 is supplemented with the following

(******)
The unit contract price for each “Walkway and Decant Valve Structure” shall be for fabricating and installing each walkway structure, including all railing, grates, decant valves, fasteners, coatings, and other incidentals required to construct each walkway structure as shown on the Plans. Concrete for the walkway structure foundations will be paid for under the Bid Item “Conc. Class 4000.”
DIVISION 7
DRAINAGE STRUCTURES, STORM SEWERS, SANITARY SEWERS, WATER MAINS,
AND CONDUITS

SECTION 7-12 VALVES FOR WATER MAINS

7-12.2 Materials

Section 7-12.2 is supplemented with the following:

(******)
The 12-inch decant valve shall meet the requirements of 9-30.3 of the Special Provisions.

7-12.4 Measurement

Section 7-12.4 is supplemented with the following:

(******)
There shall be no separate measurement for “Decant Valve 12 In. Diam.”. It shall be incidental to the Bid Item "Walkway and Decant Valve Structure".

Measurement of "Leachate Pump System" shall be per lump sum for each installed system including HDPE pipe sump, submersible pump, and all associated appurtenances for a fully contained and operational leachate pump system.

7-12.5 Payment

Section 7-12.5 is supplemented with the following:

(******)
All costs to furnish and install the valve complete in place, including all piping and fittings up to the connection to the PVC pipe, brackets, and fittings necessary for attachment to the walkway, and other incidentals required to install the valve shall be paid under the Bid Item "Walkway and Decant Valve Structure", per each. No additional payment shall be made for decant valve or components.

Payment for "Leachate Pump System" shall include all costs to furnish and install leachate pump system. No additional payment shall be made for leachate pump system components.
DIVISION 8
MISCELLANEOUS CONSTRUCTION

SECTION 8-01  EROSION CONTROL AND WATER POLLUTION CONTROL

8-01.1 Description

Section 8-01.1 is revised to read:

(*****)
The temporary erosion and sediment control provisions contained herein shall be coordinated with the permanent erosion control features specified elsewhere in the Contract to the extent practicable to assure economical, effective, and continuous erosion control throughout the construction and post construction period. The temporary or permanent drainage facilities shall be installed as required by these Specifications or as ordered by the Engineer.

8-01.3 Construction Requirements

Section 8-01.3 is supplemented with the following:

(*****)
The Contractor shall follow Section 8-01.3(2)F for dates of application of seed, fertilizer, and mulch for east of the summit of the Cascade Range. Erosion Control Blanket shall be placed within a day of the application of hydroseeding and shall be installed according to manufacturer installation guide (e.g., anchoring method, overlapping, staple patterns, etc.).

Add new subsection:

(*****)
8-01.3(17) Temporary Erosion Control

Temporary erosion control to protect the work, borrow areas, stockpile area(s), haul roads, and adjacent property shall be the sole responsibility of the Contractor. The Contractor shall meet the requirements and pollution control laws, rules, and regulations of Federal, State, and local agencies.

The Contractor shall incorporate erosion control features into the project as specified and shown on the Plans prior to commencing clearing, grubbing and earthwork. The Contractor shall limit the surface area of earth material exposed by clearing, excavation, borrow, and embankment operations to that which is necessary to perform the next operation within a given area and his capability to provide adequate protection of the site, completed work, adjacent property, drainage, and watercourses.
Clearing of vegetation shall be confined within the limits of work which shall be actively prosecuted within 15 days. Excavation, borrow, and embankment construction shall be confined to the minimum area necessary to accommodate the Contractor's equipment and work force engaged in the earth moving work.

The Contractor shall, at the end of each work operation in any one day, shape the earthwork in such a manner as to control and direct the runoff of rainwater. Controlled discharges shall be provided for all waters impounded, directed, or controlled by project activities or erosion control measures.

Finished cut slopes shall be shaped, planted, or finished as specified as the work progresses. When excavation in a work area is suspended for an extended length of time (14 days or longer), the open slope and exposed surfaces shall be protected without delay. For on-site borrow and work areas, this protection shall include installation and maintenance of temporary soil stabilization methods such as hydromulching, straw mulch, jute matting, plastic netting, plastic tarp, or as required by the Engineer for best management practices.

8-01.4 Measurement

Section 8-01.4 is supplemented with the following:

(*-----*)

The area for seeding, fertilizing, and mulching shall be measured per acre, measured in place in the completed construction, neat line within the limits shown on the Plans, or specified and as directed by the Engineer, measured horizontally with no adjustment for slope distance.

The erosion control blanket shall be measured per square yard, measured in-place in completed construction, neat line within the limits shown on the plans or specified and as directed by the Engineer, measured horizontally with no adjustment for slope distance.

Add new section:

(*-----*)

8-05 LAGOONS DOUBLE LINER WITH LEACHATE COLLECTION

8-05.1 Description

This work consists of installing geotextile, HDPE boots, and geomembrane liner for the lagoons bottom including anchor trench, as shown on the Plans.

8-05.2 Materials

Materials shall meet the requirements of the following sections:

Geotextile 9-33.1
Geomembrane 9-33.2

SP 3265 – Cheyne Landfill Septage Lagoons  Page 121  Special Provisions
8-05.3 Qualifications

8-05.3(1) Manufacturer

The geosynthetic subcontractor shall provide geosynthetics with the following minimum qualifications:

- The manufacturer shall be listed by the National Sanitation Foundation as having met the current Standard 54 for Geomembranes, and shall have at least five continuous years of experience in the manufacture of polyethylene geomembranes and shall have manufactured a minimum of five million square feet of HDPE geomembrane prior to producing materials for this project.

- The geotextile manufacturer shall have produced at least one million square feet of each type of geotextile meeting the specifications for this project prior to producing materials for this project.

8-05.3(2) Installers

The geosynthetic subcontractor shall provide for geosynthetics installation by installers with the following minimum qualifications:

The installer shall have successfully installed a minimum of five million square feet of geomembrane of the type specified for this project.

The supervisor for the installer shall have supervised the installation of at least one million square feet of HDPE of the type specified for this project.

8-05.4 Submittals

Prior to shipping geosynthetic materials to the site, the Contractor shall submit, certified test results showing that the geosynthetic components meet the requirements specified in Section 9-33. The Contractor shall also submit the information specified in the following subsections.

8-05.4(1) Geomembrane

- Qualifications statement of manufacturer
- Qualifications statement of installer
- Resume of installation supervisor
- Samples and product description
• Manufacturer’s certification that material meets project specifications, quality control certificates issued by the resin supplier, quality control certificates for each roll produced, certification that geomembrane and extrudate produced for this project have the same material properties.

• Installation procedures and quality control program, including a plan for protecting the work, testing the work, and for repairing and replacing damaged work.

• Proposed installation panel layout identifying panels, seams, and details.

8-05.4(3) Geotextile

• Qualifications statement of manufacturer

• Samples and product description for each type

• Manufacturer’s certification that material meets project specifications

8-05.4(4) Quality Control Submittal and Reports

The geomembrane installer shall submit to the Engineer, 15 days prior to delivery of geomembrane to the project site, a quality control manual including quality control procedures, tests, inspection personnel, and documentation.

The geomembrane installer shall submit, on a daily basis, the following reports:

1. Daily progress reports shall be prepared including the following:

• Project Name

• Date

• Weather conditions, including range of wind speed and temperature, cloud cover, and precipitation

• Project location

• Panels installed (by number)

• Panels seamed

• Field Observations
2. Daily quality control records acceptable to the Engineer shall be prepared
detailing the initial weld qualification of equipment and welding crews. Daily
quality control records shall be maintained of all field seaming including, but
not limited to, the following:

- Date
- Project location
- Weld location, panel number
- Sheet temperature
- Weld crew identification
- Weld samples, if taken
- Test Results
- General observations

8-05.5 Construction Requirements

No layer of geosynthetic material shall be covered until the Engineer has inspected and approved
the installation.

8-05.5(1) Geomembrane

8-05.5(1)A Packing, Labeling, Shipping, Storage, and Handling

Each roll of geomembrane shall be uniquely marked by the manufacturer
identifying the roll number and date of manufacture. Labels on each roll panel
shall identify the panel locations, thickness of the material, the length and width
of roll, and manufacturer.

Materials damaged in shipping shall be replaced at the Contractor’s expense.

The contractor shall supply to the Engineer that information required in
Section 8-05.4(1) prior to delivery to the site of each roll of geomembrane and
each batch of extrudate.

The geomembrane rolls shall be stored on a smooth, flat, non-abrasive surface,
not on wooden pallets, and stacked no more than two high. The geosynthetic
contractor shall protect the geomembrane rolls at all times from dirt, grease,
moisture, heat, and any cause of damage. The Contractor shall replace
geomembrane damaged by any means at his expense.
8-05.5(1)B Fabrication, Sampling, and Testing

General:

Prior to or during factory seaming, roll goods shall be visually inspected on both sides for defects and impurities. Defects and impurities shall be removed and repaired prior to completion of the fabrication process. Thickness measurements shall be made at the center and each edge of the beginning and end of each roll of material in accordance with ASTM D 751. Rolls having a thickness less than the value specified herein shall be rejected.

Non-Destructive Factory Seam Testing (HDPE):

Non-destructive seam testing shall be conducted in accordance with the fabricator’s approved quality control manual. Continuous visual inspection and continuous non-destructive testing shall be performed on the seams during fabrication. Defective seams shall be repaired, retested and approved prior to continuation of the seaming process.

8-05.5(1)C Installation

The installation of the geomembrane will be under constant observation and monitoring of the Engineer.

Installation shall be performed under the constant direction of a single Installation Supervisor who shall remain on site at all times during the installation and be in responsible charge for all geomembrane installation, including panel layout, seaming, patching, testing, reporting, and all other activities associated with the installation.

Surface preparation shall be performed in accordance with the section, Excavation and Grading. Material larger than 3/8 inch in diameter that could damage the geomembrane shall be removed from the surfaces to be covered with the geomembrane. Ruts or ridges more than one inch shall be filled or removed. All sharp edges shall be rounded. The subgrade surface shall be observed daily by the Engineer and installer to evaluate the surface condition. Any damage to the subgrade caused by the Contractor’s operations shall be repaired at no additional cost to the Owner.

Prior to placing geomembrane over sections of prepared subgrade, the installation supervisor shall certify in writing that the receiving surface is acceptable.

Geomembrane deployment shall proceed between ambient temperatures of 45°F to 90°F. Placement can proceed above 90° and below 45° only after it has been verified by the Engineer that the material can be seamed according to the specification. Geomembrane placement or seaming shall not be done during any
precipitation, in the presence of excessive moisture (e.g., fog, rain, dew) or in the
presence of excessive winds. The Contractor shall maintain a thermometer
on-site to measure temperatures.

The geomembrane shall be installed to the limits indicated on the Plans. The
geomembrane shall be placed in such a manner to minimize field seaming. The
geomembrane shall be installed such that field seams run longitudinally down the
slope. The Contractor shall provide temporary wind anchorage (e.g., sand bags)
during geomembrane installation. Only geomembrane panels for each day’s field
seaming shall be spread each day and shall be held in position by sand bags until
field seaming is complete. As geomembrane materials are unrolled, the
Contractor shall perform further visual inspection of the geomembrane surface. If
damage or faults not previously observed are discovered, they shall be clearly
marked and the respective sheet roll will be set aside. The Engineer shall be
notified of the damage. All faulty areas shall be repaired in an appropriate
manner. The geomembrane panels shall be installed by experienced workers and
handled in a responsible manner. All rips, tears, puncture, or other injuries to the
liner shall be repaired the same day to the satisfaction of the Engineer and in
accordance with procedures as specified herein. All rips and tears with sharp
edges shall be rounded prior to patching. All patches shall have rounded edges.

The method used to place the panels shall minimize wrinkles; however, the
geomembrane manufacturer and installer shall coordinate efforts to provide the
proper amount of slack in the deployed geomembrane so as to compensate for
contraction due to local temperature extremes. The Contractor shall not cover
over wrinkles that could fold over.

**Test Seams**

Test seams shall be made on test strips of geomembrane to verify that seaming
conditions are adequate. They shall be made in the area to be seamed and
in contact with the subgrade. Test seams shall be made each day prior to
production seaming and whenever there is a change in seaming personnel
or seaming equipment, by each seamer and each piece of seaming
equipment used that day. One sample shall be obtained from each test
seam. This sample shall be 10 feet long for hot shoe welding and 3 feet
long for extrusion welding by 20 inches wide with the seam centered
lengthwise. Ten random specimens 1 inch wide shall be cut from the
sample with an on-site die cutter. The Installer shall field test 5 seam
specimens for shear strength and 5 seam specimens for peel adhesion
using the approved quantitative on-site tensiometer. Jaw separation speed
shall be as given in NSF 54. To be acceptable, four out of five replicate
test specimens must meet specified seam strength requirements. If the
field tests fail to meet these requirements, the entire operation shall be
repeated. If the additional test seam fails, the seaming apparatus or seamer
shall not be accepted or used for seaming until the deficiencies are
corrected by the Installer and two consecutive successful test seams are achieved.

Field Seams

General Requirements:

All panels shall be overlapped a minimum of 3 inches. In corners and odd-shaped geometric locations, the number of field seams shall be minimized. Seaming shall extend to the outside edge of panels/sheets to be placed in anchor and/or drainage trenches. Seaming shall not be conducted in the presence of standing water and/or soft subgrades as determined by the Engineer. Wet surfaces shall be thoroughly dried and soft subgrades compacted and approved by the Installer and Engineer prior to seaming. The seam area shall be cleaned of all dust, dirt, and foreign material prior to and during seaming. The installer shall write at the beginning of packfield seam the welding technicians initials and machine number, speed, and temperatures.

Polyethylene geomembranes shall be seamed by hot wedge methods. Extrusion welding shall only be allowed for welding textured to smooth geomembrane, boots, patching, and seaming around appurtenances. If seam overlap grinding is required, the procedure used shall not damage the geomembrane. Grinding marks shall be oriented perpendicular to the seam direction and no marks shall extend more than 1/8 inch beyond the extrudate after placement. The depth of the grinding marks shall be no greater than 10 percent of the sheet thickness. Where extrusion fillet welds are temporarily terminated long enough to cool, they shall be ground prior to applying new extrudate over the existing seam.
Field Sampling and Testing

Non-Destructive Field Seam Testing:

Field seams shall be non-destructively tested over their full length in accordance with the Installer’s approved quality control manual. Seam testing shall be performed as the seaming work progresses, not at the completion of field seaming. Any seams which fail shall be documented and repaired in accordance with paragraph Defects and Repairs.

