CONTRACT SPECIFICATIONS
For The Construction Of:

C 3120; WOODIN ROAD, W.
(Maple Grove Road to Scoon Road)

C 2963; EDISON ROAD, W.
(Swan Road to Sunnyside City Limits)

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Improvement Plans
CERTIFICATE

I HEREBY CERTIFY THAT THE ATTACHED DOCUMENTS, PLANS AND SPECIFICATIONS CONFORM TO ORIGINALS WHICH ARE ON FILE IN THE OFFICE OF THE COUNTY ENGINEER OF YAKIMA COUNTY, WASHINGTON.

GARY N. EKSTEDT, P.E.
COUNTY ENGINEER
Informational
Bid Documents
INSTRUCTIONS TO BIDDERS

DELIVERY OF PROPOSALS

Sealed bids will be received at the following location before the specified time:

Office of the Board of County Commissioners of Yakima County, Room 232, Yakima County Courthouse, Yakima, Washington 98901 until 2:00 p.m. of the bid opening date.

Each proposal, or bid shall be completely sealed in a separate package, addressed to the Engineer of Yakima County with the name of the improvements for which the bid is submitted plainly written on the outside of the package.

No oral, telephonic, facsimile, or telegraphic Bids or modifications shall be accepted.

DATE OF OPENING BIDS

The bid opening date for this project shall be June 7, 2007.

The bids shall be publicly opened and read after 2:00 p.m. on that date at the following location:

Yakima County Road Engineer’s Office, fourth floor, Yakima County Courthouse, 128 N. 2nd Street, Yakima, Washington 98901.

RIGHT TO REJECT BIDS:

The right is reserved to reject any and all proposals, to accept the proposal or proposals deemed best for the County or to advertise for new proposals when in the opinion of the Board the best interest of the County shall be promoted thereby.

PROPOSAL GUARANTY:

A certified check, cashier’s check, cash or bid bond made payable to the Treasurer of the County of Yakima for an amount equal to at least five percent (5%) of the total amount bid must accompany each bid as evidence of good faith and as a guarantee that if awarded the Contract the bidder shall execute the Contract and give Bond as required.

FORM FURNISHED:

Each bid must be made on the form attached to these Specifications.

YAKIMA COUNTY IS AN EQUAL OPPORTUNITY EMPLOYER
PROPOSAL

C 3120 - WOODIN ROAD, W., Maple Grove Road to Scoon Road
C 2963 - EDISON ROAD, W., Swan Road to Sunnyside City Limits

BIDDER SHALL BID ONLY ONE OF THE TWO ALTERNATIVES AVAILABLE, EITHER
ALTERNATE "A", OR ALTERNATE "B"

☐ ALTERNATE "A"
COUNTY SUPPLIED CRUSHED SURFACING MATERIALS

BID AMOUNT
PRICE ADJUSTMENT
12,020 TONS CSBC @ $5.00 PER TON = $60,100.00

TOTAL BID (FOR COMPARATIVE PURPOSES) $ ________________

☐ ALTERNATE "B"
CONTRACTOR SUPPLIED CRUSHED SURFACING MATERIALS

BID AMOUNT
PRICE ADJUSTMENT

$ ________________ 0.00

$ ________________

The Total Bid for either Alternate "A" or Alternate "B" shall be used for the contract
and bond amount.
This certifies that the undersigned has examined the location of the noted projects:

C 3120 - WOODIN ROAD, W.
C 2963 - EDISON ROAD, W.

And that the Plans, Specifications and Contract governing the work embraced in these improvements, and the method by which payment will be made for said work, is understood. The undersigned hereby proposes to undertake and complete the work embraced in these improvements, or as much as can be completed with the money available, in accordance with the said Plans, Specifications, and Contract, and the following schedule of rates and prices:

NOTE: Unit Prices for all items, all extensions, and total amount of bid shall be shown. Sales Tax shall be included in Unit Prices. No oral, telephonic, facsimile, or telegraphic Bids or modifications shall be considered or accepted.

<table>
<thead>
<tr>
<th>Item No.</th>
<th>Description</th>
<th>Approx Quantity</th>
<th>Unit</th>
<th>Unit Price</th>
<th>Total Item Amount</th>
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</thead>
<tbody>
<tr>
<td>1</td>
<td>MOBILIZATION</td>
<td>1</td>
<td>L.S.</td>
<td>$</td>
<td>$</td>
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<tr>
<td>2</td>
<td>CLEARING AND GRUBBING</td>
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<td>$</td>
<td>$</td>
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<td>3</td>
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<td>1</td>
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<td>4</td>
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<td>5</td>
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<td>PREPARATION</td>
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<td>6</td>
<td>ROADWAY EXCAVATION INCL. HAUL</td>
<td>6,990</td>
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<td></td>
<td>DRAINAGE</td>
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<td>7</td>
<td>QUARRY SPALLS (Truck Measure)</td>
<td>10</td>
<td>C.Y.</td>
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<td>$</td>
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<td>8</td>
<td>UNDERDRAIN PIPE 10 IN. DIAM.</td>
<td>30</td>
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<td>9</td>
<td>UNDERDRAIN PIPE 24 IN. DIAM.</td>
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<td>10</td>
<td>SCHEDULE A CULVERT PIPE 12 IN. DIAM.</td>
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<td>11</td>
<td>SCHEDULE A CULVERT PIPE 18 IN. DIAM.</td>
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<td>12</td>
<td>SCHEDULE A CULVERT PIPE 24 IN. DIAM.</td>
<td>5</td>
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<td>13</td>
<td>SCHEDULE A CULVERT PIPE 36 IN. DIAM.</td>
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<td>Description</td>
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<td>14</td>
<td>CATCH BASIN TYPE 1L</td>
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<td>EACH</td>
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<tr>
<td>15</td>
<td>CATCH BASIN TYPE 1</td>
<td>10</td>
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<td>16</td>
<td>CATCH BASIN TYPE 2 48 IN. DIAM.</td>
<td>4</td>
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<td>CATCH BASIN TYPE 2 60 IN. DIAM.</td>
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<td>EACH</td>
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<td>18</td>
<td>SCHEDULE A STORM SEWER PIPE 12 IN. DIAM.</td>
<td>322</td>
<td>L.F.</td>
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<td>LEAN CONCRETE</td>
<td>16</td>
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<tr>
<td>20</td>
<td>CRUSHED SURFACING BASE COURSE</td>
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<td>CRUSHED SURFACING TOP COURSE</td>
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<td>$</td>
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<td>22</td>
<td>ASPHALT CRS-2P</td>
<td>73</td>
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<td>23</td>
<td>AGG. FROM STOCKPILE FOR BST</td>
<td>252</td>
<td>C.Y.</td>
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<td>ASPHALT CONCRETE PAVEMENT</td>
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<td>25</td>
<td>HMA CL. 1/2 IN. PG 64-28</td>
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<td>26</td>
<td>PVC FOR IRRIGATION 3 IN. DIAM.</td>
<td>60</td>
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<td>PVC FOR IRRIGATION 4 IN. DIAM.</td>
<td>300</td>
<td>L.F.</td>
<td>$</td>
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<td>28</td>
<td>GATE VALVE 4 IN. DIAM.</td>
<td>2</td>
<td>EACH</td>
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<td>29</td>
<td>ESC LEAD</td>
<td>10</td>
<td>DAY</td>
<td>$</td>
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<td>30</td>
<td>SILT FENCE</td>
<td>1,310</td>
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<td>31</td>
<td>MULCHING WITH PAM</td>
<td>2</td>
<td>ACRE</td>
<td>$</td>
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<td>32</td>
<td>SEEDING, FERTILIZING, AND MULCHING</td>
<td>2.5</td>
<td>ACRE</td>
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<td>TOPSOIL TYPE B</td>
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<td>TRAFFIC</td>
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<td>OTHER ITEMS</td>
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<td>--------------------------------------------</td>
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<tr>
<td>34</td>
<td>CEMENT CONC. TRAFFIC CURB AND GUTTER</td>
<td>3,630</td>
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<td>36</td>
<td>OTHER TEMPORARY TRAFFIC CONTROL</td>
<td>1</td>
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<td>$</td>
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<tr>
<td>37</td>
<td>FLAGGERS AND SPOTTERS</td>
<td>1,200</td>
<td>HOUR</td>
<td>$</td>
<td>$</td>
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<tr>
<td>38</td>
<td>TRAFFIC CONTROL SUPERVISOR</td>
<td>1</td>
<td>L.S.</td>
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<td>$</td>
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<tr>
<td>39</td>
<td>CONSTRUCTION SIGNS CLASS A</td>
<td>217</td>
<td>S.F.</td>
<td>$</td>
<td>$</td>
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<td>40</td>
<td>SHORING OR EXTRA EXCAVATION CLASS B</td>
<td>1,050</td>
<td>S.F.</td>
<td>$</td>
<td>$</td>
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<td>41</td>
<td>GRAVEL BACKFILL FOR DRYWELLS</td>
<td>320</td>
<td>TON</td>
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<td>$</td>
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<td>42</td>
<td>GRAVEL BACKFILL FOR PIPE BEDDING AND TRENCH</td>
<td>109</td>
<td>TON</td>
<td>$</td>
<td>$</td>
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<td>43</td>
<td>MINOR CHANGES</td>
<td>EST.</td>
<td>F.A.</td>
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<td>5,000.00</td>
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<td>1</td>
<td>L.S.</td>
<td>$</td>
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<tr>
<td>45</td>
<td>MAILBOX SUPPORT TYPE 1</td>
<td>17</td>
<td>EACH</td>
<td>$</td>
<td>$</td>
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<tr>
<td>46</td>
<td>MAILBOX SUPPORT TYPE 2</td>
<td>6</td>
<td>EACH</td>
<td>$</td>
<td>$</td>
</tr>
</tbody>
</table>

**BID AMOUNT**: $   

**NOTE**: BIDDER MUST COMPLETE PAGE 2 OF BID DOCUMENTS TO CALCULATE THE TOTAL BID.
The bidder is hereby advised that by signature of this proposal he/she is deemed to have acknowledged all requirements and signed all certificates contained herein.

A proposal guaranty in an amount of five percent (5%) of the total bid, based upon the approximate estimate of quantities at the above prices and in the form as indicated below, is attached hereto:

CASH [ ] IN THE AMOUNT OF __________________________

CASHIER’S CHECK [ ] _______________________________ DOLLARS

CERTIFIED CHECK [ ] ($________) PAYABLE TO THE COUNTY TREASURER

PROPOSAL BOND [ ] IN THE AMOUNT OF 5 PERCENT (5%) OF THE BID

Bidder acknowledges receipt of the following Addendums:

<table>
<thead>
<tr>
<th>No.</th>
<th>Date</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
</tr>
</tbody>
</table>

The undersigned has telephoned the Office of the Yakima County Engineer for verification of the number of Addendums issued.

SIGNATURE OF AUTHORIZED OFFICIAL(S)

Title:

Firm Name:

Address:

Phone No.:

Washington Registration No.:

Federal ID Tax No.:

UBI No.:

E-Mail:

Signed and sworn (or affirmed) before me on __________________________

Date

____________________________

NOTARY PUBLIC

My appointment expires __________________________

(Seal and Stamp)

NOTE: (1) This proposal is not transferrable and any alteration of the firm's name entered hereon without prior permission from the County Engineer shall be cause for considering the proposal irregular and subsequent rejection of the bid.

(2) Please refer to Section 1-02.6 of the Standard Specifications, re: “Preparation of Proposal”

(3) Should it be necessary to modify this proposal either in writing or by electronic means, please make reference to the following proposal number in your communications C 3120 & C 2963.
LETTER OF RESPONSIBILITY

Date: __________
County Road Project No.: C 3120 & C 2963

TO: 
BOARD OF COUNTY COMMISSIONERS OF YAKIMA COUNTY, WASHINGTON
(Party awarding principal contract)

Dear Sirs:

I hereby maintain that I am a responsible bidder as contemplated by the policies of the State of Washington (Chapter 157, Laws of Washington of 1937).

a. My permanent place of business is ____________________________, which I have maintained for ________ years.

b. I have adequate plant equipment to do expeditiously and properly the work contemplated for Yakima County, Washington.

DESCRIPTION OF WORK:

C 3120 - Woodin Road, W. (Maple Grove Road to Scoon Road)

C 2963 - Edison Road, W. (Swan Road to Sunnyside City Limits)

I have the following equipment available for this work:

________________________________________________________________________

________________________________________________________________________

c. I have adequate funds to promptly meet obligations incident to this work.

Bank reference: ____________________________

________________________________________________________________________

d. I have had experience in this class of work, having constructed the following improvements.

I hereby certify that the above is a true and accurate statement.

Very truly yours,

__________________________
Contractor

NOTE: This sheet need not be submitted, unless so requested by the Engineer subsequent to opening of bid. This “letter of responsibility” shall not be construed to be a request for Prequalification of bidder.
DEFINITION OF TERMS

In interpreting these specifications, the following definitions shall prevail:


SECRETARY OF TRANSPORTATION: Secretary of Transportation of the State of Washington.

BOARD: The Board of County Commissioners of Yakima County.

ENGINEER: County, or construction engineer, or his duly authorized assistants by whom all explanations and directions necessary for the satisfactory prosecution and completion of the work described in these specifications will be given.

CONTRACTOR: The person, firm, co-partnership, or corporation, or any lawful agent of such person, firm, partnership or corporation constituting one of the principals to the contract and undertaking to perform the work herein specified.

CONTRACT: The Agreement between the Contractor and the County of Yakima acting through the Board of County Commissioners. The contract shall include the accepted "Proposal", "Plans", "Specifications" and "Contract Bond", also any and all supplemental agreements which reasonably could be required to complete the construction of the work in a substantial and acceptable manner.

PROPOSAL: The written offer, or copy thereof of the bidder to perform the work proposed.

PLANS: The officially approved drawings, or reproductions thereof attached to this contract.

SPECIFICATIONS: The directions, provisions and requirements contained herein, together with all written agreements made, or to be made pertaining to the method and manner of performing the work, or to the quantities and qualities of materials to be furnished under the contract.

CONTRACT BOND: The approved form of security furnished by the Contractor and his surety as a guarantee of good faith on the part of the Contractor to execute the work in accordance with the terms of the contract.

LABORATORY: The laboratories of the Department of Transportation, or other laboratories designated by the engineer.

AMOUNT OF THE CONTRACT: For the purpose of awarding the contract and determining the amount of the bond, the lump sum bid, or the summation of the products of the approximate quantities shown on the plans or otherwise stated by the unit prices will be considered the total amount of the bid and the full amount of the contract price.
NON-COLLUSION DECLARATION

I, by signing the proposal, hereby declare, under penalty of perjury under the laws of the United States that the following statements are true and correct:

1. That the undersigned person(s), firm, association or corporation has (have) not, either directly or indirectly, entered into any agreement, participated in any collusion, or otherwise taken any action in restraint of free competitive bidding in connection with the project for which this proposal is submitted.

2. That by signing the signature page of this proposal, I am deemed to have signed and have agreed to the provisions of this declaration.

NOTICE TO ALL BIDDERS

To report bid rigging activities call:

1-800-424-9071

The U. S. Department of Transportation (USDOT) operates the above toll-free “hotline” Monday through Friday, 8:00 a.m. to 5:00 p.m., eastern time. Anyone with knowledge of possible bid rigging, bidder collusion, or other fraudulent activities should use the “hotline” to report such activities.

The “hotline” is part of USDOT’s continuing effort to identify and investigate highway construction contract fraud and abuse and is operated under the direction of the USDOT Inspector General. All information will be treated confidentially and caller anonymity will be respected.
Certification Regarding
Debarment, Suspension, Ineligibility and Voluntary Exclusion
Lower Tier Covered Transactions

This certification is required by the regulations implementing Executive Order 12549, Debarment and Suspension, 29 CFR Part 98, Section 98.510, Participant’s responsibilities. The regulations were published as Part VII of the May 26, 1998 Federal Register (pages 19160-19211).

(BEFORE COMPLETING CERTIFICATION, READ ATTACHED INSTRUCTIONS WHICH ARE AN INTEGRAL PART OF THE CERTIFICATION)

(1) The prospective recipient of federal assistance funds certifies, by submission of this proposal, that neither it nor its principals are presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from participation in this transaction by any federal department or agency.

(2) Where the prospective recipient of federal assistance funds is unable to certify to any of the statements in this certification, such prospective participant shall attach an explanation to this proposal.

Name and Title of Authorized Representative

_____________________________  _______________________
Signature                      Date
CONTRACT

THIS AGREEMENT, made and entered into between Yakima County acting under and by virtue of Titles 36 and 39 RCW, hereinafter called the “COUNTY” and ____________________________, hereinafter called the “CONTRACTOR”.

That in consideration of the terms and conditions contained herein and attached and made a part of this agreement, the parties hereto covenant and agree as follows:

I. The CONTRACTOR shall do all work and furnish all tools, materials and equipment for C 3120 – Woodin Road, W, and C 2963 - Edison Road, W, and shall perform any changes in the work in accordance with the Contract Documents, “Contract Documents” are this Contract, the attached Plans and Specifications and the current edition of the Standard Specifications of the Washington State Department of Transportation and American Public Works Association which are by this reference incorporated herein and made a part hereof. In using said Standard Specifications and Amendments thereto, “Secretary of Transportation”, “Engineer” and like terms used therein will be construed to mean Yakima County Engineer and “State” or “Thurston County” shall mean Yakima County.

II. The CONTRACTOR shall provide and bear the expense of all equipment, material and labor of any sort whatsoever that may be required for the transfer of materials and for constructing and completing the work provided for in the Contract Documents except those items mentioned therein to be furnished by Yakima County.

III. The COUNTY hereby promises and agrees to pay the CONTRACTOR according to the attached Specifications and the schedule of unit or itemized prices at the time and in the manner and upon the conditions provided for in the Contract Documents.

IV. The CONTRACTOR for itself, and for its heirs, executors, administrators, successors and assigns does hereby agree to the full performance of all the covenants herein contained upon the part of the CONTRACTOR.

V. It is further provided that no liability shall attach to the COUNTY by reason of entering into this Contract, except as expressly provided herein.

IN WITNESS WHEREOF, the CONTRACTOR has executed this instrument, on the date indicated below and Yakima County has caused this instrument to be executed in the name of said COUNTY by and through the Board of Yakima County Commissioners on the date indicated below.

Executed by the CONTRACTOR            , 20

BOARD OF YAKIMA COUNTY COMMISSIONERS

Chair

Commissioner

Commissioner

ATTEST: Clerk of the Board

Christina Steiner

Approved as to form:

Deputy Prosecuting Attorney
PERFORMANCE BOND

(KCW 39.08)

KNOW ALL MEN BY THESE PRESENTS, That __________________________, as "PRINCIPAL", and __________________________, a corporation authorized to do business in the State of Washington, as "SURETY", are jointly and severally held and bound unto Yakima County, Washington in the penal sum __________________________ Dollars ($__________) for the payment of which by these presents we jointly and severally bind ourselves, our heirs, executors, administrators, assigns, and successors.

THE CONDITION of this bond is such that WHEREAS, on ________________, 20____, the PRINCIPAL executed a certain Contract with the County, by the terms of which PRINCIPAL agrees to furnish all material and labor and will undertake and complete the construction of for C 3120 - Woodin Road, W. and C 2963 - Edison Road, W. according to the maps, plans and specifications made a part of said Contract, which Contract is attached hereto and by this reference is incorporated herein and made a part hereof. FURTHER, the SURETY agrees to be bound by the laws of the State of Washington and subjected to the jurisdiction of the State of Washington.

NOW, THEREFORE, if the PRINCIPAL shall faithfully perform all the provisions of such contract and pay all laborers, mechanics, subcontractors and materialmen, and all persons who supply such persons or subcontractors with provisions or supplies for the carrying on of such work, then this obligation to be void, otherwise to remain in full force and effect.

Dated this ______ day of __________________________, 20____.