Destructive Field Seam Testing:

A minimum of one destructive test sample per 400 feet of field seam shall be obtained at locations specified by the Engineer. Sample locations shall not be identified prior to seaming. Samples shall be a minimum of 12 inches wide by 36 inches long with the seam centered lengthwise. Each sample shall be cut into two equal pieces with one piece retained by the Installer and the remaining piece given to the Owner for quality assurance testing and permanent record. Each sample shall be numbered and cross referenced by the Installer to a field log which identifies: (1) panel/sheet number; (2) seam number; (3) top sheet; (4) date and time cut; (5) ambient temperature; (6) seaming unit designation; (7) name of seamer; and (8) seaming apparatus temperature and pressures (where applicable). A minimum of ten 1-inch-wide replicate specimens shall be cut from the Installer’s sample with the Installer supplied die cutter. A minimum of five specimens shall be tested for shear strength and five for peel adhesion using an approved field quantitative tensiometer. Field testing shall be in accordance with ASTM D 4437. Jaw separation speed shall be as given in NSF 54. To be acceptable, all replicate test specimens must meet the specified seam strength requirements. If the field or laboratory tests fail, the seam shall be repaired in accordance with paragraph Repair Procedures. In addition, destructive seam sample holes shall be repaired the same day as cut. Test results on field seams shall be submitted to and approved by the Engineer prior to acceptance of the seam.

Defects and Repairs

Identification:

Immediately prior to covering the geomembrane or after covering if using electrical leak location detection survey, seams and non-seam areas shall be visually inspected by the installer and Engineer for defects, holes, or damage due to weather conditions or construction activities.
At the Engineer’s discretion, the surface of the geomembrane shall be brushed, blown, or washed by the Installer if the amount of dust, mud, or foreign material inhibits inspection or functioning of the overlying material.

Evaluation:

Each suspect location shall be non-destructively tested. Each location that fails non-destructive testing shall be repaired and retested by the Installer until it passes.

Repair Procedures:

Defective seam areas may be overlaid with a strip of new material and seamed (cap stripped). Alternatively, the seaming path shall be retraced to an intermediate location a minimum of 10 feet on each side of the failed seam location. At each location a 12-inch by 12-inch minimum size seam sample shall be taken for 3 additional shear strength and 3 additional peel adhesion tests using an approved quantitative field tensiometer. If these tests pass, then the remaining seam sample portion shall be sent to the Independent Laboratory for 2 shear strength and 2 peel adhesion tests in accordance with ASTM D 4437. If these laboratory tests pass, then the seam shall be cap stripped between that location and the original failed location. If field or laboratory tests fail, then the process is repeated. After cap stripping, the entire cap stripped seam shall be non-destructively tested. Certified test results on all repaired seams shall be submitted and approved by the Owner prior to covering the seamed areas.

Patches:

Tears, holes, blisters, and areas with undispersed raw materials or foreign material contamination shall be repaired with patches. Patches shall have rounded corners, be made of the same geomembrane, and extend a minimum of 6 inches beyond the edge of defects. Minor localized flaws shall be repaired by spot welding or seaming as determined by the Engineer. Repairs shall be non-destructively tested. The Engineer may also elect to perform a destructive seam test on a suspect area.

Standards

The installer shall have and maintain on-site copies of the installers’, fabricators’, and manufacturers’ quality control manuals. Copies of all ASTM, GRI, and NSF standards that apply to this project for the field installation of the selected geomembrane shall be maintained on site by the Installer. Work
shall not begin until the Installer shows copies of all the required manuals and standards to the Engineer.

**Reporting**

The Installer shall submit copies of the previous day’s daily reports, destructive test results, nondestructive test results, trial seam results, material deployed, certification of subgrade acceptance, and defects and repairs to the Engineer prior to beginning work the following day. Any discrepancies shall be brought to the attention of the Engineer. No geomembrane installation shall begin until the Engineer has received and accepted the daily submittal from the Installer.

**8-05.5(1)D Warranty**

A written Warranty shall be obtained from the Manufacturer (for material) and the Installation Contractor (for workmanship). These documents shall warrant the quality of the material for a period of fifteen (15) years and workmanship for a period of two (2) years.

**8-05.5(1)E Field Seams**

The field seams shall meet the following specifications:

<table>
<thead>
<tr>
<th>Seam Property</th>
<th>Test Method</th>
<th>Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Shear Strength</td>
<td>ASTM D 3080 (as modified in App. A of NSF 54*)</td>
<td>&gt;90% of yield strength Film Tear Bond**</td>
</tr>
<tr>
<td>Peel Strength</td>
<td>ASTM D 413 (as modified in App. A of NSF 54*)</td>
<td>&gt;50% of yield strength Film Tear Bond**</td>
</tr>
</tbody>
</table>

* National Sanitation Foundation, Standard 54; “Flexible Membrane Liner.”
** Film Tear Bond (FTB) is defined as failure of one of the sheets by tearing, instead of separating from the other sheet at the weld interface area (sheet fails before weld).

**8-05.5(2) Geonet**

**8-05.5(2)A Packaging, Labeling, Shipping, and Storage**

Geonet shall be packaged and shipped in protective wrapping, labeled with appropriate identification including manufacturer, fabric weight, and roll length.

The geonet rolls shall be stored in its protective wrapping in a manner that protects the material from dirt, moisture, heat, and any other cause of damage.
8-05.5(2)B Installation

Care shall be taken to keep the geonet clean and free from debris prior to installation. The geotextile surface of the geonet to be placed face up and the surface to receive the geonet shall be free of dust, dirt, stones, and any other objects or debris. If the geonet is not clean before installation it shall be washed by the geosynthetic contractor just prior to installation. Cleaning of the geonet and geomembrane will be at the Contractor's expense.

The geonet shall be installed as shown on the Plans.

The geonet panels or sheets shall be joined/seamed to adjacent panels or sheets by overlapping and securing with plastic ties. Plastic ties shall be white or other bright colors for easy inspection. Metallic ties shall not be allowed.

Adjacent panels shall be overlapped a minimum of 2 inches. On slopes, the ends of geonet panels shall overlap adjacent panels ends a minimum of 4 inches by shingle method, so that the edge of the adjacent uphill panel is on top of the edge of the adjacent downhill panel.

Plastic ties shall be installed to secure panels at a minimum spacing of 5 foot intervals along the length of the panel (longest dimension) and a minimum of 2 foot intervals along the end of the panels (shortest dimension).

The geonet shall not be dragged across a textured geomembrane. A slip sheet shall be installed between the geonet and textured geomembrane. The slip sheet must be removed when the geonet is in its final position.

The Contractor shall secure and protect installed sections of geonet by approved methods. The geosynthetic contractor shall not operate equipment or vehicles of any kind on the geonet. Any damaged sections of geonet caused by wind, weather, Contractor's activities, or any other means, shall be repaired or replaced at no additional cost to the Owner.

8-05.5(3) Geotextile

8-05.5(3)A Packaging, Labeling, Shipping, and Storage

Geotextile shall be packaged and shipped in protective wrapping, labeled with appropriate identification including manufacturer, fabric weight, and roll length.

The geotextile rolls shall be stored in its protective wrapping in a manner that protects the material from dirt, moisture, heat, and any other cause of damage.
8-05.5(3)B Installation

Geotextile shall be placed as shown on the project plans with overlaps a minimum of 12 inches or sewn, unless otherwise stated. Geotextile shall be laid smooth without excessive wrinkles and held in place by an approved method until covered. Type 3 geotextile will be placed over installed HDPE liner prior to installing protective cover soil.

8-05.5(3)C Repair

Torn geotextile shall be covered with a piece of the same material with at least 12 inch overlap all around.

8-05.6 Measurement

Geomembrane liner will be measured by the square foot actually installed for “Geomembrane Double Liner System” as measured parallel to the slope to the limits of liner entry in the anchor trench. The quantity installed in the anchor trench and the textured geomembrane overlay shall be incidental.

Geocell will be measured by the square foot of actually installed as measured for bank erosion support.

8-05.7 Payment

The unit contract price for “Geomembrane Double Liner System” shall include all costs for materials, labor, equipment, and tools required to furnish and install the 60 mil HDPE geomembrane primary liner, geocomposite drainage net, 60 mil HDPE geomembrane secondary liner and all attachments, embeds, and anchors for lined ponds.

The unit contract price for “Geotextile,” per square foot, shall include all costs to supply and install geotextile.

The unit contract price for "Geocell" per square foot shall include all costs to supply and install geocell and quarry spalls.

DIVISION 9
MATERIALS

9-05 DRAINAGE STRUCTURES, CULVERTS, AND CONDUITS

9-05.21 High Density Polyethylene (HDPE) Pipe and Fitting (New Section)

HDPE pipe shall meet the following requirements:
1. Pipe and fittings used for lysimeter collection and transfer piping shall be High Density Polyethylene (HDPE) pipe conforming to the following specifications:

a. Pipe sizing shall be in accordance with ASTM F 714 and ASTM D 3035.

b. The pipe shall be made from Premium High Density Polyethylene resin qualified as Type III, Category 5, Class C, Grade P 34 in ASTM D 1248 and a minimum SDR of 26 (i.e., SDR value of 26 or less).

c. The perforated pipe shall be shop fabricated from nominal diameter pipe size and shall be of basic configuration of holes or slots as follows:

1) Holes: shall have four 1/4-inch diameter holes, offset 90°, every 1 foot on-center for the entire length of perforated pipe as shown on the Plans.

2) Slots: shall be uniformly placed perpendicular to the axis of the pipe in straight rows. The slot shall be 40 slot (0.040) with a 0.025 spacing between slots as supplied by Aardvark, Puyallup, WA or approved equal.

d. This material shall have a long-term hydrostatic strength of 1,600 psi when tested and analyzed by ASTM D 2837, and listed by the Plastic Pipe Institute as P.E. 3408 resin.

e. The following minimum engineering design specifications are required:

   ASTM D 638 Tensile Strength Yield (2 in/min), ≥3,200 psi
   ASTM D 638 Elongation at break, 750 percent
   ASTM D 638 Modulus of Elasticity, 120,000 psi
   ASTM D 790 Flexural Modulus, 135,000 psi
   ASTM D 1693 Environmental stress crack resistance (E.S.C.R.)
   Condition C, >5,000° F, 20 hours
   ASTM D 2837 Long Term Strength (L.T.H.S.) at 73.4° Fahrenheit, 1,600 psi
f. In addition to the above, the High Density Polyethylene Material shall have
the following general characteristics:
ASTM D 1505 Density with carbon black, 0.955 g/cm 3 (min)
ASTM D 1238 Melt index (E) Condition, ≤ 0.14 g/10 min
ASTM D 1238 Melt index (F) Condition, ≤ 11.0 g/10 min
ASTM D 1525 Vicat softening point, 257° Fahrenheit (min)
ASTM D 746 Brittleness temperature, < -180° Fahrenheit (max)
ASTM C 177 Thermal conductivity, 2.7 BTU, in/ft² hrs./degrees Fahrenheit
ASTM D 696 Thermal expansion, 1.2 x 10-4 in/in/degrees Fahrenheit (max)
ASTM D 2240 Hardness shore "D", 65
ASTM D 3350 Cell Class, 345434C
Resin to be N.S.F. listed

g. The pipe shall contain no recycled compound except that generated on the
manufacturer's own plant from resin of the same specification from the same
raw supplier.

h. The HDPE pipe shall be homogenous throughout and free from visible
cracks, holes, foreign inclusions, or other injurious defects. The pipe shall
be uniform in color, opacity, density, and other physical properties. The
following information shall be continuously marked on the pipe or spaced at
intervals not exceeding 5 feet:

1) Name and/or trademark of the pipe manufacturer
2) Nominal pipe size
3) Standard Dimension Ratio (SDR)
4) PE 3408
5) Manufacturer's Standard Reference
6) A production code from which the date and place of manufacturer can
   be determined.

i. Polyethylene compound shall be protected against ultraviolet degradation by
carbon black in concentrations of not less than 2 percent.

j. Flanges and blind flanges shall consist of a polyethylene flange adapter
(ribbed face) fused to each stubout of pipe, with a hot-dipped galvanized
finish convoluted ductile iron backup ring (Class 150# ANSI B16.5).

k. Flange bolts shall conform to material requirements of ASTM A 307 Grade B
with ANSI B 18.2.1 standard hex head pattern, ANSI B 1.1 coarse thread,
Class 2 ft. Nuts shall meet the requirements of ASTM A 307,
ANSI B 18.2.2 standard hex head pattern ANSI B 1.1 coarse thread and
have a Class 2B ft. Flat washers shall be provided with each nut for
protection of flanges. All bolting materials shall be hot-dip galvanized per
ASTM A 153.
1. Blind flange (above ground – see Plans) shall be of HDPE material and shall be of nominal size and thickness.

m. Gaskets shall be full-faced, 1/8-inch thick neoprene for HDPE to HDPE, and HDPE to steel application.

9-05.22 Carbon Steel Pipe (New Section)

Steel pipe shall meet Section 9.06 and the following requirements:

1. 6-inch nominal diameter, Schedule 40, Grade B seamless pipe per ASTM A 53.

2. Guard posts shall conform to the details as shown on the Plans.

3. "Galvanized" shall mean hot-dipped galvanized per ASTM A 153 and A 123.

Add the following subsection:

(******)

9-05.23 Septage Discharge Pipe

Pipe and fittings shall meet the following requirements:

1. Flexible hose used for septage discharge station shall be Heavy Duty Suction Hose. The flexible hose shall have PVC helical coils construction which provides a smooth bore. The size shall be of 6-inch nominal size and shall have vacuum rating of 28 inches mercury, working pressure of 33 psi, and bending a minimum bending radius of 15 inches, at 72° F.

2. Fittings shall be aluminum quick disconnect cam operating couplings with EPT gaskets standard. Fitting shall be receive hose and capable of connecting to suction hose. Each fitting shall be connected to suction hose with a galvanized power lock clamp.

9-14 EROSION CONTROL AND ROADSIDE PLANTING

(******)

9-14.2 Seed

Section 9-14.2 is supplemented with the following:

The seed shall be applied at the rate of 30 pounds per acre and shall consist of following:

- Certified Covar Sheep Fescue – 40 percent by weight
- Certified Siberian P-27 Wheatgrass – 40 percent by weight
- Certified Critana Thickspike – 20 percent by weight
Weed seed, inert material and other crop seed not to exceed 6 percent by weight.

9-14.3 Fertilizer

Supplement this section with the following:

(******)

Fertilizer shall be a slow release 22.5/0/10 mixture with 70 percent of the nitrogen being derived from urea or ureaformaldehyde. The fertilizer shall be applied at the rate of 20 pounds per acre. In addition, gypsum and/or other amendment shall be applied at the rate recommended by the seed and fertilizer supplier or as directed by the Engineer.

Add new section:

9-14.3(1) Application

(******)
The Contractor shall utilize the following sequence for application of the seed and fertilizer materials:

- Grade the area to receive the application to finish grade.
- Apply fertilizer mixture.
- Harrow and pack the area.
- Grain drill the seed.

The drill spacing shall be 6 inches. If practical, it is also acceptable to use 12 inch spacing with two perpendicular passes to each other.

This seed mixture can be planted in the fall or spring, with fall planting after September 30 preferred.

9-14.4 Mulch and Amendments

Section 9-14.4 is supplemented with the following:

(******)

Wood cellulose fiber mulch shall be applied at the rate of 2,000 pounds per acre.

9-14.5 Erosion Control Devices

9-14.5(2) Erosion Control Blanket

This section is revised to read:

(******)
Erosion control blanket shall be manufactured from 100 percent mattress grade coconut fiber, the matting shall be a machine fabricated mat, covered on both sides by netting, and sewn together on two inch centers. Coconut fiber shall be homogeneously blended and evenly distributed throughout the mat. Netting shall be polypropylene, treated with UVI inhibitors to resist photodegrading, with mesh openings of approximately 5/8" x 5/8". The mat shall be sewn on approximately two inch centers with minimum 1,000 denier UVI treated black polypropylene thread.

9-30 WATER DISTRIBUTION MATERIALS

9-30.3 Valves

Add the following subsection:

(*****)

9-30.3(2) Decant Valves

Decant valve shall be rising stem, slip tube style valve as shown on the Drawings. The valve shall consist of floor stand, actuator rod, handwheel slip tube, mounting bracket, and equipment fasteners of the following materials:

Mounting Bracket – Hot dip galvanized
Floor Stand – Hot dip galvanized
Handwheel – Stainless steel 18-inch diameter
Actuator Rod – Stainless steel 1–1/8-inch diameter ACME threaded rod attached to slip tube bail
Slip Tube – Stainless steel minimum 1/8 inch wall thickness
Neoprene Gasket
Equipment Fasteners – Stainless steel
Contact: Dave Myers (704) 554-8397

9-33 CONSTRUCTION GEOSYNTHETIC

Section 9-33 is supplemented with the following:

(*****)

Geotextile

Geotextile shall be a needle punched, nonwoven pervious sheet of polymeric yarn as defined by ASTM D 123. Geotextile fiber shall consist of long-chain polymers composed of at least 85 percent by weight of polypropylene, polyester, polyethylene, nylon, or polyvinylidene-chloride. Stabilizers and/or inhibitors shall be added to the base polymer if necessary to make the filaments resistant to deterioration by ultraviolet and heat exposure. The geotextiles shall be constructed so that the filaments will retain their relative position with respect to each other. The edges of the geotextiles shall be sealed or otherwise finished to prevent the outer material from pulling away from the geotextile or raveling. During all periods of shipment and storage, the
geotextiles shall be protected from direct sunlight, ultraviolet rays, temperatures greater than 140 degrees Fahrenheit, mud, dirt, dust, and trash. To the extent possible, the geotextiles shall be maintained wrapped in a heavy duty protective covering. Non-woven geotextiles for separation shall meet the minimum physical requirement listed in Table 1 below for Type 3. Non-woven geotextile for filtration shall meet the minimum physical requirement listed in Table 1 below for Type 1.

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Test Values</th>
</tr>
</thead>
<tbody>
<tr>
<td>Weight, oz./sq. yd., min.</td>
<td>ASTM D 3776</td>
<td>5.5</td>
</tr>
<tr>
<td>Tensile Strength, lbs., in any principal direction</td>
<td>ASTM D 4632</td>
<td>130</td>
</tr>
<tr>
<td>Grab Elongation, %, in any principal direction</td>
<td>ASTM D 4632</td>
<td>50</td>
</tr>
<tr>
<td>Puncture Strength, lbs., min.</td>
<td>ASTM D 4833</td>
<td>60</td>
</tr>
<tr>
<td>Water Permeability, cm/sec, min.</td>
<td>ASTM D 4491</td>
<td>0.40</td>
</tr>
<tr>
<td>Permittivity, 1/sec</td>
<td>ASTM D 4491</td>
<td>2.3</td>
</tr>
<tr>
<td>Apparent Opening Size (AOS), U.S. Standard Sieve, max. opening size</td>
<td>ASTM D 4751</td>
<td>80-40</td>
</tr>
<tr>
<td>Wide Width Tensile, lbs/in</td>
<td>ASTM D 4595</td>
<td>65</td>
</tr>
<tr>
<td>Wide Width Elongation, %</td>
<td>ASTM D 4595</td>
<td>&gt;50</td>
</tr>
</tbody>
</table>

**Geomembrane**

**Geomembrane Raw Materials**

The geomembrane shall be manufactured of new, first-quality resin and shall be compounded and manufactured specifically for the intended purpose. The resin manufacturer shall certify each batch for the following properties.

The HDPE geomembrane shall be 60-mil thick and conform to the requirements of the National Sanitation Foundation (NSF) Standard 54.
The High Density Polyethylene (Compounded) resin shall meet the following specifications:

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Specific Gravity</td>
<td>(ASTM D 792 or ASTM D 1505)</td>
<td>&gt;0.940</td>
</tr>
<tr>
<td>Melt Index</td>
<td>(ASTM D 1238 Condition E)</td>
<td>&lt;0.4 g/10 min.</td>
</tr>
<tr>
<td>Carbon Black Content</td>
<td>(ASTM D 1603)</td>
<td>2 – 3%</td>
</tr>
</tbody>
</table>

Rolls

The geomembrane rolls shall be labeled to identify the thickness of the material, roll length and width, roll batch and number, and name of manufacturer.

The surface of the geomembrane shall be free of holes, blisters, undispersed raw materials, or any contamination by foreign matter; except that if in the opinion of the Engineer and blemish will not adversely affect properties and use of geomembrane, the Engineer may accept the geomembrane after sufficient laboratory test data are provided to support such acceptance, and further, provided all such testing is done at the sole expense of the Contractor.

Textured geomembrane  The rolls shall meet the following properties:

TYPICAL PROPERTIES: 60-mil

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Value*</th>
</tr>
</thead>
<tbody>
<tr>
<td>Roll width, feet, min.</td>
<td>ASTM D 5994</td>
<td>23</td>
</tr>
<tr>
<td>Thickness, mils, min.</td>
<td>ASTM D 1505</td>
<td>57</td>
</tr>
<tr>
<td>Density (g/cc), min.</td>
<td>ASTM D 1238</td>
<td>0.94</td>
</tr>
<tr>
<td>Melt Index (g/10min., max.)</td>
<td>ASTM D 4218</td>
<td>≤1.0</td>
</tr>
<tr>
<td>Carbon Black Content (%)</td>
<td>ASTM D 5596</td>
<td>2-3</td>
</tr>
<tr>
<td>Carbon Black Dispersion</td>
<td>ASTM D 638</td>
<td>A-2</td>
</tr>
<tr>
<td>Tensile Properties</td>
<td>Type IV specimen</td>
<td>132</td>
</tr>
<tr>
<td>1. Tensile Strength at Yield (pounds/inch width)</td>
<td>at 2 inches/minute</td>
<td></td>
</tr>
<tr>
<td>2. Tensile Strength at Break (pounds/inch width)</td>
<td></td>
<td>132</td>
</tr>
<tr>
<td>3. Elongation at Yield (%)</td>
<td></td>
<td>13</td>
</tr>
<tr>
<td>4. Elongation at Break (%)</td>
<td></td>
<td>350</td>
</tr>
<tr>
<td>5. Modulus of Elasticity</td>
<td></td>
<td>80,000</td>
</tr>
<tr>
<td>(1% secant; pounds/square inch)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Tear Strength (lbs.)</td>
<td>ASTM D 1004 Die C</td>
<td>45</td>
</tr>
<tr>
<td>Puncture Resistance</td>
<td>ASTM D 4633</td>
<td>120</td>
</tr>
<tr>
<td>Environmental Stress Crack (hours)</td>
<td>ASTM D 5397</td>
<td>300</td>
</tr>
<tr>
<td>(Single Point NCTC)</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>
* All values, except when specified as minimum or maximum, represent average lot property values.

** Federal Test Method Standards.

** Geomembrane Penetration**

Pipe penetrations through the geomembrane shall be sealed with a geomembrane boot as shown on the Plans. The boot shall be field or shop fabricated and shall be made to seal around the pipe without folds. The flange portion of the boot shall match the angle between the slope and the pipe for a smooth fit without excess folds or stretching of the material. The boot shall be attached to the pipe with two stainless steel T-bolt type band clamps, cushion strips, and sealant tape as recommended by the geomembrane manufacturer and approved by the Engineer. The boot shall be extrusion welded to the geomembrane liner as shown on the Plans.

Rubber cushion strip shall be 1/8-inch by 2-inch EPDM potable grade cured live rubber with 1/4-inch by 1-inch slots punched with the same openings as the metal batten.

Sealant tape shall be butyl tape 2-inch by 1/8-inch.

** Geonet**

The polymer used to manufacture the geonet shall be non-thermally degraded polyethylene which is clean and free of any foreign contaminants. The manufactured geonet shall conform to the property requirements listed in Table 2 and shall be free of defects including tears, nodules, or other manufacturing defects which may affect its serviceability.

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Test Value</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Polymer Density</strong>, minimum</td>
<td>ASTM D 1505</td>
<td>0.930 g/cc</td>
</tr>
<tr>
<td><strong>Polymer Melt Index</strong>, maximum</td>
<td>ASTM D 1238</td>
<td>1.1 g/10 min.</td>
</tr>
<tr>
<td><strong>Carbon Black Content</strong></td>
<td>ASTM D 4218</td>
<td>2-3 percent</td>
</tr>
<tr>
<td><strong>Transmissivity, minimum</strong></td>
<td>ASTM D 4716</td>
<td>$5 \times 10^{-3}$ sq. ft./sec</td>
</tr>
</tbody>
</table>

Table 2
Geonet Physical Properties

Note: Transmissivity shall be measured using water at 68°F with a maximum gradient of 0.25 under a normal pressure of 1,000 psf. Geotextiles shall be attached to the geonet in the same configuration as will be used in the field for transmissivity testing. The drainage net shall be sandwiched between the Contractor selected subgrade embankment soil and geomembrane on the bottom and cover soil on the top. A minimum seating period of 15 minutes shall be used.

A geonet shall be created by heat bonding geotextile, Type 3 (see Section 9-33.1) to one side of the geonet with ply adhesion meeting the requirements of ASTM D 413. The bond between the geotextile and the geonet shall exhibit a minimum peel strength of 2 pounds per inch.

** Geocell**
Geocell shall be a lightweight, polyethylene, mat made up of ultrasonically welded strips to form an extremely strong honeycomb mat having the following properties:

<table>
<thead>
<tr>
<th>Expanded Dimensions</th>
<th>Panel Cell Area</th>
<th>8' x 20' 8&quot; x 9.6&quot; nominal</th>
</tr>
</thead>
<tbody>
<tr>
<td>Collapsed Dimension</td>
<td></td>
<td>11' x 5&quot; x cell height</td>
</tr>
<tr>
<td>Color</td>
<td>Standard</td>
<td>Black</td>
</tr>
<tr>
<td>Seams Tensile Peel Strength</td>
<td>COE GL-86-19</td>
<td>640 lbs.</td>
</tr>
<tr>
<td>Seam Hang Strength</td>
<td>Per 100 mm seam shall sustain 72.5 kg weight for at least 7 days, undergoing the variances from room temperature to 54°C on one hour cycle.</td>
<td></td>
</tr>
<tr>
<td>Polyethylene Thickness</td>
<td>ASTM D 5199</td>
<td>50 mils ±5%</td>
</tr>
<tr>
<td>Typical Weight</td>
<td></td>
<td>106 lbs.</td>
</tr>
<tr>
<td>Installation Temperature Range</td>
<td>-16° F to +110°F</td>
<td></td>
</tr>
<tr>
<td>Carbon Black Content</td>
<td>ASTM D 4218</td>
<td>2 - 3%</td>
</tr>
<tr>
<td>Density (MARV)</td>
<td>ASTM D 1505</td>
<td>.94 g/cm³</td>
</tr>
<tr>
<td>Environmental Stress Crack Resistance</td>
<td>&gt;4,000 hrs</td>
<td></td>
</tr>
</tbody>
</table>

(*****)

LEACHATE LIFT PUMP

A sump pump shall be installed to lift water from beneath the primary liner back to the top of the lagoon cell.

The pump shall be operated by 1/3 Hp shaded pole motor, cast iron pump housing, and have the following construction:

**Construction**

<table>
<thead>
<tr>
<th>Cover</th>
<th>Polycarbonate</th>
</tr>
</thead>
<tbody>
<tr>
<td>Motor Housing</td>
<td>Epoxy-coated cast iron</td>
</tr>
<tr>
<td>Impeller Material</td>
<td>Nylon</td>
</tr>
<tr>
<td>Impeller Type</td>
<td>Vortex</td>
</tr>
<tr>
<td>Volute</td>
<td>Epoxy-coated cast iron/polypropylene</td>
</tr>
<tr>
<td>Power Cord</td>
<td>SJTW-A</td>
</tr>
<tr>
<td>Mechanical Shaft Seal</td>
<td>Nitrile with carbon and ceramic faces</td>
</tr>
<tr>
<td>Fasteners</td>
<td>Stainless steel</td>
</tr>
<tr>
<td>Shaft</td>
<td>Cold rolled steel</td>
</tr>
<tr>
<td>Bearings</td>
<td>Upper and lower sintered sleeve bearings</td>
</tr>
</tbody>
</table>

Pump flow rate shall be 10 gpm at 17 feet of TDH. Automatic operation will be adjustable with 7 inch to 10 inch "On" level and 2 inch to 5 inch "Off" level. Power cord to match system
electrical power as required. Pump to be as manufactured by Little Giant Model 6E-CIA-SFS or approved equal.

(******)

ILLUMINATION AND ELECTRIC

Illumination and electric shall be according to WSDOT Standard Specifications, Division 9-29. Measurement shall be lump sum for a complete electric and illumination system to provide power to the new disconnect, overhead illumination at the septage discharge station, and power to each of the four leachate lift pumps. Payment for illumination and electric system will be on a lump sum basis. The lump sum contract price shall be full pay for all costs in connection with lighting, disconnects, junction boxes, pull boxes, conduit, wire and all appurtenances for a complete system.

STANDARD PLANS
April 13, 2009

The State of Washington Standard Plans for Road, Bridge and Municipal Construction M21-01 transmitted under Publications Transmittal No. PT 09-013, effective April 6, 2009 is made a part of this contract.

The Standard Plans are revised as follows:

All Standard Plans
All references in the Standard Plans to "Asphalt Concrete Pavement" shall be revised to read "Hot Mix Asphalt".

All references in the Standard Plans to the abbreviation "ACP" shall be revised to read "HMA".

B-10.20 and B10.40
Substitute “step” in lieu of “handhold” on plan

C-1b
In the ANCHOR POST ASSEMBLY, the above ground 7 1/2” long bolt connecting the Wood Breakaway Post to the Foundation Tube is revised to 10” long.

C-2r
DELETED

C-2s
DELETED

C-2t
DELETED
Note 1 is revised as follows: replace reference F-2b with F-10.42

C-4a
DELETED

C-5
In the A CONNECTION, “Type 3 transition pay limit” is revised to “transition pay limit”.