________________________________________
PRINCIPAL

By: ______________________________

Title: ______________________________

________________________________________
Chair of the Board of
Yakima County Commissioners

Date: ________________________________ 20____

SURETY

By: ______________________________

Attorney-in-Fact

Approved as to form:

________________________________________
Deputy Prosecuting Attorney

Name of Local Office of Agent

________________________________________
Address of Local Office Agent

________________________________________
BOND NUMBER

YAKIMA COUNTY CONTRACT NUMBER

C 3120 Woodin Road, W
C 2963 Edison Road, W

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Amendments to Standard Specifications
AMENDMENTS TO THE STANDARD SPECIFICATIONS

C 3120 - WOODIN ROAD, W. IMPROVEMENT PROJECT
(Maple Grove Road to Scoon Road)
C 2963 - EDISON ROAD, W. IMPROVEMENT PROJECT
(Swan Road to Sunnyside City Limits)

YAKIMA COUNTY, WASHINGTON

INTRODUCTION

The following Amendments and Special Provisions shall be used in conjunction with the 2006 Standard Specifications for Road, Bridge, and Municipal Construction.

AMENDMENTS TO THE STANDARD SPECIFICATIONS

The following Amendments to the Standard Specifications are made a part of this contract and supersede any conflicting provisions of the Standard Specifications. For informational purposes, the date following each Amendment title indicates the implementation date of the Amendment or the latest date of revision.

Each Amendment contains all current revisions to the applicable section of the Standard Specifications and may include references which do not apply to this particular project.

SECTION 1-04, SCOPE OF THE WORK
April 3, 2006

1-04.6 Variation in Estimated Quantities
The third paragraph beginning with “If the adjusted final quantity of any items”, is revised to read:

If the adjusted final quantity of any item does not vary from the quantity shown in the proposal by more than 25%, then the Contractor and the Contracting Agency agree that all work under that item will be performed at the original contract unit price.

SECTION 1-06, CONTROL OF MATERIAL
April 3, 2006

1-06.1 Approval of Materials Prior To Use
The second sentence in the first paragraph is revised to read:

The Contractor shall use the Qualified Product List (QPL), the Aggregate Source Approval (ASA) Database, or the Request for Approval of Material (RAM) form.

Number 1 under the second paragraph is revised to read:
1. Shall be new, unless the Special Provisions or Standard Specifications permit otherwise;

1-06.1(1) Qualified Products List (QPL)
This section is supplemented with the following:

The current QPL can be accessed on-line at www.wsdot.wa.gov/biz/mats/QPL/QPL.cfm

The following new sub-section is inserted to follow 1-06.1(2).

1-06.1(3) Aggregate Source Approval (ASA) Database

The ASA is a database containing the results of WSDOT preliminary testing of aggregate sources. This database is used by the Contracting Agency to indicate the approval status of these aggregate sources for applications that require preliminary testing as defined in the contract. The ASA 'Aggregate Source Approval Report' identifies the currently approved applications for each aggregate source listed. The acceptance and use of these aggregates is contingent upon additional job sampling and/or documentation.

Aggregates approved for applications on the ASA 'Aggregate Source Approval Report' not conforming to the specifications, not fulfilling the acceptance requirements, or improperly handled or installed, shall be replaced at the Contractor's expense.

For questions regarding the approval status of an aggregate source, contact the WSDOT Regional Materials Engineer for the Region the source is located in. The Contracting Agency reserves the right to make revisions to the ASA database at anytime.

If there is a conflict between the ASA database and the contract, then the contract shall take precedence over the ASA database in accordance with Section 1-04.2. The ASA database can be accessed on-line at www.wsdot.wa.gov/biz/mats/ASA

1-06.2(2)D Quality Level Analysis

Item 9 under the first paragraph is revised to read:

9. Determine the Composite Pay Factor (CPF) for each lot.

\[
CPF = \frac{f_1(\overline{PF_1}) + f_2(\overline{PF_2}) + \ldots + f_j(\overline{PF_j})}{\sum f_i}
\]

where:

\(f_i = \text{price adjustment factor listed in these Specifications for the applicable material}\)

\(j = \text{number of constituents being evaluated}\)
SECTION 1-07, LEGAL RELATIONS AND RESPONSIBILITIES TO THE PUBLIC
April 2, 2007

1-07.9(1) General
The fifth paragraph is revised to read:

If employing labor in a class not listed in the contract provisions on state funded projects only, the Contractor shall request a determination of the correct wage and benefits rate for that class and locality from the Industrial Statistician, Washington State Department of Labor and Industries (State L&I), and provide a copy of those determinations to the Engineer.

The fifth paragraph is supplemented with the following new paragraph:

If employing labor in a class not listed in the contract provisions on federally funded projects, the Contractor shall request a determination of the correct wage and benefits for that class and locality from the U. S. Secretary of Labor through the project engineer’s office. Generally, the Contractor initiates the request by preparing Standard Form 1444 Request for Authorization of Additional Classification and Rate, available at http://www.wdol.gov/docs/sfl444.pdf, and submitting it to the Project Engineers’ office for further action.

1-07.10 Worker’s Benefits
The fourth paragraph is revised to read:

The Public Works Contract Division of the Washington State Department of Labor and Industries will provide the Contractor with applicable industrial insurance and medical aid classification and premium rates. After receipt of Revenue Release from the Washington State Department of Revenue, the contracting agency will verify through the Department of Labor and Industries that the Contractor is current with respect to the payments of industrial insurance and medical aid premiums.

1-07.15 Temporary Water Pollution/Erosion Control
The first paragraph is revised to read:

In an effort to prevent, control, and stop water pollution and erosion within the project, thereby protecting the work, nearby land, streams, and other bodies of water, the Contractor shall perform all work in strict accordance with all Federal, State, and local laws and regulations governing waters of the State, as well as permits acquired for the project.

1-07.17 Utilities and Similar Facilities
This section is revised to read:

The Contractor shall protect all private and public utilities from damage resulting from the Work. Among others, these utilities include: telephone, telegraph, and power lines; pipelines, sewer and water lines; railroad tracks and equipment; and highway lighting and
signing systems. All costs required to protect public and private utilities shall be at the
Contractor's expense, except as provided otherwise in this section.

Chapter 19.122 of the Revised Code of Washington (RCW) relates to underground utilities.
In accordance with this RCW, the Contractor shall call the One-Number Locator Service for
field location of utilities. If no locator service is available for the area, notice shall be
provided individually to those owners of utilities known to, or suspected of, having
underground facilities within the area of the proposed excavation.

This section is supplemented with the following two new sub-sections:

1-07.17(1) Utility Construction, Removal or Relocation by the Contractor
If the Work requires removing or relocating a utility, the contract will assign the task to the
Contractor or the utility owner. When the task is assigned to the Contractor it shall be
performed in accordance with the Plans and Special Provisions. New utility construction
shall be performed according to the appropriate contract requirements.

To ease or streamline the Work for its own convenience, the Contractor may desire to ask
utility owners to move, remove, or alter their equipment in ways other than those listed in
the Plans or Special Provisions. The Contractor shall make the arrangements and pay all
costs that arise from work performed by the utility owner at the Contractor's request. Two
weeks prior to implementing any such utility work, the Contractor shall submit plans and
details to the Engineer for approval describing the scope and schedule of all work performed
at the Contractors request by the utility owner.

In some cases, the Plans or special provisions may not show all underground facilities. If
the Work requires these to be moved or protected, the Engineer will assign the task to others
or issue a written change order requiring the Contractor to do so as provided in Section 1-
04.4.

1-07.17(2) Utility Construction, Removal or Relocation by Others
Any authorized agent of the Contracting Agency or utility owners may enter the highway
right-of-way to repair, rearrange, alter, or connect their equipment. The Contractor shall
cooperate with such efforts and shall avoid creating delays or hindrances to those doing the
work. As needed, the Contractor shall arrange to coordinate work schedules.

If the contract provides notice that utilities will be adjusted, relocated, replaced, or
constructed by others during the prosecution of the work, the Special Provisions will
establish the utility owners anticipated completion. The Contractor shall carry out the Work
in a way that will minimize interference and delay for all forces involved. Any costs
incurred prior to the utility owners anticipated completion (or if no completion is specified,
within a reasonable period of time) that results from the coordination and prosecution of the
Work regarding utility adjustment, relocation, replacement, or construction shall be at the
Contractor's expense as provided in Section 1-05.14.
When others delay the Work through late removal or relocation of any utility or similar facility, the Contractor shall adhere to the requirements of Section 1-04.5. The Contracting Agency will either suspend Work according to Section 1-08.6, or order the Contractor to coordinate the Work with the work of the utility owner in accordance with Section 1-04.4. When ordered to coordinate the Work with the work of the utility owner, the Contractor shall prosecute the Work in a way that will minimize interference and delay for all forces involved.

SECTION 1-08, PROSECUTION AND PROGRESS

December 4, 2006

1-08.1 Subcontracting

The eighth paragraph (beginning with - On all projects funded with both Contracting Agency funds and Federal assistance ...) is supplemented with the following:

The Contractor has the option of submitting actual MBE/WBE or DBE payment data, on Federally assisted, Federally assisted and Contracting agency funded, and Contracting Agency funded only contracts to the contracting agency on a monthly basis using the Contract Monitoring and Tracking System (CMATS) through the BizWeb application located at http://www.omwbe.wa.gov/bizwebatwashington. Use of CMATS will become a requirement for all contractors effective January 7, 2008.

1-08.3 Progress Schedule

Section 1-08.3 and all subsections are deleted in their entirety and replaced with the following:

1-08.3 Progress Schedule

1-08.3(1) General Requirements

The Contractor shall submit Type A or Type B Progress Schedules and Schedule Updates to the Engineer for approval. Schedules shall show work that complies with all time and order of work requirements in the contract. Scheduling terms and practices shall conform to the standards established in Construction Planning and Scheduling, Second Edition, published by the Associated General Contractors of America. Except for Weekly Look-Ahead Schedules, all schedules shall meet these General Requirements, and provide the following information:

1. Include all activities necessary to physically complete the project.

2. Show the planned order of work activities in a logical sequence.

3. Show durations of work activities in working days as defined in Section 1-08.5.

4. Show activities in durations that are reasonable for the intended work.
5. Define activity durations in sufficient detail to evaluate the progress of individual activities on a daily basis.

6. Show the physical completion of all work within the authorized contract time.

The Contracting Agency allocates its resources to a contract based on the total time allowed in the contract. The Contracting Agency may accept a Progress Schedule indicating an early physical completion date but cannot guarantee the Contracting Agency’s resources will be available to meet an accelerated schedule. No additional compensation will be allowed if the Contractor is not able to meet their accelerated schedule due to the unavailability of Contracting Agency’s resources or for other reasons beyond the Contracting Agency’s control.

If the Engineer determines that the Progress Schedule or any necessary Schedule Update does not provide the required information, then the schedule will be returned to the Contractor for correction and resubmittal.

The Engineer’s approval of any schedule shall not transfer any of the Contractor’s responsibilities to the Contracting Agency. The Contractor alone shall remain responsible for adjusting forces, equipment, and work schedules to ensure completion of the work within the time(s) specified in the contract.

1-08.3(2) Progress Schedule Types
Type A Progress Schedules are required on all projects that do not contain the bid item for Type B Progress Schedule. Type B Progress Schedules are required on all projects that contain the bid item for Type B Progress Schedule. Weekly Look-Ahead Schedules and Schedule Updates are required on all projects.

1-08.3(2)(A) Type A Progress Schedule
The Contractor shall submit five copies of a Type A Progress Schedule no later than 10 days after the date the contract is executed, or some other mutually agreed upon submittal time. The schedule may be a critical path method (CPM) schedule, bar chart, or other standard schedule format. Regardless of which format used, the schedule shall identify the critical path. The Engineer will evaluate the Type A Progress Schedule and approve or return the schedule for corrections within 15 calendar days of receiving the submittal.

1-08.3(2)(B) Type B Progress Schedule
The Contractor shall submit a preliminary Type B Progress Schedule no later than five calendar days after the date the contract is executed. The preliminary Type B Progress Schedule shall comply with all of these requirements and the requirements of Section 1-08.3(1), except that it may be limited to only those activities occurring within the first 60 working days of the project.

The Contractor shall submit five copies of a Type B Progress Schedule no later than 30 calendar days after the date the contract is executed. The schedule shall be
a critical path method (CPM) schedule developed by the Precedence Diagramming Method (PDM). Restraints may be utilized, but may not serve to change the logic of the network or the critical path. The schedule shall display at least the following information:

- Contract Number and Title
- Construction Start Date
- Critical Path
- Activity Description
- Milestone Description
- Activity Duration
- Predecessor Activities
- Successor Activities
- Early Start (ES) and Early Finish (EF) for each activity
- Late Start (LS) and Late Finish (LF) for each activity
- Total Float (TF) and Free Float (FF) for each activity
- Physical Completion Date
- Data Date

The Engineer will evaluate the Type B Progress Schedule and approve or return the schedule for corrections within 15 calendar days of receiving the submittal.

1-08.3(2)C Vacant

1-08.3(2)D Weekly Look-Ahead Schedule
Each week that work will be performed, the Contractor shall submit a Weekly Look-Ahead Schedule showing the Contractor’s and all subcontractors’ proposed work activities for the next two weeks. The Weekly Look-Ahead Schedule shall include the description, duration and sequence of work, along with the planned hours of work. This schedule may be a network schedule, bar chart, or other standard schedule format. The Weekly Look-Ahead Schedule shall be submitted to the Engineer by the midpoint of the week preceding the scheduled work or some other mutually agreed upon submittal time.

1-08.3(3) Schedule Updates
The Engineer may request a Schedule Update when any of the following events occur:

1. The project has experienced a change that affects the critical path.
2. The sequence of work is changed from that in the approved schedule.
3. The project is significantly delayed.
4. Upon receiving an extension of contract time.
The Contractor shall submit five copies of a Type A or Type B Schedule Update within 15 calendar days of receiving a written request, or when an update is required by any other provision of the contract. A “significant” delay in time is defined as 10 working days or 10 percent of the original contract time, whichever is greater.

In addition to the other requirements of this Section, Schedule Updates shall reflect the following information:

1. The actual duration and sequence of as-constructed work activities, including changed work.

2. Approved time extensions.

3. Any construction delays or other conditions that affect the progress of the work.

4. Any modifications to the as-planned sequence or duration of remaining activities.

5. The physical completion of all remaining work in the remaining contract time.

Unresolved requests for time extensions shall be reflected in the Schedule Update by assuming no time extension will be granted, and by showing the effects to follow-on activities necessary to physically complete the project within the currently authorized time for completion.

1-08.3(4) Measurement
No specific unit of measurement shall apply to the lump sum item for Type B Progress Schedule.

1-08.3(5) Payment
Payment will be made in accordance with Section 1-04.1, for the following bid item when it is included in the proposal:

“Type B Progress Schedule”, lump sum.

The Lump Sum price shall be full pay for all costs for furnishing the Type B Progress Schedule and preliminary Type B Progress Schedule.

Payment of 80 percent of the lump sum price will be made upon approval of the Progress Schedule.

Payment will be increased to 100 percent of the lump sum price upon completion of 80 percent of the original total contract award amount.
All costs for providing Type A Progress Schedules and Weekly Look-Ahead Schedules are considered incidental to other items of work in the contract.

No payment will be made for Schedule Updates that are required due to the Contractors operations. Schedule Updates required by events that are attributed to the actions of the Contracting Agency will be paid for in accordance with Section 1-09.4.

1-08.4 Prosecution of Work

The first sentence is revised to read:

The Contractor shall begin work within 21 calendar days from the date of execution of the contract by the Contracting Agency, unless otherwise approved in writing.

1-08.5 Time for Completion

This section is revised to read:

The Contractor shall complete all physical contract work within the number of “working days” stated in the Contract Provisions or as extended by the Engineer in accordance with Section 1-08.8. Every day will be counted as a “working day” unless it is a nonworking day or an Engineer determined unworkable day. A nonworking day is defined as a Saturday, a Sunday, a whole or half day on which the contract specifically prohibits work on the critical path of the Contractor’s approved progress schedule, or one of these holidays: January 1, the third Monday of January, the third Monday of February, Memorial Day, July 4, Labor Day, November 11, Thanksgiving Day, the day after Thanksgiving, and Christmas Day. When any of these holidays fall on a Sunday, the following Monday shall be counted a nonworking day. When the holiday falls on a Saturday, the preceding Friday shall be counted a nonworking day. The days between December 25 and January 1 will be classified as nonworking days.

An unworkable day is defined as a half or whole day the Engineer declares to be unworkable because of weather or conditions caused by the weather that prevents satisfactory and timely performance of the work shown on the critical path of the Contractor’s approved progress schedule. Other conditions beyond the control of the Contractor may qualify for an extension of time in accordance with Section 1-08.8.

Contract time shall begin on the first working day following the 21st calendar day after the date the Contracting Agency executes the contract. If the Contractor starts work on the project at an earlier date, then contract time shall begin on the first working day when onsite work begins. The contract provisions may specify another starting date for contract time, in which case, time will begin on the starting date specified.

Each working day shall be charged to the contract as it occurs, until the contract work is physically complete. If substantial completion has been granted and all the authorized working days have been used, charging of working days will cease. Each week the Engineer will provide the Contractor a statement that shows the number of working days: (1) charged to the contract the week before; (2) specified for the physical completion of the
contract; and (3) remaining for the physical completion of the contract. The statement will also show the nonworking days and any half or whole day the Engineer declares as unworkable. Within 10 calendar days after the date of each statement, the Contractor shall file a written protest of any alleged discrepancies in it. To be considered by the Engineer, the protest shall be in sufficient detail to enable the Engineer to ascertain the basis and amount of time disputed. By not filing such detailed protest in that period, the Contractor shall be deemed as having accepted the statement as correct.

The Engineer will give the Contractor written notice of the physical completion date for all work the contract requires. That date shall constitute the physical completion date of the contract, but shall not imply the Secretary’s acceptance of the work or the contract.

The Engineer will give the Contractor written notice of the completion date of the contract after all the Contractor’s obligations under the contract have been performed by the Contractor. The following events must occur before the Completion Date can be established:

1. The physical work on the project must be complete; and

2. The Contractor must furnish all documentation required by the contract and required by law, to allow the Contracting Agency to process final acceptance of the contract. The following documents must be received by the Project Engineer prior to establishing a completion date:

   a. Certified Payrolls (Federal-aid Projects)
   b. Material Acceptance Certification Documents
   d. FHWA 47 (Federal-aid Projects)
   e. Final Contract Voucher Certification

1-08.8 Extensions of Time
Section 1-08.8 is revised to read:

The Contractor shall submit any requests for time extensions to the Engineer in writing no later than 10 working days after the delay occurs. The requests for time extension shall be limited to the affect on the critical path of the Contractor’s approved schedule attributable to the change or event giving rise to the request.

To be considered by the Engineer, the request shall be in sufficient detail (as determined by the Engineer) to enable the Engineer to ascertain the basis and amount of the time requested. The request shall include an updated schedule that supports the request and demonstrates that the change or event: (1) had a specific impact on the critical path, and except in cases of concurrent delay, was the sole cause of such impact, and (2) could not have been avoided by resequencing of the work or by using other reasonable alternatives. If a request
combined with previous extension requests, equals 20 percent or more of the original
contract time then the Contractor’s letter of request must bear consent of Surety. In
evaluating any request, the Engineer will consider how well the Contractor used the time
from contract execution up to the point of the delay and the effect the delay has on any
completion times included in the special provisions. The Engineer will evaluate and
respond within 15 calendar days of receiving the request.

The authorized time for physical completion will be extended for a period equal to the time
the Engineer determines the work was delayed because of:

1. Adverse weather causing the time requested to be unworkable, provided that the
   Engineer had not already declared the time to be unworkable and the Contractor
   has filed a written protest according to Section 1-08.5.

2. Any action, neglect, or default of the Contracting Agency, its officers, or
   employees, or of any other contractor employed by the Contracting Agency.

3. Fire or other casualty for which the Contractor is not responsible.

4. Strikes.

5. Any other conditions for which these Specifications permit time extensions such as:
   a. In Section 1-04.4 if a change increases the time to do any of the work
      including unchanged work.
   b. In Section 1-04.5 if increased time is part of a protest that is found to be a
      valid protest.
   c. In Section 1-04.7 if a changed condition is determined to exist that caused
      a delay in completing the contract.
   d. In Section 1-05.3 if the Contracting Agency does not approve properly
      prepared and acceptable drawings within 30 calendar days.
   e. In Section 1-07.13 if the performance of the work is delayed as a result of
      damage by others.
   f. In Section 1-07.17 if the removal or the relocation of any utility by forces
      other than the Contractor caused a delay.
   g. In Section 1-07.24 if a delay results from all the right of way necessary
      for the construction not being purchased and the special provisions does
      not make specific provisions regarding unpurchased right of way.
h. In Section 1-08.6 if the performance of the work is suspended, delayed, or
interrupted for an unreasonable period of time that proves to be the
responsibility of the Contracting Agency.

i. In Section 1-09.11 if a dispute or claim also involves a delay in
completing the contract and the dispute or claim proves to be valid.

j. In Section 1-09.6 for work performed on a force account basis.