C-10 (sheet 2 of 2)
COVER PLATE DETAIL, dimension of the 1” dia. holes, changes from 8” to 3”

C-11c
DELETED

F-40.12 through F-40.18
The following note is added to these five plans:

Note 7. To the maximum extent feasible, the ramp cross slope shall not exceed 2%.

G-9a
DELETED

J-6f
DELETED

J-6g
DELETED

J-6h
DELETED

J-11a
DELETED

J-11c
DELETED

J-15a
DELETED

J-15b
DELETED

K-80.30-00
In the NARROW BASE, END view, the reference to Std. Plan C-8e is revised to Std. Plan K-80.35

L-20.10-00, Sheet 1
Delete all references to tension cable and substitute tension wire.
Add knuckled selvage is required on the top edge of the fence fabric.

L-20.10-00, Sheet 2
Delete all references to tension cable and substitute tension wire.
All rope thimbles, wire rope clips and seizing are not required.

L-30.10-00, Sheet 1
Delete all references to tension cable and substitute tension wire.

L-30.10-00, Sheet 2
Delete all references to tension cable and substitute tension wire.
All rope thimbles, wire rope clips and seizing are not required.

M-1.60
COLLECTOR DISTRIBUTOR ROAD OFF- CONNECTION, taper dimensions of 225° MIN. is changed to 300° MIN.

The following are the Standard Plan numbers applicable at the time this project was advertised. The date shown with each plan number is the publication approval date shown in the lower right-hand corner of that plan. Standard Plans showing different dates shall not be used in this contract.

A-10.10-00......8/07/07 A-30.30-00......11/08/07 A-50.20-00......11/17/08
A-10.20-00......10/05/07 A-30.35-00......10/12/07 A-50.30-00......11/17/08
A-10.30-00......10/05/07 A-40.10-00......10/05/07 A-50.40-00......11/17/08
A-20.10-00......8/31/07 A-40.20-00......9/20/07 A-60.10-00......10/05/07
A-30.10-00......11/08/07 A-40.50-00......11/08/07 A-60.20-00......10/05/07
A-30.15-00......11/08/07 A-50.10-00......11/17/08 A-60.30-00......11/08/07
A-60.40-00......8/31/07

B-5.20-00......6/01/06 B-30.50-00......6/01/06 B-75.20-01......6/10/08
B-5.40-00......6/01/06 B-30.70-01......8/31/07 B-75.50-01......6/10/08
B-5.60-00......6/01/06 B-30.80-00......6/08/06 B-75.60-00......6/08/06
B-10.20-00......6/01/06 B-30.90-01......9/20/07 B-80.20-00......6/08/06
B-10.40-00......6/01/06 B-35.20-00......6/08/06 B-80.40-00......6/01/06
B-10.60-00......6/08/06 B-35.40-00......6/08/06 B-82.20-00......6/01/06
B-15.20-00......6/01/06 B-40.20-00......6/01/06 B-85.10-01......6/10/08
B-15.40-00......6/01/06 B-40.40-00......6/01/06 B-85.20-00......6/01/06
B-15.60-00......6/01/06 B-45.20-00......6/01/06 B-85.30-00......6/01/06
B-20.20-01......11/21/06 B-45.40-00......6/01/06 B-85.40-00......6/08/06
B-20.40-02......6/10/08 B-50.20-00......6/01/06 B-85.50-01......6/10/08
| C-1 | 5/30/97 | C-1d | 10/31/03 | C-1a | 2/10/09 | C-1c | 5/30/97 | C-1b | 10/31/03 | C-1e | 6/21/06 | C-2b | 6/21/06 | C-2f | 3/14/97 | C-2c | 7/27/01 | C-2a | 6/21/06 | C-2d | 6/21/06 | C-2e | 6/21/06 | C-2 | 10/31/03 | C-2g | 7/27/01 | C-2h | 3/28/97 | C-2 | 10/31/03 | C-3 | 10/30/03 | C-3a | 10/30/03 | C-3b | 10/30/03 | C-3c | 10/30/03 | C-4 | 2/21/07 | C-4b | 6/08/06 |
|-----|--------|-----|----------|-----|--------|-----|--------|-----|----------|-----|--------|-----|--------|-----|--------|-----|--------|-----|----------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|--------|-----|
| B-20.60-02 | 6/10/08 | B-55.20-00 | 6/01/06 | B-90.10-00 | 6/08/06 |
| B-25.20-00 | 6/08/06 | B-60.20-00 | 6/08/06 | B-90.20-00 | 6/08/06 |
| B-25.60-00 | 6/01/06 | B-60.40-00 | 6/01/06 | B-90.30-00 | 6/08/06 |
| B-30.10-00 | 6/08/06 | B-65.20-00 | 6/01/06 | B-90.40-00 | 6/08/06 |
| B-30.20-01 | 11/21/06 | B-65.40-00 | 6/01/06 | B-90.50-00 | 6/08/06 |
| B-30.30-00 | 6/01/06 | B-70.20-00 | 6/01/06 | B-95.20-01 | 2/03/09 |
| B-30.40-00 | 6/01/06 | B-70.60-00 | 6/01/06 | B-95.40-00 | 6/08/06 |
| C-1 | 2/10/09 | C-4e | 2/20/03 | C-14i | 2/10/09 | C-1 | 10/31/03 | C-5 | 10/31/03 | C-14k | 2/10/09 | C-1 | 5/30/97 | C-6 | 5/30/97 | C-15a | 7/3/08 | C-1d | 3/14/97 | C-15b | 7/3/08 | C-2 | 10/31/03 | C-6c | 10/31/03 | C-16a | 11/08/05 | C-2a | 5/30/97 | C-6d | 5/30/97 | C-16b | 11/08/05 | C-2b | 7/25/97 | C-6f | 7/25/97 | C-17 | 11/08/05 |
| C-2c | 7/27/01 | C-7 | 10/31/03 | C-18 | 2/10/09 | C-2d | 6/21/06 | C-7a | 10/31/03 | C-19 | 2/10/09 | C-2e | 6/21/06 | C-8 | 2/10/09 | C-2f | 7/25/97 | C-8a | 2/10/09 | C-2g | 7/27/01 | C-8b | 2/10/09 | C-2h | 2/10/09 | C-8e | 2/10/09 | C-2i | 6/30/04 | C-8f | 6/30/04 | C-3 | 10/31/03 | C-10 | 7/31/98 |
| C-2j | 6/12/98 | C-11 | 7/31/98 | C-12 | 7/3/08 | C-2k | 7/27/01 | C-13 | 7/3/08 | C-13a | 7/3/08 | C-2n | 7/27/01 | C-13b | 7/3/08 | C-14 | 7/3/08 | C-14a | 7/3/08 | C-2o | 7/13/01 | C-14b | 2/03/09 | C-14c | 7/3/08 | C-2p | 7/13/01 | C-14d | 7/3/08 | C-14e | 7/3/08 | C-2q | 7/13/01 | C-14f | 7/3/08 | C-14g | 7/3/08 | C-2r | 7/13/01 | C-14h | 7/3/08 |

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PREVAILING WAGE RATES
## Washington State Prevailing Wage Rates For Public Works Contracts

The **PREVAILING WAGES** listed here include both the hourly wage rate and the hourly rate of fringe benefits. On public works projects, workers' wage and benefit rates must add to not less than this total. A brief description of overtime calculation requirements is provided on the Benefit Code Key.

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**EFFECTIVE 03-04-2009**

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<td>BUMP CUTTER</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
</tr>
<tr>
<td>CRANES, 45 TONS - 99 TONS, UNDER 150 FT OF BOOM (INCLUDING JIB WITH ATTACHMENTS)</td>
<td>$47.91</td>
<td>1T</td>
<td>5D</td>
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<tr>
<td>CRANES, 100 TONS - 199 TONS, OR 150 FT OF BOOM (INCLUDING JIB WITH ATTACHMENTS)</td>
<td>$48.46</td>
<td>1T</td>
<td>5D</td>
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<td>CRANES, 200 TONS TO 300 TONS, OR 250 FT OF BOOM (INCLUDING JIB WITH ATTACHMENTS)</td>
<td>$49.03</td>
<td>1T</td>
<td>5D</td>
<td>8P</td>
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<tr>
<td>CRANES, A-FRAME, 10 TON AND UNDER</td>
<td>$44.64</td>
<td>1T</td>
<td>5D</td>
<td>8P</td>
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<tr>
<td>CRANES, A-FRAME, OVER 10 TON</td>
<td>$47.00</td>
<td>1T</td>
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<td>CRANES, OVER 300 TONS, OR 300' OF BOOM INCLUDING JIB WITH ATTACHMENTS</td>
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<td>CRANES, OVERHEAD, BRIDGE TYPE (20 - 44 TONS)</td>
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<td>CRANES, OVERHEAD, BRIDGE TYPE (45 - 99 TONS)</td>
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<td>5D</td>
<td>8P</td>
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<td>CRANES, OVERHEAD, BRIDGE TYPE (100 TONS &amp; OVER)</td>
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<td>1T</td>
<td>5D</td>
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<tr>
<td>CRANES, TOWER CRANE UP TO 175' IN HEIGHT, BASE TO BOOM</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>CRANES, TOWER CRANE OVER 175' IN HEIGHT, BASE TO BOOM</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>CRUSHERS</td>
<td>$47.42</td>
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<td>8P</td>
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<td>DECK ENGINEER/DECK WINCHES (POWER)</td>
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<td>1T</td>
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<tr>
<td>DERRICK, BUILDING</td>
<td>$47.91</td>
<td>1T</td>
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<td>DOZERS, D-9 &amp; UNDER</td>
<td>$47.00</td>
<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>DRILL OILERS - AUGER TYPE, TRUCK OR CRANE MOUNT</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>8P</td>
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<tr>
<td>ELEVATOR AND MANLIFT, PERMANENT AND SHAFT-TYPE</td>
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<tr>
<td>EQUIPMENT SERVICE ENGINEER (OILER)</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>FINISHING MACHINE/BIDWELL GAMACO AND SIMILAR EQUIP</td>
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<td>FORK LIFTS, (3000 LBS AND OVER)</td>
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<td>GRADE ENGINEER</td>
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<td>1T</td>
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<td>GRADE CHECKER AND STAKEMAN</td>
<td>$44.64</td>
<td>1T</td>
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<td>GUARDRAIL PUNCH</td>
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<td>HOISTS, OUTSIDE (ELEVATORS AND MANLIFTS), AIR TUGGERS</td>
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<td>HORIZONTAL/DIRECTIONAL DRILL LOCATOR</td>
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<td>HYDRAULIFTS/BOOM TRUCKS (10 TON &amp; UNDER)</td>
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<td>1T</td>
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<td>1T</td>
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<td>LOADERS, OVERHEAD (6 YD UP TO 8 YD)</td>
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<td>LOCOMOTIVES, ALL</td>
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<td>MECHANICS, ALL</td>
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<td>MIXERS, ASPHALT PLANT</td>
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<td>8P</td>
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<tr>
<td>MOTOR PATROL GRADER (FINISHING)</td>
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<tr>
<td>MOTOR PATROL GRADER (NON-FINISHING)</td>
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<td>1T</td>
<td>5D</td>
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<tr>
<td>MUCKING MACHINE, MOLE, TUNNEL DRILL AND/OR SHIELD</td>
<td>$47.91</td>
<td>1T</td>
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<td>8P</td>
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<td>OIL DISTRIBUTORS, BLOWER DISTRIBUTION AND MULCH SEEDING OPERATOR</td>
<td>$44.64</td>
<td>1T</td>
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<td>PAVEMENT BREAKER</td>
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<td>5D</td>
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<td>PILE DRIVER (OTHER THAN CRANE MOUNT)</td>
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<td>1T</td>
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<td>PLANT OILER (ASPHALT, CRUSHER)</td>
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<td>POSTHOLE DIGGER, MECHANICAL</td>
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<td>8P</td>
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<td>POWER PLANT</td>
<td>$44.64</td>
<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>PUMPS, WATER</td>
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<td>Classification</td>
<td>Prevailing Wage</td>
<td>Time Code</td>
<td>Holiday Code</td>
<td>Note</td>
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<tr>
<td>--------------------------------------------------------------------------------</td>
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<tr>
<td>QUAD 9, D-10, AND HD-41</td>
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<td>QUICK TOWER-NO CAB, UNDER 100 FEET IN HEIGHT BASED TO BOOM</td>
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<td>8P</td>
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<td>REMOTE CONTROL OPERATOR ON RUBBER TIRED EARTH MOVING EQUIP</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<td>RIGGER AND BELLMAN</td>
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<td>5D</td>
<td>8P</td>
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<td>ROLLAGON</td>
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<td>ROLLER, OTHER THAN PLANT ROAD MIX</td>
<td>$44.64</td>
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<tr>
<td>ROLLERS, PLANTMIX OR MULTILIFT MATERIALS</td>
<td>$47.91</td>
<td>1T</td>
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<tr>
<td>ROTO-MILL, ROTO-GRINDER</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<tr>
<td>SAWS, CONCRETE</td>
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<td>1T</td>
<td>5D</td>
<td>8P</td>
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<tr>
<td>SCRAPERS - SELF PROPELLED, HARD TAIL END DUMP, ARTICULATING</td>
<td>$47.91</td>
<td>1T</td>
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<tr>
<td>OFF-ROAD EQUIPMENT (45 YD AND OVER)</td>
<td>$47.91</td>
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<td>SCRAPERS, CONCRETE AND CARRY ALL</td>
<td>$47.91</td>
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<td>8P</td>
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<tr>
<td>SCREE MAN</td>
<td>$47.91</td>
<td>1T</td>
<td>5D</td>
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<td>SHOTCRETE GUNITE</td>
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<td>SLIFFFORM PAVERS</td>
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<td>8P</td>
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<td>SPREADER, TOPSIDER &amp; SCREAMAN</td>
<td>$47.91</td>
<td>1T</td>
<td>5D</td>
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<td>SUBGRADE TRIMMER</td>
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<td>TOWER BUCKET ELEVATORS</td>
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<td>TRACTORS, (75 HP &amp; UNDER )</td>
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<td>TRACTORS, (OVER 75 HP)</td>
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<td>TRANSFER MATERIAL SERVICE MACHINE</td>
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<td>TRANSPORTERS, ALL TRACK OR TRUCK TYPE</td>
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<td>5D</td>
<td>8P</td>
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<td>TRENCHING MACHINES</td>
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<td>TRUCK CRANE OILER/DRIVER (UNDER 100 TON)</td>
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<td>TRUCK CRANE OILER/DRIVER (100 TON &amp; OVER)</td>
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<td>TRUCK MOUNT PORTABLE CONVEYER</td>
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<td>WHEEL TRACTORS, FARMALL TYPE</td>
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<td>YO YO PAY DOZER</td>
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<td>POWER EQUIPMENT OPERATORS- UNDERGROUND SEWER &amp; WATER</td>
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<td>4A</td>
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<td>(SEE POWER EQUIPMENT OPERATORS)</td>
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<td>POWER LINE CLEARANCE TREE TRIMMERS</td>
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<td>JOURNEY LEVEL IN CHARGE</td>
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<td>SPRAY PERSON</td>
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<td>TREE TRIMMER</td>
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<td>MECHANIC</td>
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<td>Time Code</td>
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<tr>
<td>RESIDENTIAL INSULATION APPLICATORS</td>
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<td>JOURNEY LEVEL</td>
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<td>JOURNEY LEVEL</td>
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<td>RESIDENTIAL PAINTERS</td>
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<td>JOURNEY LEVEL</td>
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<td>RESIDENTIAL PLUMBERS &amp; PIPEFITTERS</td>
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<td>JOURNEY LEVEL</td>
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<td>JOURNEY LEVEL (FIELD OR SHOP)</td>
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<td>RESIDENTIAL SOFT FLOOR LAYERS</td>
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<td>JOURNEY LEVEL</td>
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<td>RESIDENTIAL TERRAZZO TILE FINISHERS</td>
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<td>JOURNEY LEVEL</td>
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<td>ROOFERS</td>
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<tr>
<td>USING IRRITABLE BITUMINOUS MATERIALS</td>
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<td>SHEET METAL WORKERS</td>
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<td>JOURNEY LEVEL (FIELD OR SHOP)</td>
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<td>SIGN MAKERS &amp; INSTALLERS (ELECTRICAL)</td>
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<td>JOURNEY LEVEL</td>
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<td>SIGN MAKERS &amp; INSTALLERS (NON-ELECTRICAL)</td>
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<td>JOURNEY LEVEL</td>
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<td>SOFT FLOOR LAYERS</td>
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<td>JOURNEY LEVEL</td>
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<td>JOURNEY LEVEL</td>
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<td>2B</td>
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<td>TERRAZZO WORKERS &amp; TILE SETTERS</td>
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BENEFIT CODE KEY - EFFECTIVE 03-04-2009