6. If the actual quantity of work performed for a bid item was more than the original
plan quantity and increased the duration of a critical activity. Extensions of time
will be limited to only that quantity exceeding the original plan quantity.

7. Exceptional causes not specifically identified in items 1 through 6, provided the
request letter proves the Contractor had no control over the cause of the delay and
could have done nothing to avoid or shorten it.

Working days added to the contract by time extensions, when time has overran, shall only
apply to days on which liquidated damages or direct engineering have been charged, such as
the following:

If substantial completion has been granted prior to all of the authorized working days
being used, then the number of days in the time extension will eliminate an equal
number of days on which direct engineering charges have accrued. If the substantial
completion date is established after all of the authorized working days have been used,
then the number of days in the time extension will eliminate an equal number of days
on which liquidated damages or direct engineering charges have accrued.
The Engineer will not allow a time extension for any cause listed above if it resulted
from the Contractor’s default, collusion, action or inaction, or failure to comply with
the contract.
The Contracting Agency considers the time specified in the special provisions as sufficient
to do all the work. For this reason, the Contracting Agency will not grant a time extension
for:

- Failure to obtain all materials and workers unless the failure was the result of
exceptional causes as provided above in subsection 7;

- Changes, protests, increased quantities, or changed conditions (Section 1-04) that
do not delay the completion of the contract or prove to be an invalid or
inappropriate time extension request;

- Delays caused by nonapproval of drawings or plans as provided in Section 1-05.3;

- Rejection of faulty or inappropriate equipment as provided in Section 1-05.9;
• Correction of thickness deficiency as provided in Section 5-05.5(1)B.

The Engineer will determine whether the time extension should be granted, the reasons for the extension, and the duration of the extension, if any. Such determination will be final as provided in Section 1-05.1.

SECTION 1-09, MEASUREMENT AND PAYMENT
December 4, 2006

1-09.6 Force Account
The last paragraph under “3. For Equipment” is revised to read:

Copies of the AGC/WSDOT Equipment Rental Agreement will be maintained on the Contracting Agency's web site at www.wsdot.wa.gov.

1-09.9(1) Retainage
The fourth paragraph is revised to read:

Release of the retainage will be made 60 days following the Completion Date (pursuant to RCW 39.12, and RCW 60.28) provided the following conditions are met:

1. On contracts totaling more than $20,000, a release has been obtained from the Washington State Department of Revenue.

2. Affidavits of Wages Paid for the Contractor and all Subcontractors are on file with the Contracting Agency (RCW 39.12.040).

3. A certificate of Payment of Contributions Penalties and Interest on Public Works Contract is received from the Washington State Employment Security Department.

4. Washington State Department of Labor and Industries (per section 1-07.10) shows the Contractor is current with payments of industrial insurance and medical aid premiums.

5. All claims, as provided by law, filed against the retainage have been resolved. In the event claims are filed and provided the conditions of 1, 2, 3 and 4 are met, the Contractor will be paid such retained percentage less an amount sufficient to pay any such claims together with a sum determined by the Contracting Agency sufficient to pay the cost of foreclosing on claims and to cover attorney’s fees.
SECTION 2-02, REMOVAL OF STRUCTURES AND OBSTRUCTIONS
April 2, 20072-02.3(3)

Removal of Pavement, Sidewalks, Curbs, and Gutters
Item 3. is revised to read:

3. Make a vertical full depth saw cut between any existing pavement, sidewalk, curb, or
gutter that is to remain and the portion to be removed. For portland cement concrete
pavement removal, a second vertical full depth relief saw cut offset 12 inches to 18
inches from and parallel to the initial saw cut is also required, unless the Engineer
approves otherwise.

SECTION 2-03, ROADWAY EXCAVATION AND EMBANKMENT
August 7, 2006

2-03.3(2) Rock Cuts
This section is revised to read:

1. **Preserving Rock Below Subgrade.** The Contractor shall take care not to break down,
loosen, or damage the rock under the subgrade line, except as provided by Section 2-
03.3(3). Normally cuts will be made from the top, lift by lift, to protect the rock bench
that will remain. The Contractor shall be responsible for methods used and for any
damage caused to the roadbed, regardless of any previous approvals by the Engineer.

2. **Scaling and Dressing.** To leave rock cuts in a safe, stable condition, the Contractor
shall scale and dress them, removing all loose fragments and rocks not firmly fastened
to the rock slope. The Contractor shall also remove any overhanging rock the Engineer
sees as a hazard to roadway users.

If the Engineer requires it, the Contractor shall remove loose fragments and rocks lying
outside the slope stakes. Payment for such extra work shall be by force account as
provided in Section 1-09.6. The Contracting Agency will pay for loading and hauling
these materials at the unit contract prices that apply or as provided in Section 1-04.4.

3. **Drilling and Blasting.** Not less than two weeks prior to commencing drilling and
blasting operations or at any time the Contractor proposes to change the drilling and
blasting methods, the Contractor shall submit a blasting plan to the Engineer for review.
The blasting plan shall contain the full details of the drilling and blasting patterns and
controls the Contractor proposes to use for both the controlled and production blasting.
The blasting plan submittal is required for all blasting operations and shall contain the
following minimum information:

   a) Station limits of proposed shot.
b) Plan and section views of proposed drill pattern including free face, burden, blast hole spacing, blast hole diameter, blast hole angles, lift height, and subdrill depth.

c) Loading diagram showing type and amount of explosives, primers, initiators, and location and depth of stemming.

d) Initiation sequence of blast holes including delay times and delay system.

e) Manufacturer's data sheets for all explosives, primers, and initiators to be employed.

Review of the blasting plan by the Engineer shall not relieve the Contractor of the responsibility for the accuracy and adequacy of the plan when implemented in the field.

When blasting to establish slopes 1/2 to 1 or steeper, and more than 10 feet high, the Contractor shall use controlled blasting. The Engineer may require the Contractor to use controlled blasting to form the faces of other slopes, even if the slopes could be formed by nonblasting methods.

Controlled blasting refers to the controlled use of explosives and blasting accessories in carefully spaced and aligned drill holes to provide a free surface or shear plane in the rock along the specified backslope. Controlled blasting techniques covered by this specification include presplitting and cushion blasting.

In addition to the blasting plan submittal, when using controlled blasting the Contractor shall:

a) Prior to commencing full-scale blasting operations, the Contractor shall demonstrate the adequacy of the proposed blast plan by drilling, blasting, and excavating short test sections, up to 100 feet in length, to determine which combination of method, hole spacing, and charge works best. When field conditions warrant, the Contractor may be ordered to use test section lengths less than 100 feet.

Unless otherwise approved by the Engineer, the Contractor shall begin the tests with the controlled blast holes spaced 30-inches apart, then adjust if needed, until the Engineer approves the spacing to be used for full-scale blasting operations.

b) The Contractor shall completely remove all overburden soil and loose or decomposed rock along the top of the excavation for a distance of at least 30 feet beyond the end of the production hole drilling limits, or to the end of the cut, before drilling the presplitting holes.
c) The controlled blast holes shall be not less than $2^{1/2}$ inches nor more than 3 inches in diameter.

d) The Contractor shall control drilling operations by the use of the proper equipment and technique to ensure that no hole shall deviate from the plane of the planned slope by more than 9 inches either parallel or normal to the slope. Drill holes exceeding these limits shall not be paid for unless satisfactory slopes are being obtained.

e) Controlled blast holes shall extend a minimum of 30 feet beyond the limits of the production holes to be detonated, or to the end of the cut as applicable.

f) The length of controlled blast holes for any individual lift shall not exceed 20 feet unless the Contractor can demonstrate to the Engineer the ability to stay within the above tolerances and produce a uniform slope. If greater than 5 percent of the presplit holes are misaligned in any one lift, the Contractor shall reduce the height of the lifts until the 9-inch alignment tolerance is met. Upon satisfactory demonstration, the length of holes may be increased to a maximum of 60 feet with written approval of the Engineer.

g) When the cut height requires more than one lift, a maximum 2-foot offset between lifts will be permitted to allow for drill equipment clearances. The Contractor shall begin the control blast hole drilling at a point that will allow for necessary offsets and shall adjust, at the start of lower lifts, to compensate for any drift that may have occurred in the upper lifts.

h) Before placing charges, the Contractor shall determine that the hole is free of obstructions for its entire depth. All necessary precautions shall be exercised so that the placing of the charges will not cause caving of material from the walls of the holes.

i) The maximum diameter of explosives used in presplit holes shall not be greater than $1/2$ the diameter of the presplit hole.

j) Only standard explosives manufactured especially for controlled blasting shall be used in controlled blast holes, unless otherwise approved by the Engineer. Bulk ammonium nitrate and fuel oil (ANFO) shall not be allowed to be loaded in the presplit holes.

k) If fractional portions of standard explosive cartridges are used, they shall be firmly affixed to the detonating cord in a manner that the cartridges will not slip down the detonating cord nor bridge across the hole. Spacing of fractional cartridges along the length of the detonating cord shall not exceed 30 inches center to center and shall be adjusted to give the desired results.
l) Continuous column cartridge type of explosives used with detonating cord shall be assembled and affixed to the detonating cord in accordance with the explosive manufacturer’s instructions, a copy of which shall be furnished to the Engineer.

m) The bottom charge of a presplit hole may be larger than the line charges but shall not be large enough to cause overbreak. The top charge of the presplitting hole shall be placed far enough below the collar, and reduced sufficiently, to avoid overbreaking and heaving.

n) The upper portion of all presplit holes, from the top most charge to the hole collar, shall be stemmed. Stemming materials shall be sand or other dry angular material, all of which passes a 3/8-inch sieve.

o) If presplitting is specified, the detonation of these holes shall be fired first.

p) If cushion blasting is specified, the detonation of these holes shall be fired last on an instantaneous delay after all other blasting has taken place in the excavation.

q) Production blast holes shall not be drilled closer than 6 feet to the controlled blast line, unless approved by the Engineer. The bottom of the production holes shall not be lower than the bottom of the controlled blast holes. Production holes shall not exceed 6 inches in diameter, unless approved by the Engineer. Detonation of production holes shall be on a delay sequence toward a free face.

r) The use of horizontal blast holes for either production or controlled blasting is prohibited.

SECTION 2-09, STRUCTURE EXCAVATION

April 2, 2007

2-09.3(1)E Backfilling

Paragraphs three through nine including the Controlled Density Fill (CDF) chart are deleted and replaced with the following:

Alternative Sources. When material from structure excavation is unsuitable for use as backfill, the Engineer may:

1. require the Contractor to use other material covered by the contract if such substitution involves work that does not differ materially from what would otherwise have been required;
2. require the Contractor to substitute selected material in accordance with Section 2-03.3(10);

3. require the Contractor to use Controlled Density Fill (CDF) also known as Controlled Low Strength Material (CLSM), or;

4. require the Contractor to obtain material elsewhere. Material obtained elsewhere will be paid for in accordance with Section 1-04.4.

**Controlled Density Fill (CDF) or Controlled Low-Strength Material (CLSM).** CDF is a self compacting, cementitious, flowable material requiring no subsequent vibration or tamping to achieve consolidation. The Contractor shall provide a mix design in writing to the Engineer on WSDOT Form 350-040 and utilize ACI 229 as a guide to develop the CDF mix design. No CDF shall be placed until the Engineer has reviewed the mix design. CDF shall be designed to have a minimum 28-day strength of 50 psi and a maximum 28-day strength not to exceed 300 psi. The CDF consistency shall be flowable (approximate slump 3 to 10 inches).

The following testing methods shall be used by the Contractor to develop the CDF mix design:

28 day compressive strength - ASTM D 4832,
Unit weight, yield, and air content – ASTM D 6023,
Test for slump shall be in accordance with WSDOT FOP for AASHTO T 119.

The water/cement ratio shall be calculated on the total weight of cementitious material. The following are considered cementitious materials: Portland cement, fly ash, ground granulated blast furnace slag and microsilica fume.

Admixtures used in CDF shall meet the requirements of Section 9-23.6. Admixtures for Concrete, and foaming agents, if used, shall meet the requirements of ASTM C 869. Admixtures shall be used in accordance with the manufacturer’s recommendations and non-chloride accelerating admixtures may be used to accelerate the hardening of CDF.

CDF shall meet the requirements of Section 6-02.3(5)C and shall be accepted based on a Certificate of Compliance. The producer shall provide a Certificate of Compliance for each truckload of CDF in accordance with Section 6-02.3(5)B.

Item 1 of the first paragraph under Compaction is revised to read:

1. Backfill supporting roadbed, roadway embankments, or structures, including backfill providing lateral support for noise barrier wall foundations, luminaire poles, traffic signal standards, and roadside and overhead sign structure foundations — placed in horizontal layers no more than 6 inches thick with each layer compacted to 95 percent of the maximum density determined by the Compaction Control Test, Section 2-03.3(14)D.
2-09.3(3)B Excavation Using Open Pits — Extra Excavation
This section is revised to read:

The Contractor may dig open pits or perform extra excavation without shoring or cofferdams, if:

1. Footings can be placed in dry material away from running water.

2. The integrity of the completed structure and its surroundings is not reduced.

3. Worker safety is ensured as required by law.

4. The excavation does not disturb the existing pavement or any other adjacent structural elements.

If a slide occurs in an open pit, the Contractor shall remove the slide material. If the slide distorts an area over which a highway will be built, the Contractor shall backfill and compact the site to the original ground line as approved by the Engineer. If the slide damages an existing facility such as a roadway or structure, the Contractor shall repair the damage caused by the slide. The Contractor shall pay all costs related to removing slide material and restoring the slide area, including the repair of any pavement or structural elements damaged by the slide.

The Contractor shall drain or pump any water from the pit, taking care not to stir up or soften the bottom. If equipment in the pit or inadequate water removal makes the foundation material unstable, the Contractor shall, at no expense to the Contracting Agency, remove and replace it with material the Engineer approves.

When the Engineer believes ground water flow may impair a concrete footing, the Contractor shall place under it a layer of gravel at least 6 inches thick. Before placing the gravel, the Contractor shall excavate to whatever grade the Engineer requires. This provision shall not apply to the building of concrete seals.

The Contractor may omit forms when the earthen sides of a footing excavation will stand vertically. In this case, the Contractor may excavate to the neat line dimensions of the footing and pour concrete against the undisturbed earth. If the hole is larger than neat line dimensions, the Contractor shall bear the cost of the extra concrete.

For open temporary cuts, the following requirements shall be met:

1. No vehicular or construction traffic, or construction surcharge loads will be allowed within a distance of 5-feet from the top of the cut.

2. Exposed soil along the slope shall be protected from surface erosion.
3. Construction activities shall be scheduled so that the length of time the temporary cut is left open is reduced to the extent practical.

4. Surface water shall be diverted away from the excavation.

Submittals and Design Requirements. The Contractor shall submit working drawings and calculations showing the geometry and construction sequencing of the proposed excavation slopes. The Contractor shall not begin excavation operations until receiving the Engineer's approval of the excavation submittal.

The excavation stability design shall be conducted in accordance with the WSDOT Geotechnical Design Manual (M46-03). The stability of the excavation slopes shall be designed for site specific conditions which shall be shown and described in the working drawings. Examples of such items that shall be shown on the excavation submittal and supported by calculations include, but are not limited to, the following:

1. Excavation geometry and controlling cross sections showing adjacent existing foundations, utilities, site constraints, and any surcharge loading conditions that could affect the stability of the slope;

2. A summary clearly describing subsurface soil and groundwater conditions, sequencing considerations, and governing assumptions;

3. Any supplemental subsurface explorations made to meet the requirements for geotechnical design of excavation slopes, in accordance with the WSDOT Geotechnical Design Manual;

4. Supporting geotechnical calculations used to design the excavation, the soil and material properties selected for design, and the justification for the selection for those properties, in accordance with the WSDOT Geotechnical Design Manual;

5. Safety factors, or load and resistance factors used, and justification for their selection, in accordance with the WSDOT Geotechnical Design Manual, and referenced AASHTO design manuals;

6. Location and weight of construction equipment adjacent to the excavation top, and location of adjacent traffic; and,

7. A monitoring plan to evaluate the excavation performance throughout its design life.

2-09.3(3)D Shoring and Cofferdams

Paragraphs one through seven are revised to read:

Definitions. Structural shoring is defined as a shoring system that is installed prior to excavation. Structural shoring shall provide lateral support of soils and limit lateral
movement of soils supporting structures, roadways, utilities, railroads, etc., such that these items are not damaged as a result of the lateral movement of the supporting soils.

Structural shoring systems include driven cantilever sheet piles, sheet piles with tiebacks, sheet pile cofferdams with wale rings or struts, prestressed spud piles, cantilever soldier piles with lagging, soldier piles with lagging and tiebacks, and multiple tier tieback systems.

Trench boxes, sliding trench shields, jacked shores, shoring systems that are installed after excavation, and soldier pile, sheet pile, or similar shoring walls installed in front of a pre-excavated slope, are not allowed as structural shoring.

A cofferdam is any watertight enclosure, sealed at the bottom and designed for the dewatering operation, that surrounds the excavated area of a structure. The Contractor shall use steel sheet pile or interlocking steel pile cofferdams in all excavation that is under water or affected by ground water.

Submittals and Design Requirements. The Contractor shall submit working drawings and calculations showing the proposed methods and construction details of structural shoring or cofferdams in accordance with Sections 6-01.9 and 6-02.3(16). The Contractor shall not begin construction of structural shoring or cofferdams, nor begin excavation operations, until approval of the structural shoring submittal has been given by the Project Engineer.

Structural shoring and cofferdams shall be designed for conditions stated in this Section using methods shown in Division I Section 5 of the AASHTO Standard Specifications for Highway Bridges Seventeenth Edition - 2002 for allowable stress design, or the AASHTO LRFD Bridge Design Specifications, Third Edition, 2004 and current interims for load and resistance factor design. The USS Steel Sheet Piling Design Manuals, published by United States Steel, may be used for shoring walls that do not support other structures and that are 15 feet in height or less. Allowable stresses for materials shall not exceed stresses and conditions allowed by Section 6-02.3(17)B. The shoring design shall also be in compliance with the WSDOT Geotechnical Design Manual (M46-03). In the case of conflict or discrepancy between manuals, the Geotechnical Design Manual shall govern.

For open temporary cuts associated with a shoring system, the requirements for open temporary cuts specified in Section 2-09.3(3)B shall be met.

The structural shoring system shall be designed for site specific conditions which shall be shown and described in the working drawings. The structural shoring system design shall include the design of the slopes for stability above and below the shoring system. Except as otherwise noted, the design height of all structural shoring in design calculations and working drawings shall be for the depth of excavation as required by the Plans, plus an additional 2 feet to account for the possibility of overexcavation. If the Contractor provides written documentation to the satisfaction of the Engineer that the soil conditions at the site are not likely to require overexcavation, the Engineer may waive the requirement for two feet of overexcavation design height.
Examples of such items that shall be shown on the structural shoring submittal and supported by calculations include, but are not limited to, the following:

1. Heights; soil slopes; soil benches; and controlling cross sections showing adjacent existing foundations, utilities, site constraints, and any surcharge loading conditions that could affect the stability of the shoring system, including any slopes above or below the shoring.

2. A summary clearly describing performance objectives, subsurface soil and groundwater conditions, sequencing considerations, and governing assumptions.

3. Any supplemental subsurface explorations made to meet the requirements for geotechnical design of excavation slopes, shoring walls, and other means of ground support, in accordance with the WSDOT Geotechnical Design Manual.

4. Supporting geotechnical calculations used to design the shoring system, including the stability evaluation of the shoring system in its completed form as well as intermediate shoring system construction stages, the soil and material properties selected for design, and the justification for the selection for those properties, in accordance with the WSDOT Geotechnical Design Manual.