OVERTIME CODES

OVERTIME CALCULATIONS ARE BASED ON THE HOURLY RATE ACTUALLY PAID TO THE WORKER. ON PUBLIC WORKS PROJECTS, THE HOURLY RATE MUST BE NOT LESS THAN THE PREVAILING RATE OF WAGE MINUS THE HOURLY RATE OF THE COST OF FRINGE BENEFITS ACTUALLY PROVIDED FOR THE WORKER.

1. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

A. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS SHALL ALSO BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

B. ALL HOURS WORKED ON SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SATURDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

C. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TEN (10) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER OVERTIME HOURS WORKED SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

D. THE FIRST TWO (2) HOURS BEFORE OR AFTER A FIVE - EIGHT (8) HOUR WORKWEEK DAY OR A FOUR - TEN (10) HOUR WORKWEEK DAY AND THE FIRST EIGHT (8) HOURS WORKED THE NEXT DAY AFTER EITHER WORKWEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL ADDITIONAL HOURS WORKED AND ALL WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

E. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST EIGHT (8) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER HOURS WORKED MONDAY THROUGH SATURDAY, AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

F. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TEN (10) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER OVERTIME HOURS WORKED, EXCEPT LABOR DAY, SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT THREE TIMES THE HOURLY RATE OF WAGE.

G. THE FIRST TEN (10) HOURS WORKED ON SATURDAYS AND THE FIRST TEN (10) HOURS WORKED ON A FIFTH CALENDAR WEEKDAY IN A FOUR - TEN HOUR SCHEDULE, SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF TEN (10) HOURS PER DAY MONDAY THROUGH SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

H. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS IF WORK IS LOST DUE TO INCLEMENT WEATHER CONDITIONS OR EQUIPMENT BREAKDOWN) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED MONDAY THROUGH SATURDAY OVER TWELVE (12) HOURS AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

I. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

J. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TEN (10) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED OVER TEN (10) HOURS MONDAY THROUGH SATURDAY, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

K. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

L. ALL HOURS WORKED IN EXCESS OF TEN (10) HOURS PER DAY MONDAY THROUGH SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

M. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS IF WORK IS LOST DUE TO INCLEMENT WEATHER CONDITIONS) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

N. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

O. THE FIRST TEN (10) HOURS WORKED ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS, HOLIDAYS AND AFTER TWELVE (12) HOURS, MONDAY THROUGH FRIDAY, AND AFTER TEN (10) HOURS ON SATURDAY SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

P. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS IF CIRCUMSTANCES WARRANT) AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.
1. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND UP TO TEN (10) HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF TEN (10) HOURS PER DAY MONDAY THROUGH SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS (EXCEPT CHRISTMAS DAY) SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON CHRISTMAS DAY SHALL BE PAID AT TWO AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

R. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

S. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST EIGHT (8) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER OVERTIME HOURS WORKED, EXCEPT LABOR DAY, SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT THREE TIMES THE HOURLY RATE OF WAGE.

T. WORK PERFORMED IN EXCESS OF EIGHT (8) HOURS OF STRAIGHT TIME PER DAY, OR TEN (10) HOURS OF STRAIGHT TIME PER DAY WHEN FOUR TEN (10) HOUR SHIFTS ARE ESTABLISHED, OR FORTY (40) HOURS OF STRAIGHT TIME PER WEEK, MONDAY THROUGH FRIDAY, OR OUTSIDE THE NORMAL SHIFT, AND ALL WORK ON SATURDAYS SHALL BE PAID AT TIME AND ONE-HALF THE STRAIGHT TIME RATE. HOURS WORKED OVER TWELVE HOURS (12) IN A SINGLE SHIFT AND ALL WORK PERFORMED AFTER 5:00 PM SATURDAY TO 6:00 AM MONDAY AND HOLIDAYS SHALL BE PAID AT DOUBLE THE STRAIGHT TIME RATE OF PAY. THE EMPLOYER SHALL HAVE THE SOLE DISCRETION TO ASSIGN OVERTIME WORK TO EMPLOYEES. PRIMARY CONSIDERATION FOR OVERTIME WORK SHALL BE GIVEN TO EMPLOYEES REGULARLY ASSIGNED TO THE WORKS TO BE PERFORMED ON OVERTIME SITUATIONS. AFTER AN EMPLOYEE HAS WORKED EIGHT (8) HOURS AT AN APPLICABLE OVERTIME RATE, ALL ADDITIONAL HOURS SHALL BE AT THE APPLICABLE OVERTIME RATE UNTIL SUCH TIME AS THE EMPLOYEE HAS HAD A BREAK OF EIGHT (8) HOURS OR MORE.

U. ALL HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT THREE TIMES THE HOURLY RATE OF WAGE.

V. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS (EXCEPT THANKSGIVING DAY AND CHRISTMAS DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON THANKSGIVING DAY AND CHRISTMAS DAY SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. WHEN A FOUR (4) DAY, TEN (10) HOUR WORK WEEK IS ESTABLISHED, ALL HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE-AND-ONE-HALF TIMES THE HOURLY RATE OF WAGE.

W. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS (EXCEPT MAKE-UP DAYS DUE TO CONDITIONS BEYOND THE CONTROL OF THE EMPLOYER) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

X. THE FIRST FOUR (4) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TWELVE (12) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED OVER TWELVE (12) HOURS ON SATURDAY, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. WHEN HOLIDAY FALLS ON SATURDAY OR SUNDAY, THE DAY BEFORE SATURDAY, FRIDAY, AND THE DAY AFTER SUNDAY, MONDAY, SHALL BE CONSIDERED THE HOLIDAY AND ALL WORK PERFORMED SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

Y. ALL HOURS WORKED OUTSIDE THE HOURS OF 5:00 AM AND 5:00 PM (OR SUCH OTHER HOURS AS MAY BE AGREED UPON BY ANY EMPLOYER AND THE EMPLOYEE) AND ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY (10 HOURS PER DAY FOR A 4 X 10 WORK WEEK) AND ON SATURDAYS AND HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. (EXCEPT FOR EMPLOYEES WHO ARE ABSENT FROM WORK WITHOUT PRIOR APPROVAL ON A SCHEDULED WORKDAY DURING THE WORK WEEK) ALL HOURS WORKED MONDAY THROUGH SATURDAY OVER TWELVE (12) HOURS AND ALL HOURS WORKED ON SUNDAYS AND LABOR DAY SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

Z. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID THE STRAIGHT TIME RATE OF PAY IN ADDITION TO HOLIDAY PAY.

2. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

A. THE FIRST SIX (6) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF SIX (6) HOURS ON SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

B. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.
C. ALL HOURS WORKED ON SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE.

D. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE. THE FIRST EIGHT (8) HOURS WORKED ON HOLIDAYS SHALL BE PAID AT STRAIGHT TIME IN ADDITION TO THE HOLIDAY PAY. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS ON HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE.

E. ALL HOURS WORKED ON SATURDAYS OR HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS OR ON LABOR DAY SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE.

F. THE FIRST EIGHT (8) HOURS WORKED ON HOLIDAYS SHALL BE PAID AT THE STRAIGHT HourLY RATE OF WAGE IN ADDITION TO THE HOLIDAY PAY. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE.

G. ALL HOURS WORKED ON SUNDAY SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED ON PAID HOLIDAYS SHALL BE PAID AT TWO AND ONE-HALF TIMES THE HourLY RATE OF WAGE INCLUDING HOLIDAY PAY.

H. ALL HOURS WORKED ON SUNDAY SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE.

I. ALL HOURS WORKED ON SATURDAYS AND HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND ON LABOR DAY SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE.

J. ALL HOURS WORKED ON SUNDAYS SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED ON PAID HOLIDAYS SHALL BE PAID AT TWO AND ONE-HALF TIMES THE HourLY RATE OF WAGE, INCLUDING THE HOLIDAY PAY. ALL HOURS WORKED ON UNPAID HOLIDAYS SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE.

K. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE IN ADDITION TO THE HOLIDAY PAY.

L. ALL HOURS WORKED ON SATURDAYS (OR ON THE REGULAR DAY OFF DURING A WORKWEEK OTHER THAN MONDAY THROUGH FRIDAY) AND HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE, EXCEPT LABOR DAY WHICH SHALL BE PAID AT DOUBLE THE HourLY RATE. ALL HOURS WORKED MONDAY THROUGH SATURDAY OVER TWELVE (12) HOURS AND ALL HOURS WORKED ON SUNDAYS SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE.

M. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE.

O. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE.

P. THE FIRST EIGHT (8) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS ON SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT TWO TIMES THE HourLY RATE OF WAGE.

Q. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE.

R. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS AND ALL HOURS WORKED OVER SIXTY (60) IN ONE WEEK SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE.

S. ALL HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HourLY RATE OF WAGE.

ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE, EXCEPT THE DAY AFTER THANKSGIVING, THE DAY AFTER CHRISTMAS AND A FLOATING HOLIDAY, WHICH SHALL BE PAID AT THE STRAIGHT TIME RATE IF WORKED, IN ADDITION TO HOLIDAY PAY.

4A. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HourLY RATE OF WAGE.
5. A. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (7).

B. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, THE DAY BEFORE CHRISTMAS, AND CHRISTMAS DAY (8).

C. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).

D. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AND SATURDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).

E. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, PRESIDENTIAL ELECTION DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).


G. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE LAST WORK DAY BEFORE CHRISTMAS DAY, AND CHRISTMAS DAY (7).


I. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS DAY (6).

J. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS EVE DAY, AND CHRISTMAS DAY (7).


L. HOLIDAYS: NEW YEAR'S DAY, MARTIN LUTHER KING JR. DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).

M. HOLIDAYS: NEW YEAR'S DAY, MARTIN LUTHER KING JR. DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, THE DAY BEFORE CHRISTMAS AND CHRISTMAS DAY (9).

N. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, VETERANS' DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (9).

P. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AND SATURDAY AFTER THANKSGIVING DAY, THE DAY BEFORE CHRISTMAS, AND CHRISTMAS DAY (9). If a holiday falls on Sunday, the following Monday shall be considered as a holiday.

Q. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS DAY (6).

R. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, ONE-HALF DAY BEFORE CHRISTMAS DAY, AND CHRISTMAS DAY. (7 1/2).

S. PAID HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS DAY (7).

T. PAID HOLIDAYS: NEW YEAR'S DAY, WASHINGTON'S BIRTHDAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, AND THE DAY BEFORE OR AFTER CHRISTMAS (9).

U. PAID HOLIDAYS: NEW YEAR'S DAY, MARTIN LUTHER KING JR. DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS DAY (8).

V. PAID HOLIDAYS: SIX (6) PAID HOLIDAYS.

W. PAID HOLIDAYS: NINE (9) PAID HOLIDAYS.
X. HOLIDAYS: AFTER 520 HOURS - NEW YEAR'S DAY, THANKSGIVING DAY AND CHRISTMAS DAY. AFTER 2080 HOURS - NEW YEAR'S DAY, WASHINGTON'S BIRTHDAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, CHRISTMAS DAY AND A FLOATING HOLIDAY (8).

Y. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, PRESIDENTIAL ELECTION DAY, THANKSGIVING DAY, THE FRIDAY FOLLOWING THANKSGIVING DAY, AND CHRISTMAS DAY (8).

Z. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, VETERANS DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).


B. PAID HOLIDAYS: NEW YEAR'S EVE DAY, NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS EVE'S DAY, AND CHRISTMAS DAY (9).

C. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE LAST WORK DAY BEFORE CHRISTMAS DAY, AND CHRISTMAS DAY (9).


E. PAID HOLIDAYS: NEW YEAR'S DAY, DAY BEFORE OR AFTER NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, AND A HALF-DAY ON CHRISTMAS EVE DAY. (9 1/2).


H. PAID HOLIDAYS: NEW YEAR'S DAY, NEW YEAR'S EVE DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, THE DAY AFTER CHRISTMAS, AND A FLOATING HOLIDAY (10).

I. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (7).

J. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, THE DAY AFTER CHRISTMAS, AND A FLOATING HOLIDAY (9).

L. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, THE LAST WORKING DAY BEFORE CHRISTMAS DAY, AND CHRISTMAS DAY. (8)

Q. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, VETERANS DAY, THANKSGIVING DAY, THE DAY AFTER THANKSGIVING DAY AND CHRISTMAS DAY (8). UNPAID HOLIDAY, PRESIDENTS' DAY.


V. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS EVE' DAY, CHRISTMAS DAY, AND ONE DAY OF THE EMPLOYEE'S CHOICE (9).

W. PAID HOLIDAYS: NEW YEAR'S DAY, DAY BEFORE NEW YEARS DAY, PRESIDENTS DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, DAY BEFORE OR AFTER CHRISTMAS DAY (10).

X. PAID HOLIDAYS: NEW YEARS DAY, DAY BEFORE OR AFTER NEW YEAR'S DAY, PRESIDENTS DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, DAY BEFORE OR AFTER CHRISTMAS DAY, EMPLOYEE'S BIRTHDAY (11).
Y. PAID HOLIDAYS: NEW YEAR’S DAY, PRESIDENTS’ DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, AND A FLOATING HOLIDAY (9).