5. Safety factors, or load and resistance factors used, and justification for their selection.

6. Location and weight of construction equipment adjacent to the excavation; location of adjacent traffic; and structural shoring system material properties, spacing, size, connection details, weld sizes, and embedment depths.

7. Structural shoring installation and construction sequence, procedure, length of time for procedure and time between operations; proof load testing procedure if any; deadman anchor design and geometry; no load zones; grouting material and strengths; and a list of all assumptions.

8. Methods and materials to be used to fill voids behind lagging, when soldier piles with lagging are used as structural shoring.

9. A monitoring/testing plan to evaluate the performance of the excavation/shoring system throughout its design life, and

10. An estimate of expected displacements or vibrations, threshold limits that would trigger remedial actions, and a list of potential remedial actions should thresholds be exceeded. Thresholds shall be established to prevent damage to adjacent facilities, as well as degradation of the soil properties due to deformation.
SECTION 2-12 CONSTRUCTION GEOTEXTILE
August 7, 2006

The section title is revised to read:

CONSTRUCTION GEOSYNTHETIC

2-12 CONSTRUCTION GEOTEXTILE

This heading is revised to read:

2-12 CONSTRUCTION GEOSYNTHETIC

2-12.1 Description
The word geotextile is revised to geosynthetic.

2-12.2 Materials
In the first and second paragraphs geotextile is revised to geosynthetic.

2-12.3 Construction Requirements
In the first, second, and third paragraphs geotextile is revised to geosynthetic.

SECTION 3-01, PRODUCTION FROM QUARRY AND PIT SITES
August 7, 2006

3-01.4(1) Acquisition and Development
The first paragraph is revised to read:

If, under the terms of the Contract, the Contractor is required to provide a source of materials, or if the Contractor elects to use materials from sources other than those provided by the Contracting Agency, the Contractor shall, at no expense to the Contracting Agency, make all necessary arrangements for obtaining the material and shall ensure the quantity of suitable material is available. Preliminary samples shall be taken by or in the presence of the Engineer or a designated representative unless the Engineer permits otherwise. Approval of the source does not relieve the Contractor from meeting these specification requirements, nor does it guarantee that the material will meet these requirements without additional or proper processing. The Engineer may require additional preliminary samples at any time.

SECTION 5-04, HOT MIX ASPHALT
April 2, 2007

5-04.3(1) HMA Mixing Plant
The first paragraph is supplemented with the following:

4. Sampling HMA. The HMA plant shall provide for sampling HMA by one of the following methods:
a. A mechanical sampling device attached to the HMA plant.

b. Platforms or devices to enable sampling from the hauling vehicle without entering the hauling vehicle.

5-04.3(8)A Acceptance Sampling and Testing—HMA Mixture

Item 3 in this section is revised to read:

3. Sampling. Samples for acceptance testing shall be obtained by the Contractor when ordered by the Engineer. The Contractor shall sample the HMA mixture in the presence of the Engineer and in accordance with WSDOT FOP for WAQTC/AASHTO T 168.

5-04.3(10)B Control

The second paragraph in item 3. is revised to read:

For compaction lots falling below a 1.00 pay factor and thus subject to price reduction or rejection, the Contractor may request that cores be used for acceptance of HMA compaction. When cores are taken by the Contracting Agency at the request of the Contractor, they shall be requested by noon of the next workday after receiving the test results. The cores will be taken at approximately the same locations as the nuclear density gauge tests in the compaction lot being challenged. When the CPF for the lot based on the results of the HMA cores is less than 1.00, the cost for the coring will be deducted from any monies due or that may become due the Contractor under the contract at the rate of $125 per core.

SECTION 6-02, CONCRETE STRUCTURES

April 2, 2007

6-02.3(2) Proportioning Materials

The third paragraph is revised to read:

The use of fly ash is required for Class 4000D and 4000P concrete, except that ground granulated blast furnace slag may be substituted for fly ash at a 1:1 ratio. The use of fly ash and ground granulated blast furnace slag is optional for all other classes of concrete.

6-02.3(2)A Contractor Mix Design

The first paragraph is revised to read:

The Contractor shall provide a mix design in writing to the Engineer for all classes of concrete specified in the Plans except for those accepted based on a Certificate of Compliance. No concrete shall be placed until the Engineer has reviewed the mix design. The required average 28 day compressive strength shall be selected per ACI 318, Chapter 5, Section 5.3.2. ACI 211.1 and ACI 318 shall be used to determine proportions. The proposed mix for Class 4000P shall provide a minimum fly ash or ground granulated blast furnace slag content per cubic yard of 100 pounds, and a minimum cement content per cubic yard of 600 pounds. The proposed mix for Class 4000D shall provide a minimum fly ash or ground
granulated blast furnace slag content per cubic yard of 75 pounds, and a minimum cement content per cubic yard of 660 pounds. All other concrete mix designs, except those for lean concrete and commercial concrete, shall have a minimum cementitious material content of 564 pounds per cubic yard of concrete.

The first sentence of the second paragraph is revised to read:

The Contractor's submittal of a mix design shall be on WSDOT form 350-040 and shall provide a unique identification for each mix design and shall include the mix proportions per cubic yard, the proposed sources, the average 28 day compressive strength for which the mix is designed, the fineness modulus, and the water cement ratio.

The following new sentence is inserted after the first sentence in the fourth paragraph.

An alternate combined aggregate gradation conforming to Section 9-03.1(5) may also be used.

6-02.3(4)(4A) Qualification of Concrete Suppliers
The first paragraph and the entire second paragraph (1 through 4) are deleted and replaced with the following:

Batch Plant Prequalification may be obtained through one of the following methods:

1. Certification by the National Ready Mix Concrete Association (NRMCA). Information concerning NRMCA certification may be obtained from the NRMCA at 900 Spring Street, Silver Springs, MD 20910 or online at www.nrmca.org. The NRMCA certification shall be good for a two year period. When this method of certification is used the following documentation shall be submitted to the project engineer.
   a. A copy of the current NRMCA Certificate of Conformance, the concrete mix design(s) (WSDOT Form 350-040), along with copies of the truck list, batch plant scale certification, admixture dispensing certification, and volumetric water batching devices (including water meters) verification.

2. Independent evaluation certified by a Professional Engineer using NRMCA checklist. The Professional Engineer shall be licensed under title 18 RCW, state of Washington, qualified in civil engineering. The independent certification using the NRMCA checklist shall be good for a two year period. When this method of certification is used the following documentation shall be submitted to the engineer.
   a. A copy of the Professional Engineer's stamped and scaled NRMCA Verification of Inspection and Application for Certificate page from the NRMCA checklist, the concrete mix design(s) (WSDOT Form 350-040), along with copies of the truck list, batch plant scale certification,
3. Inspection conducted by the Plant Manager, defined as the person directly responsible for the daily plant operation, using the NRMCA Plant Certification checklist. The Plant Manager certification shall be done prior to the start of a project, and every six months throughout the life of the project, and meet the following requirements:

a. The Agreement to Regularly Check Scales and Volumetric Batching Dispensers page in the NRMCA Plant Certification checklist shall be signed by the Plant Manager and notarized.

b. The signed and notarized Agreement to Regularly Check Scales and Volumetric Batching Dispensers page and a copy of the NRMCA Plant Certification checklist cover page showing the plant designation, address and Company operating plant shall all be submitted to the Project Engineer with the concrete mix design (WSDOT Form 350-040), along with copies of the truck list, batch plant scale certification, admixture dispensing certification, and volumetric water batching devices (including water meters) verification.

c. The NRMCA Plant Certification checklists shall be maintained by the Plant Manager and are subject to review at any time by the Contracting Agency.

e. Volumetric water batching devices (including water meters) shall be verified every 90 days.

6-02.3(5)C Conformance to Mix Design
Item 2 under the first paragraph is revised to read:

2. Fly ash and ground granulated blast furnace slag weight plus or minus 5 percent of that specified in the mix design.

6-02.3(5)H Sampling and Testing for Compressive Strength
This section including title is revised to read:

6-02.3(5)H Sampling and Testing for Compressive Strength and Initial Curing
Acceptance testing for compressive strength shall be conducted at the same frequency as the acceptance tests for temperature, consistency, and air content.

The Contractor shall provide, and maintain cure boxes for curing concrete cylinders. The Contractor shall also provide, maintain and operate all necessary power sources and connections needed to operate the curing box. Concrete cylinders shall be cured in a cure box in accordance with WSDOT FOP for AASHTO T 23. The cure boxes shall maintain a
temperature between 60°F and 80°F for concrete with specified strengths less than 6000 psi and between 68°F and 78°F for concrete with specified strengths of 6000 psi and higher. A minimum/maximum thermometer shall be installed to measure the internal temperature of the cure box. The thermometer shall be readable from outside of the box and be capable of recording the high and low temperatures in a 24-hour period. The cure boxes shall create an environment that prevents moisture loss from the concrete specimens. The top shall have a working lock and the interior shall be rustproof. A moisture-proof seal shall be provided between the lid and the box. The cure box shall be the appropriate size to accommodate the number of concrete acceptance cylinders necessary or the Contractor shall provide additional cure boxes. Once concrete cylinders are placed in the cure box, the cure box shall not be moved until the cylinders have been cured in accordance with these specifications. When concrete is placed at more than one location simultaneously, multiple cure boxes shall be provided.

The Contractor shall protect concrete cylinders in cure boxes from excessive vibration and shock waves during the curing period in accordance with Section 6-02.3(6)D.

6-02.3(6)A Weather and Temperature Limits to Protect Concrete
The section Cold Weather Protection is revised to read:

NOTE: Table 6-02.3(6) “Surface Evaporation from Concrete”, remains unchanged.

Cold Weather Protection
This Specification applies when the weather forecast predicts air temperatures below 35° F at any time during the seven days following concrete placement. Weather forecast is based on predictions from the Western Region Headquarters of the National Weather Service. This forecast can be found at [http://www.wrh.noaa.gov/](http://www.wrh.noaa.gov/).

To achieve adequate curing, the temperature of the concrete shall be maintained above 50° F during the entire curing period or seven days, whichever is greater. The concrete temperature shall not be allowed to fall below 35 ° F during this time. Prior to placing concrete in cold weather, the Contractor shall provide a written procedure for cold weather concreting to the Engineer. The procedure shall detail how the Contractor will adequately cure the concrete and prevent the concrete temperature from falling below 35° F. Extra protection shall be provided for areas especially vulnerable to freezing (such as exposed top surfaces, corners and edges, thin sections, and concrete placed into steel forms). Concrete placement will only be allowed if the Contractor’s cold weather protection plan has been approved by the Engineer.

The Contractor shall not mix nor place concrete while the air temperature is below 35° F, unless the water or aggregates (or both) are heated to at least 70° F. The aggregate shall not exceed 150° F. If the water is heated to more than 150° F, it shall be mixed with the aggregates before the cement is added. Any equipment and methods shall heat the materials evenly. Concrete placed in shafts and piles is exempt from such preheating requirements.
The Contractor may warm stockpiled aggregates with dry heat or steam, but not by applying
flame directly or under sheet metal. If the aggregates are in bins, steam or water coils or
other heating methods may be used if aggregate quality is not affected. Live steam heating
is not permitted on or through aggregates in bins. If using dry heat, the Contractor shall
increase mixing time enough to permit the super-dry aggregates to absorb moisture.

The Contractor shall provide and maintain a maturity meter sensor, continuously recording
time and temperature during the curing period, in the concrete at a location specified by the
Engineer for each concrete placement. The Contractor shall also provide recording
thermometers or other approved devices to monitor the surface temperature of the concrete.
During curing, data from the maturity meter and recording thermometer shall be readily
available to the Engineer. The Contractor shall record time and temperature data on hourly
intervals. Data shall be provided to the Engineer upon request.

Starting immediately after placement, the concrete temperatures measured by the maturity
meter and recording thermometer shall be maintained at or above 50° F and the relative
humidity shall be maintained above 80%. These conditions shall be maintained for a
minimum of seven days or for the cure period required by Section 6-02.3(11), whichever is
longer. During this time, if the temperature falls below 50° F on the maturity meter or
recording thermometer, no curing time is awarded for that day. Should the Contractor fail to
adequately protect the concrete and the temperature of the concrete falls below 35° F during
curing, the Engineer may reject it.

The Contractor is solely responsible for protecting concrete from inclement weather during
the entire curing period. Permission given by the Engineer to place concrete during cold
weather will in no way ensure acceptance of the work by the Contracting Agency. Should
the concrete placed under such conditions prove unsatisfactory in any way, the Engineer
shall still have the right to reject the work although the plan and the work were carried out
with the Engineer’s permission.

6-02.3(6)D Protection Against Vibration
The last sentence in the second paragraph is revised to read:

See the Shaft Special Provision, and Section 6-16 respectively for shaft installation, and
soldier pile shaft installation operations.

The first sentence in number 3 under Prescriptive Safe Distance Method is revised to read:

(3) Equipment Class H (High Vibration) shall include pile drivers, machine operated
impact tools, pavement breakers, and other large pieces of equipment.

6-02.3(11) Curing Concrete
In item 1. under the first paragraph, "box culvert tops" is deleted.

The second paragraph is supplemented with the following:
Runoff water shall be collected and disposed of in accordance with all applicable regulations. In no case shall runoff water be allowed to enter any lakes, streams, or other surface waters.

6.023(16) Plans for Falsework and Formwork
The address for FEDEX delivery following the fourth paragraph is revised to read:

Washington State Department of Transportation
Bridge and Structures Engineer
7345 Linderson Way SW
Tumwater, WA 98501-6504

6.023(16)A Nonpreapproved Falsework and Formwork Plans
The address for FEDEX delivery following the first paragraph is revised to read:

Washington State Department of Transportation
Bridge and Structures Engineer
7345 Linderson Way SW
Tumwater, WA 98501-6504

6.023(16)B Preapproved Formwork Plans
The address for FEDEX delivery following the second paragraph is revised to read:

Washington State Department of Transportation
Bridge and Structures Engineer
7345 Linderson Way SW
Tumwater, WA 98501-6504

6.023(17)N Removal of Falsework and Forms
The second through the fifth paragraphs are revised to read:

<table>
<thead>
<tr>
<th>Concrete Placed In</th>
<th>Percent of Specified Minimum Compressive Strength</th>
<th>Number of Days</th>
</tr>
</thead>
<tbody>
<tr>
<td>Columns, walls, non-sloping box girder webs, abutments, footings, traffic and pedestrian barriers, and any other side form not supporting the concrete weight.</td>
<td>—</td>
<td>3</td>
</tr>
<tr>
<td>Crossbeams, pier caps, struts, inclined columns and inclined walls.¹</td>
<td>80</td>
<td>5</td>
</tr>
<tr>
<td>Roadway slabs supported on wood or steel stringers or on steel or prestressed concrete girders(^1,2)</td>
<td>80</td>
<td>10</td>
</tr>
<tr>
<td>Box girders, T-beam girders, and flat-slab superstructure(^1,2)</td>
<td>80</td>
<td>14</td>
</tr>
<tr>
<td>Arches(^1,2)</td>
<td>—</td>
<td>21</td>
</tr>
</tbody>
</table>

\(^1\)Where forms support the concrete weight.

\(^2\)Where continuous spans are involved, the time for all spans will be determined by the last concrete placed affecting any span.

Before releasing supports from beneath beams and girders, the Contractor shall remove forms from columns to enable the Engineer to inspect the column concrete.

The Contractor may remove the side forms of footings 24 hours after concrete placement if a curing compound is applied immediately. This compound shall not be applied to that area of the construction joint between the footing and the column or wall.

The Contractor may remove side forms not supporting the concrete weight 24 hours after concrete placement if the concrete reaches a compressive strength of 1400 psi before form removal. This strength shall be proved by test cylinders made from the last concrete placed into the form. The cylinders shall be cured according to WSDOT FOP for AASHTO T 23.

6-02.3(24)C Placing and Fastening

The 14th paragraph is revised to read:

Clearances shall be at least:

- 4-inches between: Main bars and the top of any concrete masonry exposed to the action of salt or alkaline water.

- 3-inches between: Main bars and the top of any concrete deposited against earth without intervening forms.

- 2\(\frac{1}{2}\)-inches between: Adjacent bars in a layer. Roadway slab bars and the top of the roadway slab.

- 2-inches between: Adjacent layers. Main bars and the surface of concrete exposed to earth or weather (except in roadway slabs). Reinforcing bars and the faces of forms for exposed aggregate finish.

- 1\(\frac{1}{2}\)-inches between: Main bars and the surface of concrete not exposed to earth or weather. Slab bars and the top of the slab (except roadway slabs). Barrier and curb bars and the surface of the
concrete. Stirrups and ties and the surface of the concrete exposed to earth or weather.

1-inch between: Slab bars and the bottom of the slab. Stirrups and ties and the surface of the concrete not exposed to earth or weather.

6-02.3(24)E  Welding Reinforcing Steel

This section is revised to read:

Welding of steel reinforcing bars shall conform to the requirements of ANSI/AWS D1.4 Structural Welding Code - Reinforcing Steel, latest edition, except where superseded by the Special Provisions, Plans, and these Specifications.

Before any welding begins, the Contractor shall obtain the Engineer’s approval of a written welding procedure for each type of welded splice to be used, including the weld procedure specifications and joint details. The weld procedure specifications shall be written on a form taken from AWS D1.4 Annex A, or equivalent. Test results of tensile strength, macroetch, and visual examination shall be included. The form shall be signed and dated.

Welders shall be qualified in accordance with AWS D1.4. The Contractor shall be responsible for the testing and qualification of welders, and shall submit welder qualification and retention records to the Engineer for approval. The weld joint and welding position a welder is qualified in shall be in accordance with AWS D1.4. The welder qualifications shall remain in effect indefinitely unless, (1) the welder is not engaged in a given process of welding for which the welder is qualified for a period exceeding six months, or (2) there is some specific reason to question a welder's ability.

Filler metals used for welding reinforcing bars shall be in accordance with AWS D1.4 Table 5.1. All filler metals shall be low-hydrogen and handled in compliance with low-hydrogen practices specified in the AWS code.

All welding shall be protected from air currents, drafts, and precipitation to prevent loss of heat or loss of arc shielding. Short circuiting transfer with gas metal arc welding will not be allowed. Slugging of welds will not be allowed.

The minimum preheat and interpass temperature for welding shall be in accordance with AWS D1.4 Table 5.2 and mill certification of carbon equivalence, per lot of reinforcing. Preheating shall be applied to the reinforcing bars and other splice members within 6-inches of the weld, unless limited by the available lengths of the bars or splice member.

Generally, post heating of welded splices is only required for direct butt welded splices of AASHTO M 31/ASTM A 615 Grade 60 bars size No. 9 or larger and shall be done immediately after welding before the splice has cooled to 700°F. Post heating shall not be less than 800°F nor more than 1,000°F and held at this temperature for not less than 10 minutes before allowing the splice to cool naturally to ambient temperature.
For the purpose of compatibility with AWS D1.4, welded lap splices for spiral or hoop reinforcing shall be considered Flare-V groove welds, indirect butt joints.

The Contractor is responsible for using a welding sequence that will limit the alignment distortion of the bars due to the effects of welding. The maximum out-of-line permitted will be 1/4-inch from a 3.5-foot straight-edge centered on the weld and in line with the bar.

The following procedure for welding steel reinforcing bars is recommended:

Sheared bar ends shall be burned or sawed off a minimum of 1/2-inch to completely remove the ruptured portion of the steel shear area prior to welding butt splices. Surfaces to be welded shall be smooth, uniform, and free from fins, tears, cracks, and other defects. Surfaces to be welded and surfaces adjacent to a weld shall also be free from loose or thick scale, slag, rust, moisture, grease, paint, epoxy covering, or other foreign materials. All tack welds shall be within the area of the final weld. No other tack weld will be permitted. Double bevel groove welds require chipping, grinding, or gouging to sound metal at the root of the weld before welding the other side. Progression of vertical welding shall be upward. The ground wire from the welding machine shall be clamped to the bar being welded.

Should the Contractor elect to use a procedure which differs in any way from the procedure recommended, the Contractor shall submit the changes, in writing, to the Engineer for approval. Approved weld procedures shall be strictly followed.

6-02.3(26)A Shop Drawings
The address for FEDEX delivery under Item 1 in the first paragraph is revised to read:

Washington State Department of Transportation
Bridge and Structures Engineer
7345 Linderson Way SW
Tumwater, WA 98501-6504

6-02.3(28)A Shop Drawings
The first paragraph is revised to read:

Before casting the structural elements, the Contractor shall submit:

1. Seven sets of shop drawings for approval by the Department of Transportation Bridge and Structures Engineer, Construction Support, addressed as follows:

If sent via US Postal Service:

Washington State Department of Transportation
Bridge and Structures Engineer, Construction Support
P. O. Box 47340
Olympia, WA 98504-7340
If sent via FedEx:

Washington State Department of Transportation
Bridge and Structures Engineer, Construction Support
7345 Linderson Way SW
Tumwater, WA 98501-6504; and

2. Two sets of shop drawings to the Project Engineer.

6-02.4 Measurement
This section is supplemented with the following:

No specific unit of measure will apply to the lump sum item for cure box.

6-02.5 Payment
This section is supplemented with the following:

“Cure Box”, lump sum.
The lump sum contract price for "Cure Box" shall be full pay for all costs for providing, operating, maintaining, moving and removing the cure boxes and providing, maintaining and operating all necessary power sources and connections needed to operate the curing boxes.

SECTION 7-01, DRAINS
August 7, 2006

7-01.3 Construction Requirements
This section is revised to read:

A trench of the dimensions shown in the Plans or as specified by the Engineer shall be excavated to the grade and line given by the Engineer.

Section 7-01.3 is supplemented with the following new sub-sections:

7-01.3(1) Drain Pipe
Drain pipe shall be laid in conformity with the line and grades as shown in the Plans. The drain pipe shall be laid with soiltight joints unless otherwise specified. Concrete drain pipe shall be laid with the bell or larger end upstream. PVC drain pipe shall be jointed with a bell and spigot joint using a flexible elastomeric seal as described in Section 9-04.8. The bell shall be laid upstream. PE drain pipe shall be jointed with snap-on, screw-on, bell and spigot, or wraparound coupling bands as recommended by the manufacturer of the tubing.
7-01.3(2) Underdrain Pipe

When underdrain pipe is being installed as a means of intercepting ground or surface water, the trench shall be fine-graded in the existing soil 3 inches below the grade of the pipe as shown in the Plans. Gravel backfill shall be used under the pipe. Gravel backfill shall be placed to the depth shown in the Plans or as designated by the Engineer. All backfill shall be placed in 12-inch maximum layers and be thoroughly compacted with three passes of a vibratory compactor for each layer. The Contractor shall use care in placing the gravel backfill material to prevent its contamination.

Class 2 perforations shall be used unless otherwise specified. When Class 1 perforations are specified the perforated pipe shall be laid with the perforations down. Upon final acceptance of the work, all drain pipes shall be open, clean, and free draining. Perforated pipe does not require a watertight joint. PVC underdrain pipe shall be jointed using either the flexible elastomeric seal as described in Section 9-04.8 or solvent cement as described in Section 9-04.9, at the option of the Contractor unless otherwise specified in the Plans. The bell shall be laid upstream. PE drainage tubing underdrain pipe shall be jointed with snap-on, screw-on, bell and spigot, or wraparound coupling bands, as recommended by the manufacturer of the tubing.

SECTION 7-02, CULVERTS
January 3, 2006

7-02.2 Materials
The fifth and seventh paragraphs are deleted:

SECTION 7-04, STORM SEWERS
January 3, 2006

7-04.2 Materials
The fourth and sixth paragraphs are deleted:

SECTION 8-01, EROSION CONTROL AND WATER POLLUTION CONTROL
December 4, 2006

8-01.3(1) General
The eighth paragraph, beginning with “In western Washington, erodible soil”, is deleted and replaced with the following:

Erodible soil not being worked, whether at final grade or not, shall be covered within the following time period, using an approved soil covering practice, unless authorized otherwise by the Engineer:

In western Washington (west of the Cascade Mountain crest):
October 1 through April 30  
2 days maximum

May 1 to September 30  
7 days maximum

In eastern Washington (east of the Cascade Mountain crest):

October 1 through June 30  
5 days maximum

July 1 through September 30  
10 days maximum

8-01.3(1)B Erosion and Sediment Control (ESC) Lead

This section is revised to read:

The Contractor shall identify the ESC Lead at the preconstruction discussions and in the TESC plan. The ESC Lead shall have, for the life of the contract, a current Certificate of Training in Construction Site Erosion and Sediment Control from a course approved by the Washington State Department of Ecology. The ESC Lead shall be listed on the Emergency Contact List required under Section 1-05.13(1).

The ESC Lead shall implement the Temporary Erosion and Sediment Control (TESC) plan. Implementation shall include, but is not limited to:

1. Installing and maintaining all temporary erosion and sediment control Best Management Practices (BMPs) included in the TESC plan to assure continued performance of their intended function. Damaged or inadequate TESC BMPs shall be corrected immediately.

2. Updating the TESC plan to reflect current field conditions.

When a TESC plan is included in the contract plans, the Contractor shall inspect all on-site erosion and sediment control BMPs at least once every calendar week and within 24 hours of runoff events in which stormwater discharges from the site. Inspections of temporarily stabilized, inactive sites may be reduced to once every calendar month. The Erosion and Sediment Control Inspection Form (Form Number 220-030 EF) shall be completed for each inspection and a copy shall be submitted to the Engineer no later than the end of the next working day following the inspection.

8-01.3(2)E Tackling Agent and Soil Binders

The third paragraph, (PAM) is revised to read:

Soil Binding Using Polyacrylamide (PAM)

The PAM shall be applied on bare soil completely dissolved and mixed in water or applied as a dry powder. Dissolved PAM shall be applied at a rate of not more than 2/3 pound per 1,000 gallons of water per acre. A minimum of 200 pounds per acre of cellulose fiber mulch treated with a non-toxic dye shall be applied with the dissolved PAM. Dry powder applications may be at a rate of 5 pounds per acre using a hand-held fertilizer spreader or a tractor-mounted spreader.
8-01.3(2)F Dates for Application of Final Seed, Fertilizer, and Mulch
The second paragraph under East of the summit of the Cascade Range, beginning with “The Contractor will be responsible”, is deleted.

8-01.3(9)A Silt Fence
The fifth paragraph is revised to read:

Posts shall be either wood or steel. Wood posts shall have minimum dimensions of 1 1/4 inches by 1 1/4 inches by the minimum length shown in the Plans. Steel posts shall have a minimum weight of 0.90 lbs/ft

8-01.4 Measurement
This section is supplemented with the following:

Coir log will be measured by the linear foot along the ground line of the completed installation.

8-01.5 Payment
The following bid item is inserted after “Compost Sock”, per linear foot:

“Coir Log”, per linear foot

This section is supplemented with the following:

"Mowing", per acre.

SECTION 8-04, CURBS, GUTTERS, AND SPILLWAYS
December 4, 2006

8-04.3(2) Extruded Asphalt Concrete Curbs, and Gutters
The first paragraph is supplemented with the following:

Just prior to placing the curb, a tack coat of asphalt shall be applied to the existing pavement surface at the rate ordered by the Engineer.

8-04.4 Measurement
The first paragraph is revised to read:

All curbs, gutters, and spillways will be measured by the linear foot along the line and slope of the completed curbs, gutters, or spillways, including bends. Measurement of cement concrete curb and cement concrete curb and gutter, when constructed across driveways or sidewalk ramps, will include the width of the driveway or sidewalk ramp.
SECTION 9-00, DEFINITIONS AND TESTS
January 3, 2006

9-00.8 Sand Equivalent
The second paragraph is revised to read:

For acceptance, there must be a clear line of demarcation. If no clear line of demarcation has formed at the end of a 30 minute sedimentation period, the material will be considered as failing to meet the minimum specified sand equivalent.

SECTION 9-01, PORTLAND CEMENT
April 2, 2007

9-01.2(1) Portland Cement
The second sentence in the first paragraph is revised to read:

The total amount of processing additions used shall not exceed 1% of the weight of portland cement clinker and up to 3.0% cement kiln dust by mass of the cement as long as it complies with the requirements of ASTM C-465.

9-01.2(4) Blended Hydraulic Cement
The first paragraph is revised to read:

Blended hydraulic cement shall be either Type IP (MS), Type I (SM) (MS) or Type I (PM) (MS) cement conforming to AASHTO M 240, except that the content of alkalis shall not exceed 0.75 percent by weight calculated as Na₂O plus 0.658 K₂O and except that the content of Tricalcium aluminate (C₃A) shall not exceed 8 percent by weight calculated as 2.650Al₂O₃ minus 1.692Fe₂O₃, and meet the following additional requirements:

SECTION 9-02, BITUMINOUS MATERIALS
January 3, 2006

9-02.1(4) Asphalt Binders
This section including title is revised to read:

9-02.1(4) Performance Graded Asphalt Binder (PGAB)
PGAB meeting the requirements of AASHTO M 320 Table 1 of the grades specified in the contract shall be used in the production of HMA. The Direct Tension Test (AASHTO T 314) of M 320 is not a specification requirement.

9-02.1(4)A Performance Graded Asphalt Binder
This section including title is revised to read:

9-02.1(4)A Quality Control Plan
The Asphalt Supplier of PGAB shall have a Quality Control Plan (QCP) in accordance with WSDOT QC 2 “Standard Practice for Asphalt Suppliers That Certify Performance Graded Asphalts”. The Asphalt Supplier’s QCP shall be submitted and approved by the WSDOT State Materials Laboratory. Any change to the QCP will require a new QCP to be submitted. The Asphalt Supplier of PGAB shall certify through the Bill of Lading that PGAB meets the specification requirements of the contract.

9-02.1(6)A Polymerized Cationic Emulsified Asphalt CRS-2P

This section is revised to read:

The asphalt CRS-2P shall be a polymerized cationic emulsified asphalt. The polymer shall be milled into the asphalt or emulsion during the manufacturing of the emulsion. The asphalt CRS-2P shall meet the following specifications:

<table>
<thead>
<tr>
<th>Test Method</th>
<th>AASHTO Test Method</th>
<th>Specifications</th>
</tr>
</thead>
<tbody>
<tr>
<td>Viscosity @122°F, SFS</td>
<td>T 59</td>
<td>100-400</td>
</tr>
<tr>
<td>Storage Stability 1 day %</td>
<td>T 59</td>
<td>---</td>
</tr>
<tr>
<td>Demulsibility 35 ml. 0.8% Dioctyl Sodium Sulfosuccinate</td>
<td>T 59</td>
<td>40</td>
</tr>
<tr>
<td>Particle Charge</td>
<td>T 59</td>
<td>positive</td>
</tr>
<tr>
<td>Sieve Test %</td>
<td>T 59</td>
<td>---</td>
</tr>
<tr>
<td>Distillation</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Oil distillate by vol. of emulsion %</td>
<td>T 59&lt;sup&gt;Note 1&lt;/sup&gt;</td>
<td>0</td>
</tr>
<tr>
<td>Residue</td>
<td>T 59&lt;sup&gt;Note 1&lt;/sup&gt;</td>
<td>65</td>
</tr>
<tr>
<td>Test on the Residue From Distillation</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Penetration @77°F</td>
<td>T 49</td>
<td>100</td>
</tr>
<tr>
<td>Torsional Recovery %</td>
<td>note 2</td>
<td>18</td>
</tr>
<tr>
<td>or</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Toughness/Tenacity in-lbs</td>
<td>note 3</td>
<td>50/25</td>
</tr>
</tbody>
</table>

<sup>Note 1</sup> Distillation modified to use 300 grams of emulsion heated to 350°F ± 9°F and maintained for 20 minutes.

<sup>Note 2</sup> The Torsional Recovery test shall be conducted according to the California Department of Transportation Test Method No. 332. The residue material for this test shall come from California Department of Transportation Test Method No. 331.
Benson method of toughness and tenacity; Scott tester, inch-pounds at 77°F, 20 in.
per minute pull. Tension head 7/8 in. diameter.

At the option of the supplier the Benson Toughness/Tenacity test can be used in lieu of
Torsional Recovery based on type of modifier used. If the Benson Toughness/Tenacity
method is used for acceptance the supplier must supply all test data verifying specification
conformance.

SECTION 9-03, AGGREGATES
April 2, 2007

9-03.1(4)A Deleterious Substances
The reference to “AASHTO PT 61” in the second paragraph is revised to “AASHTO TP 61”.

9-03.4(2) Grading and Quality
The reference to “AASHTO PT 61” in the fourth paragraph is revised to “AASHTO TP 61”.

9-03.8(2) HMA Test Requirements
In the first paragraph, item 2. and the associated graph are revised to read:

2. The fracture requirements for the combined coarse aggregate shall apply to the material
retained on the U.S. No. 4 sieve and above, when tested in accordance with FOP for
AASHTO TP 61.

<table>
<thead>
<tr>
<th>ESAL's (millions)</th>
<th># Fractured Faces</th>
<th>% Fracture</th>
</tr>
</thead>
<tbody>
<tr>
<td>&lt; 10</td>
<td>1 or more</td>
<td>90</td>
</tr>
<tr>
<td>10</td>
<td>2 or more</td>
<td>90</td>
</tr>
</tbody>
</table>

9-03.9(3) Crushed Surfacing
The reference to “AASHTO PT 61” in the fourth paragraph is revised to “AASHTO TP 61”.

9-03.20 Test Methods for Aggregates
The test method for Percent of Fracture in Aggregates is revised from “AASHTO PT 61” to
“AASHTO TP 61”.

SECTION 9-05, DRAINAGE STRUCTURES, CULVERTS, AND CONDUITS
April 2, 2007
9-05.1(1) Concrete Drain Pipe
This section is revised to read:

Concrete drain pipe shall meet the requirements of ASTM C 118, heavy duty drainage pipe.
9-05.1(6) Corrugated Polyethylene Drainage Tubing Drain Pipe
This section including title is revised to read:

9-05.1(6) Corrugated Polyethylene Drain Pipe (up to 10-inch)
Corrugated polyethylene drain pipe shall meet the requirements of AASHTO M 252 type C
(corrugated both inside and outside) or type S (corrugated outer wall and smooth inner
liner). The maximum size pipe shall be 10 inches in diameter.

9-05.2(3) Perforated Bituminized Fiber Underdrain Pipe
This section including title is revised to read:

9-05.2(3) Vacant

9-05.1(7) Corrugated Polyethylene Drain Pipe
This section including title is revised to read:

9-05.1(7) Corrugated Polyethylene Drain Pipe (12-inch through 60-inch)
Corrugated polyethylene drain pipe, 12-inch through 60-inch -diameter maximum, shall
meet the minimum requirements of AASHTO M 294 Type S or 12-inch through 24 inch
diameter maximum shall meet the minimum requirements of AASHTO M 294 Type C.

9-05.2(7) Perforated Corrugated Polyethylene Drainage Tubing Underdrain Pipe
This section including title is revised to read:

9-05.2(7) Perforated Corrugated Polyethylene Underdrain Pipe (Up to 10-inch)
Perforated corrugated polyethylene underdrain pipe shall meet the requirements of
AASHTO M252, Type CP or Type SP. Type CP shall be Type C pipe with Class 2
perforations and Type SP shall be Type S pipe with either Class 1 or Class 2 perforations.
Additionally, Class 2 perforations shall be uniformly spaced along the length and
circumference of the pipe. The maximum size pipe shall be 10-inch diameter.

9-05.2(8) Perforated Corrugated Polyethylene Underdrain Pipe
This section including title is revised to read:

9-05.2(8) Perforated Corrugated Polyethylene Underdrain Pipe (12-inch through 60-
inch)
Perforated corrugated polyethylene underdrain pipe, 12-inch through 60-inch diameter
maximum, shall meet the requirements of AASHTO M 294 Type CP or Type SP. Type CP
shall be Type C pipe with Class 2 perforations and Type SP shall be Type S pipe with either
Class 1 or Class 2 perforations. Additionally, Class 2 perforations shall be uniformly spaced
along the length and circumference of the pipe.

9-05.3(1)A End Design and Joints
The second paragraph is revised to read:

The plane of the ends of the pipes shall be perpendicular to their longitudinal axes.
9-05.4(3) Protective Treatment
In Treatment 1 and 2, the reference to 9-05.4(6) is revised to read 9-05.4(5).

9-05.12(1) Solid Wall PVC Culvert Pipe, Solid Wall PVC Storm Sewer Pipe, and Solid Wall PVC Sanitary Sewer Pipe
The first paragraph is revised to read:

Solid wall PVC culvert pipe, solid wall PVC storm sewer pipe, and solid wall PVC sanitary sewer pipe and fittings shall be solid wall construction and shall conform to the following requirements:

For pipe sizes up to 15 inches: ASTM D 3034 SDR 35

For pipe sizes from 18 to 48 inches: ASTM F 679 using a minimum pipe stiffness of 115 psi in accordance with Table 1.

9-05.12(2) Profile Wall PVC Culvert Pipe, Profile Wall PVC Storm Sewer Pipe, and Profile Wall PVC Sanitary Sewer Pipe
The first paragraph is revised to read:

Profile wall PVC culvert pipe and profile wall PVC storm sewer pipe shall meet the requirements of ASTM F 794 Series 46, or ASTM F 1803. Profile wall PVC sanitary sewer pipe shall meet the requirements of ASTM F 794 Series 46, or ASTM F 1803. The maximum pipe diameter shall be as specified in the Qualified Products List.

The fifth paragraph is revised to read:

Fittings for profile wall PVC pipe shall meet the requirements of ASTM F 794 Series 46, or ASTM F 1803.

9-05.15 Metal Castings
This section is revised to read:

For all metal castings the producing foundry shall provide certification stating the country of origin, the material meets the required ASTM or AASHTO specification noted in the subsections below. The producing foundry shall detail all test results from physical testing to determine compliance to the specifications. The test reports shall include physical properties of the material from each heat and shall include tensile, yield, and elongation as specified in the appropriate ASTM or AASHTO specification. For AASHTO M 306, Section 8, Certification is deleted and replaced with the above certification and testing requirements.

Metal castings for drainage structures shall not be dipped, painted, welded, plugged, or repaired. Porosity in metal castings for drainage structures shall be considered a workmanship defect subject to rejection by the Engineer. Metal castings made from gray iron or ductile iron shall conform to the requirements of AASHTO M 306, and metal
castings made from cast steel shall conform to the requirements of Section 9-06.8. All metal castings shall meet the proof load testing requirements of AASHTO M 306.

9-05.15(1) Manhole Ring and Cover
This section is revised to read:

Castings for manhole rings shall be gray iron or ductile iron and covers shall be ductile iron.

All covers shall be interchangeable within the dimensions shown in the Standard Plans. All mating surfaces shall be machine finished to ensure a nonrocking fit.

The inside vertical recessed face of the ring and the vertical outside edge of the cover shall be machined or manufactured to the following tolerances:

| Ring       | +3/32 inch to -3/32 inch |
| Cover      | +3/32 inch to -3/32 inch |

All manhole rings and covers shall be identified by the name or symbol of the producing foundry and country of casting origin. This identification shall be in a plainly visible location when the ring and cover are installed. Ductile iron shall be identified by the following, “DUC” or “DI.” The producing foundry and material identification shall be adjacent to each other and shall be minimum ½ inch to maximum 1inch high letters, recessed to be flush with the adjacent surfaces.

9-05.15(2) Metal Frame, Grate and Solid Metal Cover for Catch Basins or Inlets
The first and second paragraphs are revised to read:

Castings for metal frames for catch basins and inlets shall be cast steel, gray iron, or ductile iron, and as shown in the Standard Plans.

Castings for grates and solid metal covers for catch basins and inlets shall be cast steel or ductile iron and as shown in the Standard Plans. Additionally, leveling pads are allowed on grates and solid metal covers with a height not to exceed 1/8 inch. The producing foundry’s name and material designation shall be embossed on the top of the grate. The material shall be identified by the following: “CS” for cast steel or “DUC” or “DI” for ductile iron and shall be located near the producing foundry’s name.

9-05.15(3) Cast Metal Inlets
The first sentence is revised to read:

The castings for cast metal inlets shall be cast steel or ductile iron, and as shown in the Standard Plans.