NOTE CODES

8. A. IN ADDITION TO THE HOURLY WAGE AND FRINGE BENEFITS, THE FOLLOWING DEPTH PREMIUMS APPLY TO DEPTHS OF FIFTY FEET OR MORE:
   - OVER 50’ TO 100’ - $2.00 PER FOOT FOR EACH FOOT OVER 50 FEET
   - OVER 100’ TO 150’ - $3.00 PER FOOT FOR EACH FOOT OVER 100 FEET
   - OVER 150’ TO 220’ - $4.00 PER FOOT FOR EACH FOOT OVER 150 FEET
   - OVER 220’ - $5.00 PER FOOT FOR EACH FOOT OVER 220 FEET

C. IN ADDITION TO THE HOURLY WAGE AND FRINGE BENEFITS, THE FOLLOWING DEPTH PREMIUMS APPLY TO DEPTHS OF FIFTY FEET OR MORE:
   - OVER 50’ TO 100’ - $1.00 PER FOOT FOR EACH FOOT OVER 50 FEET
   - OVER 100’ TO 150’ - $1.50 PER FOOT FOR EACH FOOT OVER 100 FEET
   - OVER 150’ TO 200’ - $2.00 PER FOOT FOR EACH FOOT OVER 150 FEET
   - OVER 200’ - DIVERS MAY NAME THEIR OWN PRICE

D. WORKERS WORKING WITH SUPPLIED AIR ON HAZMAT PROJECTS RECEIVE AN ADDITIONAL $1.00 PER HOUR.

L. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS - LEVEL A: $0.75, LEVEL B: $0.50, AND LEVEL C: $0.25.

M. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS: LEVELS A & B: $1.00, LEVELS C & D: $0.50.

N. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS - LEVEL A: $1.00, LEVEL B: $0.75, LEVEL C: $0.50, AND LEVEL D: $0.25

P. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS - CLASS A SUIT: $2.00, CLASS B SUIT: $1.50, CLASS C SUIT: $1.00, AND CLASS D SUIT $0.50.
Washington State Department of Labor and Industries
Policy Statement
(Regarding the Production of "Standard" or "Non-standard" Items)

Below is the department's (State L&I's) list of criteria to be used in determining whether a prefabricated item is "standard" or "non-standard". For items not appearing on WSDOT's predetermined list, these criteria shall be used by the Contractor (and the Contractor's subcontractors, agents to subcontractors, suppliers, manufacturers, and fabricators) to determine coverage under RCW 39.12. The production, in the State of Washington, of non-standard items is covered by RCW 39.12, and the production of standard items is not. The production of any item outside the State of Washington is not covered by RCW 39.12.

1. Is the item fabricated for a public works project? If not, it is not subject to RCW 39.12. If it is, go to question 2.

2. Is the item fabricated on the public works jobsite? If it is, the work is covered under RCW 39.12. If not, go to question 3.

3. Is the item fabricated in an assembly/fabrication plant set up for, and dedicated primarily to, the public works project? If it is, the work is covered by RCW 39.12. If not, go to question 4.

4. Does the item require any assembly, cutting, modification or other fabrication by the supplier? If not, the work is not covered by RCW 39.12. If yes, go to question 5.

5. Is the prefabricated item intended for the public works project typically an inventory item which could reasonably be sold on the general market? If not, the work is covered by RCW 39.12. If yes, go to question 6.

6. Does the specific prefabricated item, generally defined as standard, have any unusual characteristics such as shape, type of material, strength requirements, finish, etc? If yes, the work is covered under RCW 39.12.

Any firm with questions regarding the policy, WSDOT's Predetermined List, or for determinations of covered and non-covered workers shall be directed to State L&I at (360) 902-5330.
Below is a list of potentially prefabricated items, originally furnished by WSDOT to Washington State Department of Labor and Industries, that may be considered non-standard and therefore covered by the prevailing wage law, RCW 39.12. Items marked with an X in the "YES" column should be considered to be non-standard and therefore covered by RCW 39.12. Items marked with an X in the "NO" column should be considered to be standard and therefore not covered. Of course, exceptions to this general list may occur, and in that case shall be evaluated according to the criteria described in State and L&I's policy statement.

<table>
<thead>
<tr>
<th>ITEM DESCRIPTION</th>
<th>YES</th>
<th>NO</th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Metal rectangular frames, solid metal covers, herringbone grates, and</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>bi-directional vaned grates for Catch Basin Types 1, 1L, 1P, and 2 and Concrete</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Inlets. See Std. Plans</td>
<td></td>
<td></td>
</tr>
<tr>
<td>2. Metal circular frames (rings) and covers, circular grates, and prefabricated</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>ladders for Manhole Types 1, 2, and 3, Drywell Types 1, 2, and 3 and Catch Basin</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Type 2. See Std. Plans</td>
<td></td>
<td></td>
</tr>
<tr>
<td>3. Prefabricated steel grate supports and welded grates, metal frames and dual</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>vaned grates, and Type 1, 2, and 3 structural tubing grates for Drop Inlets.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>See Std. Plans</td>
<td></td>
<td></td>
</tr>
<tr>
<td>4. Concrete Pipe - Plain Concrete pipe and reinforced concrete pipe Class 2 to</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>5 sizes smaller than 60 inch diameter.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>5. Concrete Pipe - Plain Concrete pipe and reinforced concrete pipe Class 2 to</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>5 sizes larger than 60 inch diameter.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>6. Corrugated Steel Pipe - Steel lock seam corrugated pipe for culverts and</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>storm sewers, sizes 30 inch to 120 inches in diameter. May also be treated, 1</td>
<td></td>
<td></td>
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<tr>
<td>thru 5.</td>
<td></td>
<td></td>
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<tr>
<td>7. Corrugated Aluminum Pipe - Aluminum lock seam corrugated pipe for culverts</td>
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<td>X</td>
</tr>
<tr>
<td>and storm sewers, sizes 30 inch to 120 inches in diameter. May also be treated,</td>
<td></td>
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<tr>
<td>#5.</td>
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</tbody>
</table>
## WSDOT's Predetermined List for Suppliers - Manufactures - Fabricator

<table>
<thead>
<tr>
<th>ITEM DESCRIPTION</th>
<th>YES</th>
<th>NO</th>
</tr>
</thead>
<tbody>
<tr>
<td>8. Anchor Bolts &amp; Nuts - Anchor Bolts and Nuts, for mounting sign structures,</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>luminaries and other items, shall be made from commercial bolt stock.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>See Contract Plans and Std. Plans for size and material type.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>9. Aluminum Pedestrian Handrail - Pedestrian handrail conforming to the type</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>and material specifications set forth in the contract plans. Welding of</td>
<td></td>
<td></td>
</tr>
<tr>
<td>aluminum shall be in accordance with Section 9-28.14(3).</td>
<td></td>
<td></td>
</tr>
<tr>
<td>10. Major Structural Steel Fabrication - Fabrication of major steel items such as</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>trusses, beams, girders, etc., for bridges.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>11. Minor Structural Steel Fabrication - Fabrication of minor steel items such as</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>special hangers, brackets, access doors for structures, access ladders</td>
<td></td>
<td></td>
</tr>
<tr>
<td>for irrigation boxes, bridge expansion joint systems, etc., involving welding,</td>
<td></td>
<td></td>
</tr>
<tr>
<td>cutting, punching and/or boring of holes. See Contact Plans for item description</td>
<td></td>
<td></td>
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<tr>
<td>and shop drawings.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>12. Aluminum Bridge Railing Type BP - Metal bridge railing conforming to the</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>type and material specifications set forth in the Contract Plans. Welding of</td>
<td></td>
<td></td>
</tr>
<tr>
<td>aluminum shall be in accordance with Section 9-28.14(3).</td>
<td></td>
<td></td>
</tr>
<tr>
<td>13. Concrete Piling - Precast-Prestressed concrete piling for use as 55 and</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>70 ton concrete piling. Concrete to conform to Section 9-19.1 of Std. Spec.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>14. Precast Manhole Types 1, 2, and 3 with cones, adjustment sections and flat</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>top slabs. See Std. Plans.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>15. Precast Drywell Types 1, 2, and with cones and adjustment Sections.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>See Std. Plans.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>16. Precast Catch Basin - Catch Basin type 1, 1L, 1P, and 2 with adjustment</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>sections. See Std. Plans.</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Supplemental to Wage Rates
<table>
<thead>
<tr>
<th>ITEM DESCRIPTION</th>
<th>YES</th>
<th>NO</th>
</tr>
</thead>
<tbody>
<tr>
<td>17. Precast Concrete Inlet - with adjustment sections, See Std. Plans</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>18. Precast Drop Inlet Type 1 and 2 with metal grate supports. See Std. Plans.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>19. Precast Grate Inlet Type 2 with extension and top units. See Std. Plans</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>20. Metal frames, vanes, grates, and hoods for Combination Inlets. See Std. Plans</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>21. Precast Concrete Utility Vaults - Precast Concrete utility vaults of various sizes. Used for in ground storage of utility facilities and controls. See Contract Plans for size and construction requirements. Shop drawings are to be provided for approval prior to casting</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>22. Vault Risers - For use with Valve Vaults and Utilities Vaults.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>23. Valve Vault - For use with underground utilities. See Contract Plans for details.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>24. Precast Concrete Barrier - Precast Concrete Barrier for use as new barrier or may also be used as Temporary Concrete Barrier. Only new state approved barrier may be used as permanent barrier.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>25. Reinforced Earth Wall Panels – Reinforced Earth Wall Panels in size and shape as shown in the Plans. Fabrication plant has annual approval for methods and materials to be used. See Shop Drawing. Fabrication at other locations may be approved, after facilities inspection, contact HQ. Lab.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>26. Precast Concrete Walls - Precast Concrete Walls - tilt-up wall panel in size and shape as shown in Plans. Fabrication plant has annual approval for methods and materials to be used</td>
<td></td>
<td>X</td>
</tr>
</tbody>
</table>

Supplemental to Wage Rates
<table>
<thead>
<tr>
<th>ITEM DESCRIPTION</th>
<th>YES</th>
<th>NO</th>
</tr>
</thead>
<tbody>
<tr>
<td>27. Precast Railroad Crossings - Concrete Crossing Structure Slabs.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>28. 12, 18 and 26 inch Standard Precast Prestressed Girder – Standard Precast</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>Prestressed Girder for use in structures. Fabricator plant has annual approval</td>
<td></td>
<td></td>
</tr>
<tr>
<td>of methods and materials to be used. Shop Drawing to be provided for approval</td>
<td></td>
<td></td>
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<tr>
<td>prior to casting girders. See Std. Spec. Section 6-02.3(25)A</td>
<td></td>
<td></td>
</tr>
<tr>
<td>29. Prestressed Concrete Girder Series 4-14 - Prestressed Concrete Girder</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>for use in structures. Fabricator plant has annual approval of methods and</td>
<td></td>
<td></td>
</tr>
<tr>
<td>materials to be used. Shop Drawing to be provided for approval prior to casting</td>
<td></td>
<td></td>
</tr>
<tr>
<td>girders. See Std. Spec. Section 6-02.3(25)A</td>
<td></td>
<td></td>
</tr>
<tr>
<td>30. Prestressed Tri-Beam Girder - Prestressed Tri-Beam Girder for use in</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>structures. Fabricator plant has annual approval of methods and materials to</td>
<td></td>
<td></td>
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<tr>
<td>be used. Shop Drawing to be provided for approval prior to casting girders.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>See Std. Spec. Section 6-02.3(25)A</td>
<td></td>
<td></td>
</tr>
<tr>
<td>31. Prestressed Precast Hollow-Core Slab – Precast Prestressed Hollow-core</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>slab for use in structures. Fabricator plant has annual approval of methods</td>
<td></td>
<td></td>
</tr>
<tr>
<td>and materials to be used. Shop Drawing to be provided for approval prior to</td>
<td></td>
<td></td>
</tr>
<tr>
<td>casting girders. See Std. Spec. Section 6-02.3(25)A.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>32. Prestressed-Bulb Tee Girder - Bulb Tee Prestressed Girder for use in</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>structures. Fabricator plant has annual approval of methods and materials to</td>
<td></td>
<td></td>
</tr>
<tr>
<td>be used. Shop Drawing to be provided for approval prior to casting girders.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>See Std. Spec. Section 6-02.3(25)A</td>
<td></td>
<td></td>
</tr>
<tr>
<td>33. Monument Case and Cover</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>See Std. Plan.</td>
<td></td>
<td></td>
</tr>
<tr>
<td>ITEM DESCRIPTION</td>
<td>YES</td>
<td>NO</td>
</tr>
<tr>
<td>---------------------------------------------------------------------------------</td>
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</tr>
<tr>
<td>34. Cantilever Sign Structure - Cantilever Sign Structure fabricated from steel tubing meeting AASHTO-M-183. See Std. Plans, and Contract Plans for details. The steel structure shall be galvanized after fabrication in accordance with AASHTO-M-111.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>35. Mono-tube Sign Structures - Mono-tube Sign Bridge fabricated to details shown in the Plans. Shop drawings for approval are required prior to fabrication.</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>36. Steel Sign Bridges - Steel Sign Bridges fabricated from steel tubing meeting AASHTO-M-138 for Aluminum Alloys. See Std. Plans, and Contract Plans for details. The steel structure shall be galvanized after fabrication in accordance with AASHTO-M-111.</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>37. Steel Sign Post - Fabricated Steel Sign Posts as detailed in Std Plans. Shop drawings for approval are to be provided prior to fabrication</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>38. Light Standard-Prestressed - Spun, prestressed, hollow concrete poles.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>39. Light Standards - Lighting Standards for use on highway illumination systems, poles to be fabricated to conform with methods and materials as specified on Std. Plans. See Special Provisions for pre-approved drawings.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>40. Traffic Signal Standards - Traffic Signal Standards for use on highway and/or street signal systems. Standards to be fabricated to conform with methods and material as specified on Std. Plans. See Special Provisions for pre-approved drawings</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>41. Precast Concrete Sloped Mountable Curb (Single and Dual-faced) See Std. Plans.</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>ITEM DESCRIPTION</td>
<td>YES</td>
<td>NO</td>
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<tr>
<td>42. Traffic Signs - Prior to approval of a Fabricator of Traffic Signs, the sources of the following materials must be submitted and approved for reflective sheeting, legend material, and aluminum sheeting. <strong>NOTE:</strong> Fabrication inspection required. Only signs tagged &quot;Fabrication Approved&quot; by WSDOT Sign Fabrication inspector to be installed</td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>43. Cutting &amp; bending reinforcing steel</td>
<td></td>
<td>X</td>
</tr>
<tr>
<td>44. Guardrail components</td>
<td>X</td>
<td>X</td>
</tr>
<tr>
<td>45. Aggregates/Concrete mixes</td>
<td></td>
<td></td>
</tr>
<tr>
<td>46. Asphalt</td>
<td></td>
<td>Covered by WAC 296-127-018</td>
</tr>
<tr>
<td>47. Fiber fabrics</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>48. Electrical wiring/components</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>49. treated or untreated timber pile</td>
<td>X</td>
<td></td>
</tr>
<tr>
<td>50. Girder pads (elastomeric bearing)</td>
<td>X</td>
<td></td>
</tr>
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<td>51. Standard Dimension lumber</td>
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<td>52. Irrigation components</td>
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Supplemental to Wage Rates
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<td>Traffic Buttons</td>
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<td>Epoxy</td>
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<td>Cribbing</td>
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<td>Water distribution materials</td>
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<td>Steel &quot;H&quot; piles</td>
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<td>Steel pipe for concrete pile casings</td>
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<td>Steel pile tips, standard</td>
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<tr>
<td>Steel pile tips, custom</td>
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</tr>
</tbody>
</table>
State of Washington  
Department of Labor and Industries  
Prevailing Wage Section - Telephone (360) 902-  
PO Box 44540, Olympia, WA 98504-4540  
Washington State Prevailing Wage  
The PREVAILING WAGES listed here include both the hourly wage rate and the hourly rate of fringe  
benefits. On public works projects, workers' wage and benefit rates must add to not less than this total. A  
brief description of overtime calculation requirements is provided on the Benefit Code Key.  