9-05.19 Corrugated Polyethylene Culvert Pipe
The first paragraph is revised to read:
Corrugated polyethylene culvert pipe shall meet the requirements of AASHTO M 294 Type S or D for pipe 12-inch to 60-inch diameter with silt-tight joints.

SECTION 9-06, STRUCTURAL STEEL AND RELATED MATERIALS
December 4, 2006

9-06.5(4) Anchor Bolts
The first and second paragraphs are revised to read:

Anchor bolts shall meet the requirements of ASTM F 1554 and, unless otherwise specified, shall be Grade 105 and shall conform to Supplemental Requirements S2, S3, and S4.

Nuts for ASTM F 1554 Grade 105 black anchor bolts shall conform to AASHTO M 291, Grade D or DH. Nuts for ASTM F 1554 Grade 105 galvanized bolts shall conform to AASHTO M 291, Grade DH and shall conform to the lubrication requirements in Section 9-06.5(3). Nuts for ASTM F 1554 Grade 36 or 55 black or galvanized anchor bolts shall conform to AASHTO M 291, Grade A. Washers shall conform to ASTM F 436.

9-06.9 Gray Iron Castings
The AASHTO requirement is revised to read “AASHTO M 306”.

SECTION 9-07, REINFORCING STEEL
December 4, 2006

9-07.2 Deformed Steel Bars
The first sentence in the first paragraph is revised to read:

Deformed steel bars for concrete reinforcement shall conform to either AASHTO M 31 Grade 60, or ASTM A 706, except as otherwise noted. Steel reinforcing bar for the cast-in-place components of bridge structures (excluding sidewalks and barriers but including shafts and concrete piles), and for precast substructure components of bridge structures, shall conform to ASTM A 706 only.

SECTION 9-13, RPRAP, QUARRY SPALLS, SLOPE PROTECTION, AND ROCK WALLS
April 2, 2007

9-13 Riprap, Quarry Spalls, Slope Protection, And Rock Walls
The requirements for Quality following the first paragraph are revised to read:

<table>
<thead>
<tr>
<th>Aggregate Property</th>
<th>Test method</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Degradation Factor</td>
<td>WSDOT T 113</td>
<td>15 minimum</td>
</tr>
<tr>
<td>Los Angeles Wear, 500 Rev.</td>
<td>AASHTO T 96</td>
<td>50% maximum</td>
</tr>
</tbody>
</table>
Specific Gravity AASHTO T 85 2.55 minimum

9-13.5(2) Poured Portland Cement Concrete Slope Protection
The first paragraph is revised to read:

Cement concrete for poured concrete slope protection shall be commercial concrete in
conformance with Section 6-02.3(2)B.

SECTION 9-14, EROSION CONTROL AND ROADSIDE PLANTING
April 2, 2007
9-14.2 Seed
This section is revised to read:

Grasses, legumes, or cover crop seed of the type specified shall conform to the standards for
“Certified” grade seed or better as outlined by the State of Washington Department of
Agriculture “Rules for Seed Certification,” latest edition. Seed shall be furnished in standard
containers on which shall be shown the following information:

1. Common and botanical names of seed,
2. Lot number,
3. Net weight,
4. Pure live seed

All seed installers and vendors must have a business license issued by the Washington State
Department of Licensing with a “seed dealer” endorsement. Upon request, the contractor
shall furnish the Engineer with copies of the applicable licenses and endorsements.

Upon request, the Contractor shall furnish to the Engineer duplicate copies of a statement
signed by the vendor certifying that each lot of seed has been tested by a recognized seed
testing laboratory within six months before the date of delivery on the project. Seed which
has become wet, moldy, or otherwise damaged in transit or storage will not be accepted.

9-14.4(1) Straw
This section is revised to read:

All straw material shall be in an air dried condition free of noxious weeds and other
materials detrimental to plant life. Straw mulch so provided shall be suitable for spreading
with mulch blower equipment.

9-14.4(3) Bark or Wood Chips
This section is supplemented with the following:

Sawdust shall not be used as mulch.
9.14.4(4) Sawdust

This section including title is revised to read:

9.14.4(4) Vacant

9.14.4(8) Compost

This section is revised to read:

Compost products shall be the result of the biological degradation and transformation of plant-derived materials under controlled conditions designed to promote aerobic decomposition. Compost shall be stable with regard to oxygen consumption and carbon dioxide generation. Compost shall be mature with regard to its suitability for serving as a soil amendment or an erosion control BMP as defined below. The compost shall have a moisture content that has no visible free water or dust produced when handling the material.

Compost production and quality shall comply with Chapter 173-350 WAC.

Compost products shall meet the following physical criteria:

1. Compost material shall be tested in accordance with Testing Methods for the Examination of Compost and Composting (TMECC) Test Method 02.02-B, “Sample Sieving for Aggregate Size Classification”.

Fine Compost shall meet the following:

<table>
<thead>
<tr>
<th>Percent passing 2”</th>
<th>Min.</th>
<th>Max.</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>100%</td>
<td></td>
</tr>
<tr>
<td>Percent passing 1”</td>
<td>99%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 5/8”</td>
<td>90%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 1/4”</td>
<td>75%</td>
<td>100%</td>
</tr>
</tbody>
</table>

Maximum particle length of 6 inches

Coarse Compost shall meet the following:

<table>
<thead>
<tr>
<th>Percent passing 3”</th>
<th>Min.</th>
<th>Max.</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>100%</td>
<td></td>
</tr>
<tr>
<td>Percent passing 1”</td>
<td>90%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 3/4”</td>
<td>70%</td>
<td>100%</td>
</tr>
<tr>
<td>Percent passing 1/4”</td>
<td>40%</td>
<td>60%</td>
</tr>
</tbody>
</table>

Maximum particle length of 6 inches

2. The pH shall be between 6.0 and 8.5 when tested in accordance with TMECC 04.11-A, “1:5 Slurry pH”.

3. Manufactured inert material (plastic, concrete, ceramics, metal, etc.) shall be less than 1.0 percent by weight as determined by TMECC 03.08-A "percent dry weight basis":
4. Minimum organic matter shall be 40 percent dry weight basis as determined by TMECC 05.07A, "Loss-On-Ignition Organic Matter Method".

5. Soluble salt contents shall be less than 4.0mmhos/cm tested in accordance with TMECC 04.10-A, "1:5 Slurry Method, Mass Basis".

6. Maturity shall be greater than 80% in accordance with TMECC 05.05-A, "Germination and Vigor".

7. Stability shall be 7 or below in accordance with TMECC 05.08-B, Carbon Dioxide Evolution Rate.”

8. The compost product must originate a minimum of 65 percent by volume from recycled plant waste as defined in WAC 173-350 as “Type 1 Feedstocks.” A maximum of 35 percent by volume of other approved organic waste and/or biosolids may be substituted for recycled plant waste. The supplier shall provide written verification of feedstock sources.

9. The Engineer may also evaluate compost for maturity using the Solvita Compost Maturity Test. Fine Compost shall score a number 6 or above on the Solvita Compost Maturity Test. Coarse Compost shall score a 5 or above on the Solvita Compost Maturity Test.

The compost supplier will test all compost products within 90 calendar days prior to application. Samples will be taken using the Seal of Testing Assurance (STA) sample collection protocol. (The sample collection protocol can be obtained from the U.S. Composting Council, 4250 Veterans Memorial Highway, Suite 275, Holbrook, NY 11741. Phone: 631-737-4931, www.compostingcouncil.org). The sample shall be sent to an independent STA Program approved lab. The compost supplier will pay for the test. A copy of the approved independent STA Program laboratory test report shall be submitted to the Contracting Agency prior to initial application of the compost. Seven days prior to application, the Contractor shall submit a sample of each type compost to be used on the project to the Engineer.

Compost not conforming to the above requirements or taken from a source other than those tested and accepted shall be immediately removed from the project and replaced at no cost to the Contracting Agency.

The Contractor shall either select a compost supplier from the Qualified Products List, or submit the following information to the Engineer for approval:

1. A Request for Approval of Material Source.
2. A copy of the Solid Waste Handling Permit issued to the supplier by the Jurisdictional Health Department as per WAC 173-350 (Minimum Functional Standards for Solid Waste Handling).

3. The supplier shall verify in writing, and provide lab analyses that the material complies with the processes, testing, and standards specified in WAC 173-350 and these specifications. An independent STA Program certified laboratory shall perform the analysis.

4. A list of the feedstock by percentage present in the final compost product.

5. A copy of the producer’s Seal of Testing Assurance certification as issued by the U.S. Composting Council.

Acceptance will be based upon a satisfactory Test Report from an independent STA program certified laboratory and the sample(s) submitted to the Engineer.

9-14.5(2) Erosion Control Blanket
Footnote 1 is revised to read:

1¹UV stability shall be 80% strength retained min., after 500 hours in a xenon arc device as per ASTM D4355.

9-14.5(5) Wattles
This section is revised to read:

Wattles shall consist of cylinders of biodegradable plant material such as straw, coir, compost, or wood shavings encased within biodegradable or photodegradable netting. Wattles shall be at least 5 inches in diameter, unless otherwise specified. Encasing material shall be clean, evenly woven, and free of encrusted concrete or other contaminating materials such as preservatives. Encasing material shall be free from cuts, tears, or weak places and shall have a lifespan greater than 6 months.

Compost filler shall meet the material requirements as specified in Section 9-14.4(8), and shall be Coarse Compost.

9-14.5(6) Compost Sock
This section is revised to read:

Biodegradable fabric for compost sock and compost wattle shall be clean, evenly woven, and free of encrusted concrete or other contaminating materials and shall be free from cuts, tears, broken or missing yarns and thin, open, or weak places. Fabric for compost sock shall consist of extra heavy weight biodegradable fiber which has not been treated with any type of preservative. Compost for compost socks shall meet the material requirements as specified in Section 9-14.4(8), and shall be Coarse Compost.
Wood stakes for compost sock and wattles shall be made from Douglas-fir, hemlock, or pine species. Wood stakes shall be 2 inch by 2 inch nominal dimension and 36 inches in length, unless otherwise indicated in the Plans.

Section 9-14.5 is supplemented with the following new section.

9-14.5(7) Coir Log
Coir log: Logs shall be made of 100% durable coconut (coir) fiber uniformly compacted within an outer netting. Log segments shall have a maximum length of 20 feet, with a minimum diameter as shown in the Plans. Logs shall have a density of 7 lbs/cf or greater.

Coir logs shall be manufactured with a woven wrapping netting made of bristle coir twine with minimum strength of 80 lbs tensile strength. The netting shall have nominal 2 inch by 2 inch openings.

Stakes shall conform to the requirements of Section 9-09. Cedar wood stakes shall have a notch to secure the rope ties. Rope ties shall be one-quarter inch diameter commercially available hemp rope.

9-14.6(1) Description
This section is revised to read:

Bareroot plants are grown in the ground and harvested without soil or growing medium around their roots.

Container plants are grown in pots or flats that prevent root growth beyond the sides and bottom of the container.

Balled and burlapped plants are grown in the ground and harvested with soil around a core of undisturbed roots. This rootball is wrapped in burlap and tied or placed in a wire basket or other supportive structure.

Cuttings are live plant material without a previously developed root system. Source plants for cuttings shall be dormant when cuttings are taken. All cuts shall be made with a sharp instrument. Written permission shall be obtained from property owners and provided to the Engineer before cuttings are collected. The Contractor shall collect cuttings in accordance with applicable sensitive area ordinances. For cuttings, the requirement to be nursery grown or held in nursery conditions does not apply. Cuttings include the following forms:

A. Live branch cuttings shall have flexible top growth with terminal buds and may have side branches. The rooting end shall be cut at an approximate 45 degree angle.

B. Live stake cuttings shall have a straight top cut immediately above a bud. The lower, rooting end shall be cut at an approximate 45 degree angle. Live stakes are cut from one to two year old wood. Live stake cuttings shall be cut and installed.
with the bark intact with no branches or stems attached, and be ½ to 1 ½ inch in
diameter.

C. Live pole cuttings shall have a minimum 2inch diameter and no more than three
branches which shall be pruned back to the first bud from the main stem.

D. Rhizomes shall be a prostrate or subterranean stem, usually rooting at the nodes
and becoming erect at the apex. Rhizomes shall have a minimum of two growth
points.

E. Tubers shall be a thickened and short subterranean branch having numerous buds
or eyes.

9-14.6(2) Quality
This section is revised to read:

All plant material furnished shall meet the grades established by the latest edition of the
American Standard for Nursery Stock, (ASNS) ANSI Z60.1 shall conform to the size and
acceptable conditions as listed in the contract, and shall be free of all foreign plant material.

All plant material shall comply with State and Federal laws with respect to inspection for
plant diseases and insect infestation.

All plant material shall be purchased from a nursery licensed to sell plants in Washington
State.

Live woody or herbaceous plant material, except cuttings, rhizomes, and tubers, shall be
vigorously, well formed, with well developed fibrous root systems, free from dead branches,
and from damage caused by an absence or an excess of heat or moisture, insects, disease,
mechanical or other causes detrimental to good plant development. Evergreen plants shall be
well foliated and of good color. Deciduous trees that have solitary leaders shall have only
the lateral branches thinned by pruning. All conifer trees shall have only one leader
(growing apex) and one terminal bud, and shall not be sheared or shaped. Trees having a
damaged or missing leader, multiple leaders, or Y-crotches shall be rejected.

Root balls of plant materials shall be solidly held together by a fibrous root system and shall
be composed only of the soil in which the plant has been actually growing. Balled and
burlapped rootballs shall be securely wrapped with jute burlap or other packing material not
injurious to the plant life. Root balls shall be free of weed or foreign plant growth.

Plant materials shall be nursery grown stock. Plant material, with the exception of cuttings,
gathered from native stands shall be held under nursery conditions for a minimum of one
full growing season, shall be free of all foreign plant material, and meet all of the
requirements of these Specifications, the Plans, and the Special Provisions.
Container grown plants must be plants transplanted into a container and grown in that
container sufficiently long for new fibrous roots to have developed so that the root mass will
retain its shape and hold together when removed from the container, without having roots
that circle the pot. Plant material which is root bound, as determined by the Engineer, shall
be rejected. Container plants shall be free of weed or foreign plant growth.

Container sizes for plant material of a larger grade than provided for in the container grown
specifications of the ASNS shall be determined by the volume of the root ball specified in
the ASNS for the same size plant material.

All bare root plant materials shall have a heavy fibrous root system and must be dormant at
the time of planting.

Average height to spread proportions and branching shall be in accordance with the
applicable sections, illustrations, and accompanying notes of the ASNS.

Plants specified or identified as “Street Tree Grade” shall be trees with straight trunks, full
and symmetrical branching, central leader, and be developed, grown, and propagated with a
full branching crown. A “Street Tree Grade” designation requires the highest grade of
nursery shade or ornamental tree production which shall be supplied.

Trees with improperly pruned, broken, or damaged branches, trunk, or root structure shall be
rejected. In all cases, whether supplied balled and burlapped or in a container, the root
crown (top of root structure) of the tree shall be at the top of the finish soil level. Trees
supplied and delivered in a nursery fabric bag will not be accepted.

Plants, which have been determined by the Engineer to have suffered damage as the result of
girdling of the roots, stem, or a major branch; have deformities of the stem or major
branches; have a lack of symmetry; have dead or defoliated tops or branches; or have any
defect, injury, or condition which renders the plant unsuitable for its intended use, shall be
rejected.

Plants that are grafted shall have roots of the same genus as the specified plant.

9-14.6(3) Handling and Shipping
The last sentence in the sixth paragraph is deleted.

9-14.6(6) Substitution of Plants
The second paragraph is revised to read:

Container or balled and burlapped plant material may be substituted for bare root plant
material. Container grown plant material may be substituted for balled and burlapped plant
materials. When substitution is allowed, use current ASNS standards to determine the
correct rootball volume (container or balled and burlapped) of the substituted material that
corresponds to that of the specified material. These substitutions shall be approved by the
Engineer and be at no cost to the Contracting Agency.
9-14.6(7) Temporary Storage

The third paragraph is revised to read:

Cuttings shall continually be shaded and protected from wind. Cuttings must be protected from drying at all times and shall be heeled into moist soil or other insulating material or placed in water if not installed within 8 hours of cutting. Cuttings to be stored for later installation shall be bundled, laid horizontally, and completely buried under 6 inches of water, moist soil or placed in cold storage at a temperature of 34°F and 90% humidity. Cuttings that are not planted within 24 hours of cutting shall be soaked in water for 24 hours prior to planting. Cuttings taken when the temperature is higher than 50°F shall not be stored for later use. Cuttings that already have developed roots shall not be used.

The fourth paragraph is deleted.

SECTION 9-23, CONCRETE CURING MATERIALS AND ADMIXTURES

April 2, 2007

9-23.6 Admixture for Concrete

The footnote for Accelerating Admixture is revised to read:

* Accelerating admixtures are only allowed for use in the following applications: In Controlled Density Fill (also known as Controlled Low Strength Material) in accordance with Section 2-09.3(1)E Backfilling, in Portland Cement Concrete Pavement in accordance with Section 5-05, and in Section 5-05.3(1) Concrete Mix Designs for Paving.

SECTION 9-33, CONSTRUCTION GEOTEXTILE

August 7, 2006

Section 9-33 including title is revised in its entirety to read:

SECTION 9-33, CONSTRUCTION GEOSYNTHETIC

9-33.1 Geosynthetic Material Requirements

The term geosynthetic shall be considered to be inclusive of geotextiles, geogrids, and prefabricated drainage mats.

Geotextiles, including geotextiles attached to prefabricated drainage core to form a prefabricated drainage mat, shall consist only of long chain polymeric fibers or yarns formed into a stable network such that the fibers or yarns retain their position relative to each other during handling, placement, and design service life. At least 95 percent by weight of the material shall be polyolefins or polyesters. The material shall be free from defects or tears. The geotextile shall also be free of any treatment or coating which might adversely alter its hydraulic or physical properties after installation.
Geogrids shall consist of a regular network of integrally connected polymer tensile elements with an aperture geometry sufficient to permit mechanical interlock with the surrounding backfill. The long chain polymers in the geogrid tensile elements, not including coatings, shall consist of at least 95 percent by mass of the material of polyolefins or polyesters. The material shall be free of defects, cuts, and tears.

Prefabricated drainage core shall consist of a three dimensional polymeric material with a structure that permits flow along the core laterally, and which provides support to the geotextiles attached to it.

The geosynthetic shall conform to the properties as indicated in Tables 1 through 8 in Section 9.33.2, and additional tables as required in the Standard Plans and Special Provisions for each use specified in the Plans. Specifically, the geosynthetic uses included in this section and their associated tables of properties are as follows:

<table>
<thead>
<tr>
<th>Geotextile Geosynthetic Application</th>
<th>Applicable Property Tables</th>
</tr>
</thead>
<tbody>
<tr>
<td>Underground Drainage, Low and Moderate Survivability, Classes A, B, and C</td>
<td>Tables 1 and 2</td>
</tr>
<tr>
<td>Separation</td>
<td>Table 3</td>
</tr>
<tr>
<td>Soil Stabilization</td>
<td>Table 3</td>
</tr>
<tr>
<td>Permanent Erosion Control, Moderate and High Survivability, Classes A, B, and C</td>
<td>Tables 4 and 5</td>
</tr>
<tr>
<td>Ditch Lining</td>
<td>Table 4</td>
</tr>
<tr>
<td>Temporary Silt Fence</td>
<td>Table 6</td>
</tr>
<tr>
<td>Permanent Geosynthetic Retaining Wall</td>
<td>Table 7 and Std. Plans</td>
</tr>
<tr>
<td>Temporary Geosynthetic Retaining Wall</td>
<td>Tables 7 and 10</td>
</tr>
<tr>
<td>Prefabricated Drainage Mat</td>
<td>Table 8</td>
</tr>
<tr>
<td>Table 10 will be included in the Special Provisions.</td>
<td></td>
</tr>
</tbody>
</table>

Geogrid and geotextile reinforcement in geosynthetic retaining walls shall conform to the properties specified in the Standard Plans for permanent walls, and Table 10 for temporary walls.

For geosynthetic retaining walls that use geogrid reinforcement, the geotextile material placed at the wall face to retain the backfill material as shown in the Plans shall conform to the properties for Construction Geotextile for Underground Drainage, Moderate Survivability, Class A.