METAL FABRICATION (IN SHOP)  
EFFECTIVE 03/04/2009  
**************************************************************************************************  
(See Benefit Code Key)  
Classification Code | Prevailing Wage | Overtime Code | Holiday Code  
---------------------|-----------------|---------------|--------------  
FITTER/WELDER | $12.76 | 1 |  
LABORER | $8.55 | 1 |  
MACHINE OPERATOR | $12.66 | 1 |  
PAINTER | $10.20 | 1 |  

Counties Covered:  
ADAMS, ASOTIN, COLUMBIA, DOUGLAS, FERRY, FRANKLIN, GARFIELD, KITTITAS  
LINCOLN, OKANOGAN, PEND OREILLE, STEVENS, WALLA WALLA AND WHITMAN  

MACHINE OPERATOR | $10.53 | 1 |  
PAINTER | $9.76 | 1 |  
WELDER | $16.70 | 1 |  

Counties Covered:  
BENTON  

FITTER | $15.04 | 1 |  
LABORER | $9.54 | 1 |  
MACHINE OPERATOR | $9.71 | 1 |  
PAINTER | $9.93 | 1 |  
WELDER | $12.24 | 1 |  

Counties Covered:  
CHELAN  

FITTER/WELDER | $15.16 | 1 |  
LABORER | $11.13 | 1 |  
MACHINE OPERATOR | $10.86 | 1 |  
PAINTER | $11.41 | 1 |  

Counties Covered:  
CLALLAM, GRAYS HARBOR, ISLAND, JEFFERSON, LEWIS, MASON, PACIFIC  
SAN JUAN AND SKAGIT  

Supplemental to Wage Rates
**METAL FABRICATION (IN SHOP)**
**EFFECTIVE 03/04/2009**

(See Benefit Code Key)

<table>
<thead>
<tr>
<th>Classification Code</th>
<th>Prevailing Wage</th>
<th>Overtime Code</th>
<th>Holiday Code</th>
</tr>
</thead>
<tbody>
<tr>
<td>FITTER</td>
<td>$27.49</td>
<td>1E</td>
<td>6H</td>
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<td>LABORER</td>
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<td>1E</td>
<td>6H</td>
</tr>
<tr>
<td>MACHINE OPERATOR</td>
<td>$28.77</td>
<td>1E</td>
<td>6H</td>
</tr>
<tr>
<td>PAINTER</td>
<td>$25.31</td>
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<tr>
<td>LAYEROUT</td>
<td>$28.77</td>
<td>1E</td>
<td>6H</td>
</tr>
</tbody>
</table>

**Counties Covered:**
- CLARK

| MACHINE OPERATOR     | $24.65          | 1B            | 6V           |
| FITTER               | $24.65          | 1B            | 6V           |
| WELDER               | $24.65          | 1B            | 6V           |

**Counties Covered:**
- COWLITZ

| FITTER/WELDER        | $10.79          | 1             |              |
| PAINTER              | $8.55           | 1             |              |

**Counties Covered:**
- GRANT

| FITTER               | $15.86          | 1             |              |
| LABORER              | $9.78           | 1             |              |
| MACHINE OPERATOR     | $13.04          | 1             |              |
| PAINTER              | $11.10          | 1             |              |
| WELDER               | 15.48           |               |              |

**Counties Covered:**
- KING

| FITTER               | $26.96          | 1             |              |
| LABORER              | $8.55           | 1             |              |
| MACHINE OPERATOR     | $13.83          | 1             |              |
| WELDER               | $13.83          | 1             |              |

**Counties Covered:**
- KITSAP
### METAL FABRICATION (IN SHOP)
**EFFECTIVE 03/04/2009**

(See Benefit Code Key)

<table>
<thead>
<tr>
<th>Classification</th>
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<th>Overtime Code</th>
<th>Holiday Code</th>
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<tbody>
<tr>
<td>FITTER/WELDER</td>
<td>$16.99</td>
<td>1</td>
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<td></td>
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<td>$10.44</td>
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<td>MACHINE OPERATOR</td>
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<td></td>
</tr>
<tr>
<td>PAINTER</td>
<td>$17.03</td>
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**Counties Covered:**

KLICKITAT, SKAMANIA, WAHKIAKUM

<table>
<thead>
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<th>Classification</th>
<th>Code</th>
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<th>Overtime Code</th>
<th>Holiday Code</th>
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</thead>
<tbody>
<tr>
<td>FITTER</td>
<td>$15.25</td>
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<tr>
<td>LABORER</td>
<td>$10.32</td>
<td>1</td>
<td></td>
<td></td>
</tr>
<tr>
<td>MACHINE OPERATOR</td>
<td>$13.98</td>
<td>1</td>
<td></td>
<td></td>
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<tr>
<td>WELDER</td>
<td>$13.98</td>
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**Counties Covered:**

PIERCE

<table>
<thead>
<tr>
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<th>Holiday Code</th>
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<tr>
<td>FITTER/WELDER</td>
<td>$15.38</td>
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<td>MACHINE OPERATOR</td>
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<td>PAINTER</td>
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**Counties Covered:**

SNOHOMISH

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<tbody>
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<td>FITTER/WELDER</td>
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<td>MACHINE OPERATOR</td>
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<td>PAINTER</td>
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**Counties Covered:**

SPOKANE

---

*Supplemental to Wage Rates*
<table>
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<th>Classification Code</th>
<th>Prevailing Wage</th>
<th>Overtime Code</th>
<th>Holiday Code</th>
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</thead>
<tbody>
<tr>
<td>FITTER</td>
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<tr>
<td>LABORER</td>
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<td>LAYERCUT</td>
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<td>WELDER</td>
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Counties Covered:
THURSTON

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<td>FITTER/WELDER</td>
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Counties Covered:
WHATCOM

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<tr>
<td>FITTER</td>
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<td>LABORER</td>
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<td>PAINTER</td>
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<td>$11.32</td>
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Counties Covered:
YAKIMA

Supplemental to Wage Rates
FABRICATED PRECAST CONCRETE PRODUCTS
EFFECTIVE 03/04/2009

(See Benefit Code Key)

<table>
<thead>
<tr>
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<tr>
<td>ADAMS, ASOTIN, BENTON, COLUMBIA, DOUGLAS, FERRY, GARFIELD, GRANT, LINCOLN, OKANOGAN, PEND OREILLE, STEVENS, WALLA WALLA AND WHITMAN</td>
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<td>CLALLAM, CLARK, COWLITZ, GRAYS HARBOR, ISLAND, JEFFERSON, KITSAP, LEWIS, MASON, PACIFIC, SAN JUAN, SKAGIT, SNOHOMISH, THURSTON AND WAHGIKUM</td>
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<tr>
<td>LABORER</td>
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</tr>
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</table>

Supplemental to Wage Rates

13
WSDOT's List of State Occupations not applicable to Heavy and Highway Construction Projects

This project is subject to the state hourly minimum rates for wages and fringe benefits in the contract provisions, as provided by the state Department of Labor and Industries. The following list of occupations, is comprised of those occupations that are not normally used in the construction of heavy and highway projects. When considering job classifications for use and / or payment when bidding on, or building heavy and highway construction projects for, or administered by WSDOT, these Occupations will be excepted from the included "Washington State Prevailing Wage Rates For Public Work Contracts" documents.

- Electrical Fixture Maintenance Workers
- Electricians - Motor Shop
- Heating Equipment Mechanics
- Industrial Engine and Machine Mechanics
- Industrial Power Vacuum Cleaners
- Inspection, Cleaning, Sealing of Water Systems by Remote Control
- Laborers - Underground Sewer & Water
- Machinists (Hydroelectric Site Work)
- Modular Buildings
- Playground & Park Equipment Installers
- Power Equipment Operators - Underground Sewer & Water
- Residential *** ALL ASSOCIATED RATES ***
- Sign Makers and Installers (Non-Electrical)
- Sign Makers and Installers (Electrical)
- Stage Rigging Mechanics (Non Structural)

The following occupations may be used only as outlined in the preceding text concerning "WSDOT's list for Suppliers - Manufacturers - Fabricators"

- Fabricated Precast Concrete Products
- Metal Fabrication (In Shop)

Definitions for the Scope of Work for prevailing wages may be found at the Washington State Department of Labor and Industries web site and in WAC Chapter 296-127.
Washington State Department of Labor and Industries
Policy Statements
(Regarding Production and Delivery of Gravel, Concrete, Asphalt, etc.)

WAC 296-127-018 Agency filings affecting this section

Coverage and exemptions of workers involved in the production and delivery of gravel, concrete, asphalt, or similar materials.

(1) The materials covered under this section include but are not limited to: Sand, gravel, crushed rock, concrete, asphalt, or other similar materials.

(2) All workers, regardless of by whom employed, are subject to the provisions of chapter 39.12 RCW when they perform any or all of the following functions:

(a) They deliver or discharge any of the above-listed materials to a public works project site:

(i) At one or more point(s) directly upon the location where the material will be incorporated into the project; or

(ii) At multiple points at the project; or

(iii) Adjacent to the location and coordinated with the incorporation of those materials.

(b) They wait at or near a public works project site to perform any tasks subject to this section of the rule.

(c) They remove any materials from a public works construction site pursuant to contract requirements or specifications (e.g., excavated materials, materials from demolished structures, clean-up materials, etc.).

(d) They work in a materials production facility (e.g., batch plant, borrow pit, rock quarry, etc.) which is established for a public works project for the specific, but not necessarily exclusive, purpose of supplying materials for the project.

(e) They deliver concrete to a public works site regardless of the method of incorporation.

(f) They assist or participate in the incorporation of any materials into the public works project.
(3) All travel time that relates to the work covered under subsection (2) of this section requires the payment of prevailing wages. Travel time includes time spent waiting to load, loading, transporting, waiting to unload, and delivering materials. Travel time would include all time spent in travel in support of a public works project whether the vehicle is empty or full. For example, travel time spent returning to a supply source to obtain another load of material for use on a public works site or returning to the public works site to obtain another load of excavated material is time spent in travel that is subject to prevailing wage. Travel to a supply source, including travel from a public works site, to obtain materials for use on a private project would not be travel subject to the prevailing wage.

(4) Workers are not subject to the provisions of chapter 39.12 RCW when they deliver materials to a stockpile.

(a) A "stockpile" is defined as materials delivered to a pile located away from the site of incorporation such that the stockpiled materials must be physically moved from the stockpile and transported to another location on the project site in order to be incorporated into the project.

(b) A stockpile does not include any of the functions described in subsection (2)(a) through (f) of this section; nor does a stockpile include materials delivered or distributed to multiple locations upon the project site; nor does a stockpile include materials dumped at the place of incorporation, or adjacent to the location and coordinated with the incorporation.

(5) The applicable prevailing wage rate shall be determined by the locality in which the work is performed. Workers subject to subsection (2)(d) of this section, who produce such materials at an off-site facility shall be paid the applicable prevailing wage rates for the county in which the off-site facility is located. Workers subject to subsection (2) of this section, who deliver such materials to a public works project site shall be paid the applicable prevailing wage rates for the county in which the public works project is located.

[Statutory Authority: Chapter 39.12 RCW, RCW 43.22.051 and 43.22.270. 08-24-101, § 296-127-018, filed 12/2/08, effective 1/2/09. Statutory Authority: Chapters 39.04 and 39.12 RCW and RCW 43.22.270. 92-01-104 and 92-08-101, § 296-127-018, filed 12/19/91 and 4/1/92, effective 8/31/92.]
STANDARD PLANS
CONCRETE AND DUCTILE IRON PIPE

NOTES
1. See Standard Specifications Section 7-08.3(3) for Pipe Zone Backfill.
2. See Standard Specifications Section 9-03.12(3) for Gravel Backfill for Pipe Zone Bedding.
4. For sanitary sewer installation, concrete pipe shall be bedded to spring line.

THERMOPLASTIC PIPE

CLEARANCE BETWEEN PIPES FOR MULTIPLE INSTALLATIONS

<table>
<thead>
<tr>
<th>PIPE</th>
<th>SIZE</th>
<th>MINIMUM DISTANCE BETWEEN BARRELS</th>
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<tr>
<td>CIRCULAR PIPE</td>
<td>12&quot; to 24&quot;</td>
<td>12&quot;</td>
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<tr>
<td></td>
<td>30&quot; to 60&quot;</td>
<td>Diam. /2</td>
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<tr>
<td></td>
<td>102&quot; to 180&quot;</td>
<td>48&quot;</td>
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<tr>
<td>PIPE ARCH</td>
<td>18&quot; to 36&quot;</td>
<td>12&quot;</td>
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<td>43&quot; to 142&quot;</td>
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<tr>
<td>METAL ONLY</td>
<td>148&quot; to 200&quot;</td>
<td>48&quot;</td>
</tr>
</tbody>
</table>

PIPE ZONE BEDDING AND BACKFILL

STANDARD PLAN B-55.20-00

EXPRESS JULY 1, 2007

APPROVED FOR PUBLICATION

Herold J. Peterfi 08-01-08
WASHINGTON STATE DEPARTMENT OF TRANSPORTATION
END SECTION LENGTH SHALL BE AT LEAST SIX TIMES THE DIAMETER OF THE PIPE (SEE STD. SPEC. 7-02.3(1))

NOTES

1. The culvert ends shall be beveled to match the embankment or ditch slope and shall not be beveled flatter than 4H:1V. When slopes are between 4H:1V and 6H:1V, shape the slope in the vicinity of the culvert end to ensure that no part of the culvert protrudes more than 4" above the ground line.

2. Field cutting of culvert ends is permitted when approved by the Engineer. All field-cut culvert pipe shall be treated with treatment as shown in the Standard Specifications or General Special Provisions.