Thread used for sewing geotextiles shall consist of high strength polypropylene, polyester, or polyamide. Nylon threads will not be allowed. The thread used to sew permanent erosion control geotextiles, and to sew geotextile seams in exposed faces of temporary or permanent geosynthetic retaining walls, shall also be resistant to ultraviolet radiation. The thread shall be of contrasting color to that of the geotextile itself.
9-33.2 Geosynthetic Properties

9-33.2(1) Geotextile Properties

Table 1: Geotextile for underground drainage strength properties for survivability.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method²</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>Low Survivability</td>
</tr>
<tr>
<td>Grab Tensile Strength, in machine and x-machine direction</td>
<td>D 4632</td>
<td>180 lb min.</td>
</tr>
<tr>
<td>Grab Failure Strain, in machine and x-machine direction</td>
<td>D 4632</td>
<td>&lt; 50%</td>
</tr>
<tr>
<td>Seam Breaking Strength</td>
<td>D 4632</td>
<td>160 lb min.</td>
</tr>
<tr>
<td>Puncture Resistance</td>
<td>D 6241</td>
<td>370 lb min.</td>
</tr>
<tr>
<td>Tear Strength, in machine and x-machine direction</td>
<td>D 4533</td>
<td>67 lb min.</td>
</tr>
<tr>
<td>Ultraviolet (UV) Radiation Stability</td>
<td>D 4355</td>
<td>50% strength retained min., after 500 hours in a xenon arc device</td>
</tr>
</tbody>
</table>

Table 2: Geotextile for underground drainage filtration properties.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method²</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>Class A</td>
</tr>
<tr>
<td>AOS</td>
<td>D 4751</td>
<td>U.S. No. 40 max.</td>
</tr>
<tr>
<td>Water Permittivity</td>
<td>D 4491</td>
<td>0.5 sec⁻¹ min.</td>
</tr>
</tbody>
</table>

Table 3: Geotextile for separation or soil stabilization.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method²</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>Separation</td>
</tr>
<tr>
<td></td>
<td></td>
<td>Woven</td>
</tr>
<tr>
<td>AOS</td>
<td>D 4751</td>
<td>U.S. No. 30 max.</td>
</tr>
<tr>
<td>Water</td>
<td>D 4491</td>
<td>0.02 sec⁻¹ min.</td>
</tr>
<tr>
<td>Permittivity</td>
<td>D 4632</td>
<td>250 lb min.</td>
</tr>
<tr>
<td>------------------------------------</td>
<td>--------</td>
<td>-------------</td>
</tr>
<tr>
<td>Grab Failure Strain,</td>
<td>D 4632</td>
<td>&lt; 50%</td>
</tr>
<tr>
<td>in machine and x-machine direction</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Seam Breaking Strength</td>
<td>D 4632</td>
<td>220 lb min.</td>
</tr>
<tr>
<td>Puncture Resistance</td>
<td>D 6241</td>
<td>495 lb min.</td>
</tr>
<tr>
<td>Tear Strength,</td>
<td>D 4533</td>
<td>80 lb min.</td>
</tr>
<tr>
<td>in machine and x-machine direction</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Ultraviolet (UV) Radiation Stability</td>
<td>D 4355</td>
<td></td>
</tr>
</tbody>
</table>

50% strength retained min., after 500 hours in xenon arc device

Table 4: Geotextile for permanent erosion control and ditch lining.
<table>
<thead>
<tr>
<th>Resistance</th>
<th>min.</th>
<th>min.</th>
<th>min.</th>
<th>min.</th>
<th>min.</th>
<th>min.</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tear Strength, in machine and x-machine direction</td>
<td>D 4533</td>
<td>80 lb min.</td>
<td>50 lb min.</td>
<td>112 lb min.</td>
<td>79 lb min.</td>
<td>80 lb min.</td>
</tr>
<tr>
<td>Ultraviolet (UV) Radiation Stability</td>
<td>D 4355</td>
<td>70% strength retained min., after 500 hours in xenon arc device</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Table 5: Filtration properties for geotextile for permanent erosion control.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td>AOS</td>
<td>D 4751</td>
<td>U.S. No. 40 max.</td>
</tr>
<tr>
<td>Water Permittivity</td>
<td>D 4491</td>
<td>0.7 sec⁻¹ min.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>U.S. No. 60 max.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>0.4 sec⁻¹ min.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>U.S. No. 70 max.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>0.2 sec⁻¹ min.</td>
</tr>
</tbody>
</table>

Table 6: Geotextile for temporary silt fence.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td>AOS</td>
<td>D 4751</td>
<td>U.S. No. 30 max. for slit wovens, U.S. No. 50 for all other geotextile types, U.S. No. 100 min.</td>
</tr>
<tr>
<td>Water Permittivity</td>
<td>D 4491</td>
<td>0.02 sec⁻¹ min.</td>
</tr>
<tr>
<td>Grab Tensile Strength, in machine and x-machine direction</td>
<td>D 4632</td>
<td>180 lb min. in machine direction, 100 lb min. in x-machine direction</td>
</tr>
<tr>
<td>Grab Failure Strain, in machine and x-machine direction</td>
<td>D 4632</td>
<td>30% max. at 180 lb or more</td>
</tr>
<tr>
<td>Ultraviolet (UV) Radiation Stability</td>
<td>D 4355</td>
<td>70% strength retained min., after 500 hours in xenon arc device</td>
</tr>
</tbody>
</table>

9-33.2(2) Geosynthetic Properties For Retaining Walls and Reinforced Slopes

Table 7: Minimum properties required for geotextile reinforcement used in geosynthetic reinforced slopes and retaining walls.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>Woven</td>
</tr>
<tr>
<td></td>
<td></td>
<td>Nonwoven</td>
</tr>
</tbody>
</table>

C 3120 - Woodin Road, W.  Page 67  Amendments
C 2963 - Edison Road, W.
9-33.2(3) Prefabricated Drainage Mat
Prefabricated drainage mat shall have a single or double dimpled polymeric core with a
geotextile attached and shall meet the following requirements:

Table 8: Minimum properties required for prefabricated drainage mats.

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>ASTM Test Method</th>
<th>Geotextile Property Requirements¹</th>
</tr>
</thead>
<tbody>
<tr>
<td>AOS</td>
<td>D 4751</td>
<td>U.S. No. 20 max.</td>
</tr>
<tr>
<td>Water Permittivity</td>
<td>D 4491</td>
<td>0.02 sec⁻¹ min.</td>
</tr>
<tr>
<td>Grab Tensile Strength, in machine and</td>
<td></td>
<td></td>
</tr>
<tr>
<td>x-machine direction</td>
<td>D 4632</td>
<td>200 lb min.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>120 lb min.</td>
</tr>
<tr>
<td>Grab Failure Strain, in machine and</td>
<td></td>
<td></td>
</tr>
<tr>
<td>x-machine direction</td>
<td>D 4632</td>
<td>&lt; 50%</td>
</tr>
<tr>
<td></td>
<td></td>
<td>≥ 50%</td>
</tr>
<tr>
<td>Seam Breaking Strength</td>
<td>D 4632, 34</td>
<td>160 lb min.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>100 lb min.</td>
</tr>
<tr>
<td>Puncture Resistance</td>
<td>D 6241</td>
<td>370 lb min.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>220 lb min.</td>
</tr>
<tr>
<td>Tear Strength, in machine and</td>
<td></td>
<td></td>
</tr>
<tr>
<td>x-machine direction</td>
<td>D 4533</td>
<td>63 lb min.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>50 lb min.</td>
</tr>
<tr>
<td>Ultraviolet (UV) Radiation Stability</td>
<td>D 4355</td>
<td>70% (for polypropylene and polyethylene) and 50% (for polyester) Strength Retained min., after 500 hours in a xenon arc device</td>
</tr>
</tbody>
</table>

¹All geotextile properties in Tables 1 through 8 are minimum average roll values (i.e., the test results for any sampled roll in a lot shall meet or exceed the values shown in the table).
The test procedures used are essentially in conformance with the most recently approved ASTM geotextile test procedures, except for geotextile sampling and specimen conditioning, which are in accordance with WSDOT Test Methods T 914, Practice for Sampling of Geotextiles for Testing, and T 915, Practice for Conditioning of Geotextiles for Testing, respectively. Copies of these test methods are available at the State Materials Laboratory P.O. Box 47365, Olympia, WA 98504-7365.

With seam located in the center of 8-inch long specimen oriented parallel to grip faces.

Applies only to seams perpendicular to the wall face.

9-33.3 Aggregate Cushion of Permanent Erosion Control Geotextile
Aggregate cushion for permanent erosion control geotextile, Class A shall meet the requirements of Section 9-03.9(2). Aggregate cushion for permanent erosion control geotextile, Class B or C shall meet the requirements of Section 9-03.9(3) and 9-03.9(2).

9-33.4 Geosynthetic Material Approval and Acceptance
9-33.4(1) Geosynthetic Material Approval
If the geosynthetic source material has not been previously evaluated, or is not listed in the current WSDOT Qualified Products List (QPL), a sample of each proposed geosynthetic shall be submitted to the State Materials Laboratory in Tumwater for evaluation. Geosynthetic material approval will be based on conformance to the applicable properties from the Tables in Section 9-33.2 or in the Standard Plans or Special Provisions. After the sample and required information for each geosynthetic type have arrived at the State Materials Laboratory in Tumwater, a maximum of 14 calendar days will be required for this testing. Source approval shall not be the basis of acceptance of specific lots of material delivered to the Contractor unless the roll numbers of the lot sampled can be clearly identified as the rolls tested and approved in the geosynthetic approval process.

For geogrid and geotextile products proposed for use in permanent geosynthetic retaining walls or reinforced slopes that are not listed in the current QPL, the Contractor shall submit test information and the calculations used in the determination of $T_{sl}$ performed in accordance with WSDOT Standard Practice T 925, Standard Practice for Determination of Long-Term Strength for Geosynthetic Reinforcement, to the State Materials Laboratory in Tumwater for evaluation. The Contracting Agency will require up to 30 calendar days after receipt of the information to complete the evaluation.

The Contractor shall submit to the Engineer the following information regarding each geosynthetic material proposed for use:

Manufacturer’s name and current address,
Full product name,
Geosynthetic structure, including fiber/yarn type,
Geosynthetic polymer type(s) (for temporary and permanent geosynthetic retaining walls),
Proposed geosynthetic use(s), and
Certified test results for minimum average roll values.

9-33.4(2) Vacant

9-33.4(3) Acceptance Samples
When the quantities of geosynthetic materials proposed for use in the following geosynthetic applications are greater than the following amounts, acceptance shall be by satisfactory test report:

<table>
<thead>
<tr>
<th>Application</th>
<th>Geosynthetic Quantity</th>
</tr>
</thead>
<tbody>
<tr>
<td>Underground Drainage</td>
<td>600 sq. yd.</td>
</tr>
<tr>
<td>Temporary or Permanent Geosynthetic</td>
<td>All quantities</td>
</tr>
<tr>
<td>Retaining Walls</td>
<td></td>
</tr>
</tbody>
</table>

The samples for acceptance testing shall include the information about each geosynthetic roll to be used as stated in 9-33.4(4).

Samples will be randomly taken by the Engineer at the job site to confirm that the geosynthetic meets the property values specified.

Approval will be based on testing of samples from each lot. A "lot" shall be defined for the purposes of this specification as all geosynthetic rolls within the consignment (i.e., all rolls sent the project site) that were produced by the same manufacturer during a continuous period of production at the same manufacturing plant and have the same product name. After the samples have arrived at the State Materials Laboratory in Tumwater, a maximum of 14 calendar days will be required for this testing.

If the results of the testing show that a geosynthetic lot, as defined, does not meet the properties required for the specified use as indicated in Tables 1 through 8 in Section 9-33.2, and additional tables as specified in the Special Provisions, the roll or rolls which were sampled will be rejected. Geogrids and geotextiles for temporary geosynthetic retaining walls shall meet the requirements of Table 7, and Table 10 in the Special Provisions. Geogrids and geotextiles for permanent geosynthetic retaining wall shall meet the requirements of Table 7, and Table 9 in the Special Provisions, and both geotextile and geogrid acceptance testing shall meet the required ultimate tensile strength $T_{ult}$ as provided in the current QPL for the selected product(s). If the selected product(s) are not listed in the current QPL, the result of the testing for $T_{ult}$ shall be greater than or equal to $T_{ult}$ as determined from the product data submitted and approved by the State Materials Laboratory during source material approval.

Two additional rolls for each roll tested which failed from the lot previously tested will then be selected at random by the Engineer for sampling and retesting. If the retesting shows that any of the additional rolls tested do not meet the required properties, the entire lot will be rejected. If the test results from all the rolls retested meet the required
properties, the entire lot minus the roll(s) that failed will be accepted. All geosynthetic that has defects, deterioration, or damage, as determined by the Engineer, will also be rejected. All rejected geosynthetic shall be replaced at no additional expense to the Contracting Agency.

9-33.4(4) Acceptance by Certificate of Compliance

When the quantities of geosynthetic proposed for use in each geosynthetic application are less than or equal to the following amounts, acceptance shall be by Manufacturer’s Certificate of Compliance:

<table>
<thead>
<tr>
<th>Application</th>
<th>Geosynthetic Quantity</th>
</tr>
</thead>
<tbody>
<tr>
<td>Underground Drainage</td>
<td>600 sq. yd.</td>
</tr>
<tr>
<td>Soil Stabilization and Separation</td>
<td>All quantities</td>
</tr>
<tr>
<td>Permanent Erosion Control</td>
<td>All quantities</td>
</tr>
<tr>
<td>Temporary Silt Fence</td>
<td>All quantities</td>
</tr>
<tr>
<td>Prefabricated Drainage Mat</td>
<td>All quantities</td>
</tr>
</tbody>
</table>

The Manufacturer’s Certificate of Compliance shall include the following information about each geosynthetic roll to be used:

Manufacturer’s name and current address,
Full product name,
Geosynthetic structure, including fiber/yarn type,
Geosynthetic Polymer type (for all temporary and permanent geosynthetic retaining walls only),
Geosynthetic roll number(s),
Geosynthetic lot number(s),
Proposed geosynthetic use(s), and
Certified test results.

9-33.4(5) Approval of Seams

If the geotextile seams are to be sewn in the field, the Contractor shall provide a section of sewn seam that can be sampled by the Engineer before the geotextile is installed.

The seam sewn for sampling shall be sewn using the same equipment and procedures as will be used to sew the production seams. If production seams will be sewn in both the machine and cross-machine directions, the Contractor must provide sewn seams for sampling which are oriented in both the machine and cross-machine directions. The seams sewn for sampling must be at least 2 yards in length in each geotextile direction. If the seams are sewn in the factory, the Engineer will obtain samples of the factory seam at random from any of the rolls to be used. The seam assembly description shall be submitted by the Contractor to the Engineer and will be included with the seam sample obtained for testing. This description shall include the seam type, stitch type, sewing thread type(s), and stitch density.
SECTION 9-35, TEMPORARY TRAFFIC CONTROL MATERIALS
April 3, 2006

9-35.2 Construction Signs
The first paragraph is supplemented with the following:
Post mounted Class A construction signs shall conform to the requirements of this section and additionally shall conform to the requirements stated in section 9-28.

The second paragraph is revised to read:
Aluminum sheeting shall be used to fabricate all construction signs. The signs shall have a minimum thickness of 0.080-inches and a maximum thickness of 0.125-inches.

The first sentence in the fourth paragraph is revised to read:
The use of plywood, composite, fiberglass reinforced plastic, new fabric rollup signs, and any other previously approved sign materials except aluminum is prohibited. Any sign which otherwise meets the requirements of this section and was purchased prior to July 1, 2004, may be utilized until December 31, 2007. If a fabric sign is used, it shall have been fabricated with Type VI reflective sheeting.
Special Provisions
SPECIAL PROVISIONS

C 3120 - WOODIN ROAD, W. IMPROVEMENT PROJECT
(Maple Grove Road to Scoon Road)
C 2963 - EDISON ROAD, W. IMPROVEMENT PROJECT
(Swan Road to Sunnyside City Limits)

YAKIMA COUNTY, WASHINGTON

The following Special Provisions are made a part of this contract and supersede any conflicting provisions of the 2006 Standard Specifications for Road, Bridge and Municipal Construction, and the foregoing Amendments to the Standard Specifications.

Several types of Special Provisions are included in this contract; General, Region, Bridges and Structures, and Project Specific. Special Provisions types are differentiated as follows:

(date) General Special Provision
(******) Notes a revision to a General Special Provision
and also notes a Project Specific Special Provision.
(Regions¹ date) Region Special Provision
(BSP date) Bridges and Structures Special Provision

General Special Provisions are similar to Standard Specifications in that they typically apply to many projects, usually in more than one Region. Usually, the only difference from one project to another is the inclusion of variable project data, inserted as a “fill-in”.

Region Special Provisions are commonly applicable within the designated Region. Region designations are as follows:

Regions¹
ER Eastern Region
NCR North Central Region
NWR Northwest Region
OR Olympic Region
SCR South Central Region
SWR Southwest Region
WSF Washington State Ferries Division

Bridges and Structures Special Provisions are similar to Standard Specifications in that they typically apply to many projects, usually in more than one Region. Usually, the only difference from one project to another is the inclusion of variable project data, inserted as a “fill-in”.

Project Specific Special Provisions normally appear only in the contract for which they were developed.
DIVISION 1
GENERAL REQUIREMENTS

DESCRIPTION OF WORK

(March 13, 1995)
The work to be performed under this Contract consists of the improvement of approximately 1.00 miles of Woodin Road, W., from Maple Grove Road to Scoon Road and 0.40 miles of Edison Road, W., from Swan Road to Sunnyside Corporate Limits. These improvements consist of grading, draining, placing and compacting base course and top course, paving with Bituminous Surface Treatment and Hot Mix Asphalt, and other work, in accordance with the attached Plans, these Special Provisions and the 2006 Standard Specifications and Amendments thereto.

The portion of Woodin Road, W. to be improved is located in Section 23, Township 10 North, Range 22 East, Willamette Meridian.

The portion of Edison Road, W. to be improved is located in Section 26, Township 10 North, Range 22 East, Willamette Meridian.

The quantities of work indicated in the proposal are to be considered as estimates and are for comparative bidding purposes only. All payments shall be made on the basis of actual field measurement of Contract work completed.

Funds

(******)
Yakima County Road funds are involved in the construction of these improvements.

1-01.3 Definitions

1-01.3 Definitions
(May 25, 2006 APWA GSP)

This Section is supplemented with the following:

All references in the Standard Specifications to the terms “State”, “Department of Transportation”, “Washington State Transportation Commission”, “Commission”, “Secretary of Transportation”, “Secretary”, “Headquarters”, and “State Treasurer” shall be revised to read “Contracting Agency”.

All references to “State Materials Laboratory” shall be revised to read “Contracting Agency designated location”.
The venue of all causes of action arising from the advertisement, award, execution, and performance of the contract shall be in the Superior Court of the County where the Contracting Agency's headquarters are located.

**Additive**
A supplemental unit of work or group of bid items, identified separately in the proposal, which may, at the discretion of the Contracting Agency, be awarded in addition to the base bid.

**Alternate**
One of two or more units of work or groups of bid items, identified separately in the proposal, from which the Contracting Agency may make a choice between different methods or material of construction for performing the same work.

**Contract Documents**
See definition for “Contract”.
Contract Time The period of time established by the terms and conditions of the contract within which the work must be physically completed.

**Dates**

**Bid Opening Date**
The date on which the Contracting Agency publicly opens and reads the bids.

**Award Date**
The date of the formal decision of the Contracting Agency to accept the lowest responsible and responsive bidder for the work.

**Contract Execution Date**
The date the Contracting Agency officially binds the agency to the contract.

**Notice to Proceed Date**
The date stated in the Notice to Proceed on which the contract time begins.

**Substantial Completion Date**
The day the Engineer determines the Contracting Agency has full and unrestricted use and benefit of the facilities, both from the operational and safety standpoint, and only minor incidental work, replacement of temporary substitute facilities, or correction or repair remains for the physical completion of the total contract.

**Physical Completion Date**
The day all of the work is physically completed on the project. All documentation required by the contract and required by law does not necessarily need to be furnished by the Contractor by this date.