FOR CULVERTS 30" DIAMETER OR LESS

BEVELED END SECTIONS

STANDARD PLAN B-70.20-00

APPROVED FOR PUBLICATION
Harold J. Paterkaso 06-01-08

Washington State Department of Transportation
NOTES

1. Maximize detention of stormwater by placing fence as far away from toe of slope as possible without encroaching on sensitive areas or outside of the clearing boundaries.

2. Install silt fencing along contours whenever possible.

3. Install the ends of the silt fence to point slightly up-slope to prevent sediment from flowing around the ends of the fence.

4. Perform maintenance in accordance with Standard Specifications 8.01.3(6)A and 8.01.3(10).

SILT FENCE

STANDARD PLAN I-30.10-00

Estado de Washington

Arquitecto del paisaje

MARY W. MAURER
CERTIFICADO N. 000568

APROBADO PARA LA PUBLICACION
Pasco Bakotich III  09-20-07

Washington State Department of Transportation
SILT FENCE DESIGN

PLACE SAND BAGS AS REQUIRED AROUND CULVERT TO PROVIDE SUPPORT FOR SILT FENCE

POST - SEE STD. SPEC. 8-01.23(4)

EMBED POSTS INTO SAND BAGS AS REQUIRED

GEOTEXTILE FOR TEMPORARY SILT FENCE
- SEE STD. SPEC. 8-33.2(1), TABLE 8

FLOW

EDGE OF GEOTEXTILE

SECTION A

CULVERT, BOX CULVERT, OR PIPE ARCH
- END TREATMENT VARIES

COMPOST BERM DESIGN

CON/POST BERM - SEE STD. PLAN I-10.10

DISTURBED AREA

PROTECTED AREA

STATE OF WASHINGTON
RENEWED LANDSCAPE ARCHITECT

MARK W. MAUSER
CERTIFICATE NO. 000098

EROSION CONTROL
AT CULVERT ENDS

STANDARD PLAN I-30.20-00

APPROVED FOR PUBLICATION
Pasco Bakdich III 09-20-07
STATEWIDE ENGINEER
Washington State Department of Transportation
1. More than the minimum of one fastener per square yard may be required due to conditions such as blanket composition, soil type, surface uniformity, and slope steepness.

NOTES

1. This plan depicts the Steel Light Standard types and terms commonly referred to in the contract. All Steel Light Standards are fabricated in accordance with the Standard Specifications and the Contract Provisions.

2. The Luminaire Pole height shall not exceed 50’ (H1).

3. Slip Bases shall not be installed on 50’ (H1) poles with Double Mast Arms, nor on poles weighing more than 1000 lbs.

4. The optional location of the Luminaire head is over the edge of the traveled way. Based on the placement of the Steel Light Standard foundation, the position of the Luminaire head may vary. See Standard Plan J-28.22.

5. Light Standard mast arm orientation is typically perpendicular to roadway centerline.


STEEL LIGHT STANDARD

STANDARD PLAN J-28.10-00

STEEL LIGHT STANDARD FOUNDATION
- SEE STANDARD PLAN J-28.30

LIGHT STANDARD WITH TYPE 1 (Davit) Mast Arm
(Slip Base Shown)

LIGHT STANDARD WITH TYPE 2 (Elson) Mast Arm
(Slip Base Shown)
NOTES


2. The Strap Templates shall be held in place by nuts, 6" from the top of the foundation and 3" from the bottom of the anchor bolts. 18 heavy duty hex nuts and 6 round washers are required for a Slip Base assembly. 18 heavy duty hex nuts and 6 plate washers are required for a Fixed Base assembly.

3. Use Steel Light Standard Foundation Type A on level ground or slopes not exceeding 4½:1. Use Type B for slopes steeper than 4½:1, but not exceeding 2:1. Slope steeper than 2:1 shall require a special design.

4. These foundations are designed for a minimum of 2,000 PSF (TYPE A) or 1,500 PSF (TYPE B) allowable lateral bearing pressure for the soil. A special foundation shall be required for soil with lower allowable lateral bearing pressure than 1,500 PSF.

5. The Luminaire Pole height shall not exceed 60' (H1).

6. Slip Bases shall not be installed on 50' (H1) poles with Double Mast Arms, nor on poles weighing more than 1000 lbs.

7. Slip Bases are not required on poles placed outside of the Design Clear Zone, nor on poles installed behind traffic barrier.

8. Foundations constructed within Ecology Embankments shall be increased in depth by the depth of the Ecology Embankment.

9. Exposed portions of the foundation shall be formed to create a smooth finished surface. All forming shall be removed upon completion of foundation construction.

10. For excavation, concrete placement, and backfill options, see METHOD 1 and METHOD 2 on Sheet 2 of 2.

11. The Anchor Bolts shall be high strength steel, manufactured from ASTM A449, with heavy hex nuts and hardened washers. Galvanize the Anchor Bolts according to AASHO M232.

12. The foundation shall meet the requirements of Standard Specification Sect. 8-20.3(9).

13. See Standard Plan C-60 and C-14h for steel light standards on traffic barrier.

---

**STEEL LIGHT STANDARD FOUNDATION TYPES A & B**

**STANDARD PLAN J-28.30-00**

1 of 2 SHEETS

**APPROVED FOR PUBLICATION**

08-07-01

Pasco Bakdich III 08-07-01

Washington State Department of Transportation

---

**ANCHOR BOLT TABLE**

<table>
<thead>
<tr>
<th>LUMINARIE HEIGHT (FT)</th>
<th>MAST ARM TYPE</th>
<th>MAST ARM LENGTH</th>
<th>ANCHOR BOLT DIAMETER</th>
</tr>
</thead>
<tbody>
<tr>
<td>3' TO 9'</td>
<td>SINGLE</td>
<td>4'</td>
<td>1/2&quot;</td>
</tr>
<tr>
<td>13' TO 30'</td>
<td>DOUBLE</td>
<td>8'</td>
<td>1/2&quot;</td>
</tr>
<tr>
<td>20' TO 42'</td>
<td>DOUBLE</td>
<td>12'</td>
<td>1/2&quot;</td>
</tr>
<tr>
<td>40' TO 60'</td>
<td>DOUBLE</td>
<td>16'</td>
<td>1 1/8&quot;</td>
</tr>
</tbody>
</table>

---

**ANCHOR BOLT LAYOUT**

---

**SLIP BASE & FIXED BASE**

---

**DRAWN BY BILL BEHRMAN**

---

**RICHARD P. ZELDENREICH**

City of West Seattle

---

**LUPES NOVEMBER 4, 2003**
NOTES

1. Pole Base Plate for a Slip Base design shall be 1/4" steel manufactured from ASTM A572 GR. 50 or ASTM A588. Pole Base Plate for a Fixed Base design shall be either 1/4" or 1/2" steel manufactured from ASTM A572 GR. 50, ASTM A588, or 1 1/2" manufactured from ASTM A36. All Pole Base Plate notched surfaces shall be finished smooth.


3. Galvanizing shall be in accordance with AASHTO M 111.

4. See Standard Plan C-8b, C-14h and J-28.55 for foundation and base plate requirements when steel light standards are mounted on concrete traffic barriers.


(Cover not shown for clarity)

ELEVATION VIEW

Configuration and location of the hand hole varies among manufacturers. Minimum size opening shown.

VIEW A

Typical Hand Hole Orientation

(1) The conductor attachment configurations vary among different manufacturers. Conductor attachments are required on all poles, fixed or slip base.

(2) T = Rim plate thickness by luminaire pole fabricator.

VIEW B

Orientation for installation on bridge or retaining wall - see standard plan J-28.45

For details not shown see view A above.

VIEW C

FOR PLATE THICKNESS, REFER TO NOTE 1

VIEW D

SECTION D

(Cover not shown for clarity)

ISOMETRIC VIEW
IMPROVEMENT PLANS
Project No. SP 3265 CHEYNE LANDFILL
Septage Lagoons

YAKIMA COUNTY PUBLIC SERVICES DEPARTMENT
GARY N. EXKOPF, P.E.
COUNTY ENGINEER

APPROVED FOR CONSTRUCTION:

DATE: 7/21/09

PLAN SHEET INDEX

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<tr>
<td>3</td>
<td>GENERAL SITE PLAN</td>
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<td>LAGOON SITE PLAN</td>
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<td>SITE GRADING</td>
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<td>ELECTRICAL / ILLUMINATION DETAILS</td>
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PREPARED UNDER THE DIRECTION OF:
ANDERSON - PERRY & ASSOCIATES, INC.
DATE: 7/21/09

PROJECT ENGINEER:
J. WELLS

CHECKED BY:
E. STEPHEN

COVER SHEET

SHEET 1 OF 19
RADIUS DIMENSIONS DIAGRAM
NOTE: FOR EACH AT ENTRANCE AND EXIT USE BY:

STORM WATER DITCH TYPICAL SECTION

LINER SYSTEM - TYPICAL SECTION

LEACHATE SUMP COLLECTION TABLE

TYPICAL DIKE SECTION
CHEYNE LANDFILL
SEPTAGE
LAGOONS

SP 3265

PREPARED UNDER THE DIRECTION OF:

ANDERSON-PERRY & ASSOCIATES, INC.

DATE: 7/21/69

PROJECT ENGINEER: J. WELS

CHECKED BY: C. ERSKINE

CROSS SECTIONS

NOTE:
ALL WORKPOND ELEVATIONS ARE PREPARED SUBSEQUENT TO LAND AND FINAL GRADE. TOP OF POND
ROAD ELEVATIONS ARE FINISHED GRADE.
GENERAL NOTES

1. SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL SHOWING COMPLETE DIMENSIONS AND DETAILS OF FABRICATION AND GALVANIZING. MATERIALS BEING USED SHALL BE SPECIFIED IN THE SHOP DRAWINGS.

2. ALL STEEL COMPONENTS SHALL BE GALVANIZED PRIOR TO ASSEMBLY.

3. CUTTING SHALL BE DONE BY SAWING OR MILLING AND ALL CUTS SHALL BE TRUE AND SMOOTH.

4. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1.

5. REPAIR OF GALVANIZED STEEL SURFACES DAMAGED BY FIELD OPERATIONS SHALL BE COMPLETE BY PAINTING THE DAMAGED AREA WITH TWO COATS OF PAINT CONFORMING TO FORMULA A-9-73 AS SPECIFIED IN SECTION 9-2.1 OF THE WASHINGTON STATE STANDARD SPECIFICATIONS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION.

6. ALL STEEL STRUCTURES SHALL HAVE A MINIMUM YIELD STRESS OF 50 KSI AND MEET OR EXCEED THE REQUIREMENTS OF ASTM A325. ALL OTHER STEEL SHALL HAVE A MINIMUM YIELD STRESS OF 36 KSI AND MEET OR EXCEED ASTM A36 REQUIREMENTS.

7. UNLESS OTHERWISE SPECIFIED ALL BOLTS SHALL MEET OR EXCEED THE REQUIREMENTS OF ASTM A325. ALL BOLTS AND WASHERS SHALL BE GALVANIZED.

8. DECK GRATING SHALL BE AL-12-4 WITH 1 1/4" X 3/8" BEARING DIAMETERS AS MANUFACTURED BY GROUTING PACIFIC OR APPROVED EQUAL. ALL ENDS AND OPENINGS IN THE GRATING SHALL BE SHOT BOLDED. GROUTING SHALL BE ATTACHED USING "B" CLIPS. A MINIMUM OF TWO CLIPS ARE REQUIRED AT EACH PANEL BEARING END.

CONCRETE

9. CONCRETE SHALL BE AIR ENTRAINMENT CLASS 4000.

10. REINFORCING STEEL SHALL BE AASHTO #1, 0.060 IN. GAUGE.

11. ALL EXPOSED SURFACES ACROSS THE FINISHED GRADE TO RECEIVE A CLASS 1 SURFACE FINISH.

12. ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED WITH A 1/4" CHAMFER.

13. UNLESS OTHERWISE SHOWN, THE MINIMUM REINFORCING BAR SPACING LENGTH SHALL BE 2'-0".

14. UNLESS OTHERWISE SHOWN, CONCRETE COVER FROM THE FACE OF THE CONCRETE TO REINFORCING BARS SHALL BE 2'-0" FOR CONCRETE CAST AGAINST SOIL.

WALKWAY BRIDGE LOCATION TABLE

<table>
<thead>
<tr>
<th>BRIDGE</th>
<th>NORTHING</th>
<th>EASTING</th>
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ELEVATION

SEE SOLDS PONDS OUTLET DETAILS SHEET 14

POST PRODUCTION

SEE LINK TO CONCRETE ATTACHMENT DETAILS SHEET 16

SEE LIP SYSTEM TYPICAL SECTION SHEET 16

ELEVATION

PREPARED UNDER THE DIRECTION OF:

AMANDER PERRY & ASSOCIATES, INC.

PROJECT ENGINEER: J. WELLS

DATE: 11/09

WALKWAY BRIDGE PLAN AND PROFILE

SHEET 11 OF 19
CHEYNE LANDFILL
SEPTAGE
LAGOONS

SP 3265

PREPARED UNDER
THE DIRECTION OF:

ANDERSON—PERRY
& ASSOCIATES, INC.
DATE: 7/21/09

PROJECT ENGINEER:
J. WELL

DRAWN: B.S. / H.K.
CHECKED BY: E. ABDO

WALKWAY
STRUCTURAL DETAILS

SHEET 12 OF 19
GENERAL NOTES
1. FLOOR STAND SUPPORT BRACKET AND MANKHEE SHALL BE HOT DIPPED GALVANIZED. ALL OTHER METAL VALVE COMPONENTS AND FASTENERS SHALL BE STAINLESS STEEL.
2. SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL SHOWING COMPLETE DIMENSIONS AND DETAILS.
3. ANCHORAGE AND SUPPORT DEVICES SHALL BE DESIGNED AND SPECIFIED BY THE VALVE MANUFACTURER.

ELEVATION

12-INCH TELESCOPIC VALVE

SECTION

12" FLO 90° BEND
12" PVC
12" FLO 90° BEND
12" PVC, RESTRAINED
TOP OF FLANGE
12" PIPE TYPICAL, SECTION SHEET 6

SLIP TUBE
3/4" GUIDE ROD
SUPPORT BRACKET

SLIP TUBE
3/4" GUIDE ROD
SUPPORT BRACKET

CLEAR STEM COVER
W/ INDICATOR STEEP
FLOOR STAND
BRIDGE DECK
MINERAL
GENERAL NOTES
1. Contain Overflow Ditch from Top of Embankment to Bottom of Natural Channel.
CHEYNE LANDFILL
SEPTAGE
LAGOONS

SP 3265

PREPARED UNDER
THE DIRECTION OF:

ANDERSON–PERRY
& ASSOCIATES, INC.
DATE: 4/21/09

PROJECT ENGINEER:
J. WILLS

REVISED

WELL
DECOMMISSIONING
PLAN

WELL LOG DETAIL

CASING PERFORATION DETAIL

SOURCE: WASHINGTON STATE DEPARTMENT OF ECOLOGY
WATER WELL REPORT NO. 64-2499:
L.E. DOLLING INC. 1998