**Completion Date**
The day all the work specified in the contract is completed and all the obligations of the Contractor under the contract are fulfilled by the Contractor. All documentation required by the contract and required by law must be furnished by the Contractor before establishment of this date.
Final Acceptance Date
The date on which the Contracting Agency accepts the work as complete.

Notice of Award
The written notice from the Contracting Agency to the successful bidder signifying the Contracting Agency’s acceptance of the bid.

Notice to Proceed
The written notice from the Contracting Agency or Engineer to the Contractor authorizing and directing the Contractor to proceed with the work and establishing the date on which the contract time begins.

Traffic
Both vehicular and non-vehicular traffic, such as pedestrians, bicyclists, wheelchairs, and equestrian traffic.

SECTION 1-02, BID PROCEDURES AND CONDITIONS

1-02.1 Prequalification of Bidders
Delete this Section and replace it with the following:

1-02.1 Qualifications of Bidder
(October 1, 2005 APWA GSP)
Bidders shall be qualified by experience, financing, equipment, and organization to do the work called for in the Contract Documents. The Contracting Agency reserves the right to take whatever action it deems necessary to ascertain the ability of the bidder to perform the work satisfactorily.

1-02.2 Plans and Specifications
(October 1, 2005 APWA GSP)
Delete this section and replace it with the following:

Information as to where Bid Documents can be obtained or reviewed will be found in the Call for Bids (Advertisement for Bids) for the work.

After award of the contract, plans and specifications will be issued to the Contractor at no cost as detailed below:

<table>
<thead>
<tr>
<th>To Prime Contractor</th>
<th>No. of Sets</th>
<th>Basis of Distribution</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reduced plans (11&quot; x 17&quot;) and Contract Provisions</td>
<td>10</td>
<td>Furnished automatically upon award.</td>
</tr>
<tr>
<td>Large plans (22&quot; x 34&quot;)</td>
<td>0</td>
<td>Furnished only upon</td>
</tr>
</tbody>
</table>

C 3120 - Woodin Road, W.
C 2963 - Edison Road, W.
and Contract Provisions request.

Additional plans and Contract Provisions may be purchased by the Contractor by payment of the cost stated in the Call for Bids.

1-02.5 Proposal Forms
(October 1, 2005 APWA GSP)

Delete this section and replace it with the following:

At the request of a bidder, the Contracting Agency will provide a proposal form for any project on which the bidder is eligible to bid.

The proposal form will identify the project and its location and describe the work. It will also list estimated quantities, units of measurement, the items of work, and the materials to be furnished at the unit bid prices. The bidder shall complete spaces on the proposal form that call for, but are not limited to, unit prices; extensions; summations; the total bid amount; signatures; date; and, where applicable, retail sales taxes and acknowledgment of addenda; the bidder’s name, address, telephone number, and signature; the bidder’s D/M/WBE commitment, if applicable; a State of Washington Contractor’s Registration Number; and a Business License Number, if applicable. Bids shall be completed by typing or shall be printed in ink by hand, preferably in black ink. The required certifications are included as part of the proposal form.

The Contracting Agency reserves the right to arrange the proposal forms with alternates and additives, if such be to the advantage of the Contracting Agency. The bidder shall bid on all alternates and additives set forth in the proposal forms unless otherwise specified.

Any correction to a bid made by interlineation, alteration, or erasure, shall be initialed by the signer of the bid. The bidder shall make no stipulation on the Bid Form, nor qualify the bid in any manner.

A bid by a corporation shall be executed in the corporate name, by the president or a vice president (or other corporate officer accompanied by evidence of authority to sign).

A bid by a partnership shall be executed in the partnership name, and signed by a partner. A copy of the partnership agreement shall be submitted with the Bid Form if any D/M/WBE requirements are to be satisfied through such an agreement.

A bid by a joint venture shall be executed in the joint venture name and signed by a member of the joint venture. A copy of the joint venture agreement shall be submitted with the Bid Form if any D/W/MBE requirements are to be satisfied through such an agreement.
1-02.7 Bid Deposit

_October 1, 2005 APWA GSP_

Supplement this section with the following:

Bid bonds shall contain the following:
1. Contracting Agency-assigned number for the project;
2. Name of the project;
3. The Contracting Agency named as obligee;
4. The amount of the bid bond stated either as a dollar figure or as a percentage which represents five percent of the maximum bid amount that could be awarded;
5. Signature of the bidder’s officer empowered to sign official statements. The signature of the person authorized to submit the bid should agree with the signature on the bond, and the title of the person must accompany the said signature;
6. The signature of the surety’s officer empowered to sign the bond and the power of attorney.

If so stated in the Contract Provisions, bidder must use the bond form included in the Contract Provisions.

1-02.9 Delivery of Proposal

_(October 1, 2005 APWA GSP)_

Revise the first paragraph to read:

Each proposal shall be submitted in a sealed envelope, with the Project Name and Project Number as stated in the Advertisement for Bids clearly marked on the outside of the envelope, or as otherwise stated in the Bid Documents, to ensure proper handling and delivery.

1-02.13 Irregular Proposals

_(October 1, 2005 APWA GSP)_

Revise item 1 to read:

1. A proposal will be considered irregular and will be rejected if:
   a. The bidder is not prequalified when so required;
   b. The authorized proposal form furnished by the Contracting Agency is not used or is altered;
   c. The completed proposal form contains any unauthorized additions, deletions, alternate bids, or conditions;
   d. The bidder adds provisions reserving the right to reject or accept the award, or enter into the contract;
   e. A price per unit cannot be determined from the bid proposal;
   f. The proposal form is not properly executed;
g. The bidder fails to submit or properly complete a subcontractor list, if applicable, as required in Section 1 02.6.

h. The bidder fails to submit or properly complete a Disadvantaged, Minority or Women’s Business Enterprise Certification, if applicable, as required in Section 1-02.6; or

i. The bid proposal does not constitute a definite and unqualified offer to meet the material terms of the bid invitation.

1-02.14 Disqualification of Bidders

(October 1, 2005 APWA GSP)

Revise this section to read:

A bidder may be deemed not responsible and the proposal rejected if:

1. More than one proposal is submitted for the same project from a bidder under the same or different names;

2. Evidence of collusion exists with any other bidder or potential bidder. Participants in collusion will be restricted from submitting further bids;

3. The bidder, in the opinion of the Contracting Agency, is not qualified for the work or to the full extent of the bid, or to the extent that the bid exceeds the authorized prequalification amount as may have been determined by a prequalification of the bidder;

4. An unsatisfactory performance record exists based on past or current Contracting Agency work or for work done for others, as judged from the standpoint of conduct of the work; workmanship; progress; affirmative action; equal employment opportunity practices; or Disadvantaged Business Enterprise, Minority Business Enterprise, or Women’s Business Enterprise utilization;

5. There is uncompleted work (Contracting Agency or otherwise) which might hinder or prevent the prompt completion of the work bid upon;

6. The bidder failed to settle bills for labor or materials on past or current contracts;

7. The bidder has failed to complete a written public contract or has been convicted of a crime arising from a previous public contract;

8. The bidder is unable, financially or otherwise, to perform the work;

9. A bidder is not authorized to do business in the State of Washington (not registered in accordance with RCW 18.27);

10. There are any other reasons deemed proper by the Contracting Agency.

SECTION 1-03, AWARD AND EXECUTION OF CONTRACT

1-03.1 Consideration of Bids

(January 23, 2006 APWA GSP)

Revise the first paragraph to read:

After opening and reading proposals, the Contracting Agency will check them for correctness
of extensions of the prices per unit and the total price. If a discrepancy exists between the
price per unit and the extended amount of any bid item, the price per unit will control. If a
minimum bid amount has been established for any item and the bidder’s unit or lump sum
price is less than the minimum specified amount, the Contracting Agency will unilaterally
revise the unit or lump sum price, to the minimum specified amount and recalculate the
extension. The total of extensions, corrected where necessary, including sales taxes where
applicable and such additives and/or alternates as selected by the Contracting Agency, will be
used by the Contracting Agency for award purposes and to fix the Awarded Contract Price
amount and the amount of the contract bond.

1-03.3 Execution of Contract

(October 1, 2005 APWA GSP)

Revise this section to read:

Copies of the Contract Provisions, including the unsigned Form of Contract, will be available
for signature by the successful bidder on the first business day following award. The number
of copies to be executed by the Contractor will be determined by the Contracting Agency.

Within _10_ calendar days after the award date, the successful bidder shall return the signed
Contracting Agency-prepared contract, an insurance certification as required by Section 1-
07.18, and a satisfactory bond as required by law and Section 1-03.4. Before execution of the
contract by the Contracting Agency, the successful bidder shall provide any pre-award
information the Contracting Agency may require under Section 1-02.15.

Until the Contracting Agency executes a contract, no proposal shall bind the Contracting
Agency nor shall any work begin within the project limits or within Contracting Agency-
furnished sites. The Contractor shall bear all risks for any work begun outside such areas and
for any materials ordered before the contract is executed by the Contracting Agency.

If the bidder experiences circumstances beyond their control that prevents return of the
contract documents within _the_ calendar days after the award date stated above, the
Contracting Agency may grant up to a maximum of _10_ additional calendar days for return
of the documents, provided the Contracting Agency deems the circumstances warrant it.

1-03.4 Contract Bond

(October 1, 2005 APWA GSP)

Revise the first paragraph to read:

The successful bidder shall provide an executed contract bond for the full contract amount.
This contract bond shall:
1. Be on a Contracting Agency-furnished form;
2. Be signed by an approved surety (or sureties) that:
a. Is registered with the Washington State Insurance Commissioner, and
b. Appears on the current Authorized Insurance List in the State of Washington published by the Office of the Insurance Commissioner,

3. Be conditioned upon the faithful performance of the contract by the Contractor within the prescribed time;

4. Guarantee that the surety shall indemnify, defend, and protect the Contracting Agency against any claim of direct or indirect loss resulting from the failure:
   a. Of the Contractor (or any of the employees, subcontractors, or lower tier subcontractors of the Contractor) to faithfully perform the contract, or
   b. Of the Contractor (or the subcontractors or lower tier subcontractors of the Contractor) to pay all laborers, mechanics, subcontractors, lower tier subcontractors, material person, or any other person who provides supplies or provisions for carrying out the work;

5. Be accompanied by a power of attorney for the Surety’s officer empowered to sign the bond; and

6. Be signed by an officer of the Contractor empowered to sign official statements (sole proprietor or partner). If the Contractor is a corporation, the bond must be signed by the president or vice-president, unless accompanied by written proof of the authority of the individual signing the bond to bind the corporation (i.e., corporate resolution, power of attorney or a letter to such effect by the president or vice-president).

1-04.2 Coordination of Contract Documents, Plans, Special Provisions, Specifications, and Addenda
(October 1, 2005 APWA GSP)

Revise the second paragraph to read:

Any inconsistency in the parts of the contract shall be resolved by following this order of precedence (e.g., 1 presiding over 2, 2 over 3, 3 over 4, and so forth):

1. Addenda,

2. Proposal Form,

3. Special Provisions, including APWA General Special Provisions, if they are included,

4. Contract Plans,

5. Amendments to the Standard Specifications,

6. WSDOT/APWA Standard Specifications for Road, Bridge and Municipal Construction,

7. Contracting Agency’s Standard Plans (if any), and

8. WSDOT/APWA Standard Plans for Road, Bridge, and Municipal Construction.

SECTION 1-05, CONTROL OF WORK

1-05.7 Removal of Defective and Unauthorized Work
(October 1, 2005 APWA GSP)

Supplement this section with the following:
If the Contractor fails to remedy defective or unauthorized work within the time specified in a written notice from the Engineer, or fails to perform any part of the work required by the Contract Documents, the Engineer may correct and remedy such work as may be identified in the written notice, with Contracting Agency forces or by such other means as the Contracting Agency may deem necessary.

If the Contractor fails to comply with a written order to remedy what the Engineer determines to be an emergency situation, the Engineer may have the defective and unauthorized work corrected immediately, have the rejected work removed and replaced, or have work the Contractor refuses to perform completed by using Contracting Agency or other forces. An emergency situation is any situation when, in the opinion of the Engineer, a delay in its remedy could be potentially unsafe, or might cause serious risk of loss or damage to the public.

Direct or indirect costs incurred by the Contracting Agency attributable to correcting and remediing defective or unauthorized work, or work the Contractor failed or refused to perform, shall be paid by the Contractor. Payment will be deducted by the Engineer from monies due, or to become due, the Contractor. Such direct and indirect costs shall include in particular, but without limitation, compensation for additional professional services required, and costs for repair and replacement of work of others destroyed or damaged by correction, removal, or replacement of the Contractor’s unauthorized work.

No adjustment in contract time or compensation will be allowed because of the delay in the performance of the work attributable to the exercise of the Contracting Agency’s rights provided by this Section.

The rights exercised under the provisions of this section shall not diminish the Contracting Agency’s right to pursue any other avenue for additional remedy or damages with respect to the Contractor’s failure to perform the work as required.

1-05.13 Superintendents, Labor and Equipment of Contractor
(May 25, 2006 APWA GSP)

Revise the seventh paragraph to read:

Whenever the Contracting Agency evaluates the Contractor’s qualifications pursuant to Section 1-02.1, it will take these performance reports into account.

1-05.14 Cooperation With other Contractors
(March 13, 1995)

Cooperation With Other Contractors

Section 1-05.14 is supplemented with the following:
Other Contracts Or Other Work

It is anticipated that the following work adjacent to or within the limits of this project will be performed by others during the course of this project and will require coordination of the work:

1. Utility Work.
   No additional payment will be made for this utility coordination work and all costs shall be incidental to the unit contract prices and no further payment shall be made.

Add the following new section:

1-05.16 Water and Power
(October 1, 2005 APWA GSP)

The Contractor shall make necessary arrangements, and shall bear the costs for power and water necessary for the performance of the work, unless the contract includes power and water as a pay item.

Add the following new section:

1-05.17 Oral Agreements
(October 1, 2005 AWPA GSP)

No oral agreement or conversation with any officer, agent, or employee of the Contracting Agency, either before or after execution of the contract, shall affect or modify any of the terms or obligations contained in any of the documents comprising the contract. Such oral agreement or conversation shall be considered as unofficial information and in no way binding upon the Contracting Agency, unless subsequently put in writing and signed by the Contracting Agency.

SECTION 1-06, CONTROL OF MATERIAL

1-06.2(2) Statistical Evaluation of Materials for Acceptance

(******)

Section 1-06.2(2) of the Standard Specifications is deleted.

March 13, 1995

Foreign Made Materials

Section 1-06 is supplemented with the following:
The major quantities of steel and iron construction material that is permanently incorporated into the project shall consist of American-made materials only.

The Contractor may utilize minor amounts of foreign steel and iron in this project provided the cost of the foreign material used does not exceed one-tenth of one percent of the total contract cost or $2,500.00, whichever is greater.

American-made material is defined as material having all manufacturing processes occur in the United States. The action of applying a coating to steel or iron is deemed a manufacturing process. Coating includes epoxy coating, galvanizing, aluminizing, painting, and any other coating that protects or enhances the value of steel or iron. Any process from the original reduction from ore to the finished product constitutes a manufacturing process for iron. The following are considered to be steel manufacturing processes:

1. Production of steel by any of the following processes:
   a. Open hearth furnace.
   b. Basic oxygen.
   c. Electric furnace.
   d. Direct reduction.

2. Rolling, heat treating, and any other similar processing.

3. Fabrication of the products.
   a. Spinning wire into cable or strand.
   b. Corrugating and rolling into culverts.
   c. Shop fabrication.

A certification of materials origin will be required for any items comprised of, or containing, steel or iron construction materials prior to such items being incorporated into the permanent work. The certification shall be on DOT Form 350-109 provided by the Engineer, or such other form the Contractor chooses, provided it contains the same information as DOT Form 350-109.

SECTION 1-07, LEGAL RELATIONS AND RESPONSIBILITIES TO THE PUBLIC

1-07.5 Environmental Regulations
1-07.2 State Sales Tax

Delete this section, including its sub-sections, in its entirety and replace it with the following:

1-07.2 State Sales Tax
(October 1, 2005 APWA GSP)

1-07.2(1) General

The Washington State Department of Revenue has issued special rules on the State sales tax. Sections 1-07.2(1) through 1-07.2(4) are meant to clarify those rules. The Contractor should contact the Washington State Department of Revenue for answers to questions in this area. The Contracting Agency will not adjust its payment if the Contractor bases a bid on a misunderstood tax liability.

The Contractor shall include all Contractor-paid taxes in the unit bid prices or other contract amounts. In some cases, however, state retail sales tax will not be included. Section 1-07.2(3) describes this exception.

The Contracting Agency will pay the retained percentage only if the Contractor has obtained from the Washington State Department of Revenue a certificate showing that all contract-related taxes have been paid (RCW 60.28.050). The Contracting Agency may deduct from its payments to the Contractor any amount the Contractor may owe the Washington State Department of Revenue, whether the amount owed relates to this contract or not. Any amount so deducted will be paid into the proper State fund.

1-07.2(2) State Sales Tax — Rule 171

WAC 458-20-171, and its related rules, apply to building, repairing, or improving streets, roads, etc., which are owned by a municipal corporation, or political subdivision of the state, or by the United States, and which are used primarily for foot or vehicular traffic. This includes storm or combined sewer systems within and included as a part of the street or road drainage system and power lines when such are part of the roadway lighting system. For work performed in such cases, the Contractor shall include Washington State Retail Sales Taxes in the various unit bid item prices, or other contract amounts, including those that the Contractor pays on the purchase of the materials, equipment, or supplies used or consumed in doing the work.

1-07.2(3) State Sales Tax — Rule 170

WAC 458-20-170, and its related rules, apply to the constructing and repairing of new or existing buildings, or other structures, upon real property. This includes, but is not limited to, the construction of streets, roads, highways, etc., owned by the state of Washington; water mains and their appurtenances; sanitary sewers and sewage disposal systems unless such sewers and disposal systems are within, and a part of, a street or road drainage system; telephone, telegraph, electrical power distribution lines, or other conduits or lines in or above
streets or roads, unless such power lines become a part of a street or road lighting system; and installing or attaching of any article of tangible personal property in or to real property, whether or not such personal property becomes a part of the realty by virtue of installation.

For work performed in such cases, the Contractor shall collect from the Contracting Agency, retail sales tax on the full contract price. The Contracting Agency will automatically add this sales tax to each payment to the Contractor. For this reason, the Contractor shall not include the retail sales tax in the unit bid item prices, or in any other contract amount subject to Rule 170, with the following exception.

Exception: The Contracting Agency will not add in sales tax for a payment the Contractor or a subcontractor makes on the purchase or rental of tools, machinery, equipment, or consumable supplies not integrated into the project. Such sales taxes shall be included in the unit bid item prices or in any other contract amount.

1-07.2(4) Services

The Contractor shall not collect retail sales tax from the Contracting Agency on any contract wholly for professional or other services (as defined in Washington State Department of Revenue Rules 138 and 244).

1-07.13 Contractor's Responsibility For Work

1-07.13(4) Repair of Damage

(August 6, 2001)

Repair of Damage

Section 1-07.13(4) is revised to read:

The Contractor shall promptly repair all damage to either temporary or permanent work as directed by the Engineer. For damage qualifying for relief under Sections 1-07.13(1), 1-07.13(2) or 1-07.13(3), payment will be made in accordance with Section 1-04.4. Payment will be limited to repair of damaged work only. No payment will be made for delay or disruption of work.

1-07.17 Utilities and Similar Facilities

(April 2, 2007)

Utilities and Similar Facilities

Section 1-07.17 is supplemented with the following:

Locations and dimensions shown in the Plans for existing facilities are in accordance with available information obtained without uncovering, measuring, or other verification.

Public and private utilities, or their Contractors, will furnish all work necessary to adjust, relocate, replace, or construct their facilities unless otherwise provided for in the Plans or
these Special Provisions. Such adjustment, relocation, replacement, or construction will be
done during the prosecution of the work for this project. It is anticipated that utility
adjustment, relocation, replacement or construction within the project limits will be
completed as follows:

Most of the utility relocation has been completed, however minor relocations may be
necessary due to conflicts during construction.

The Contractor shall attend a mandatory utility preconstruction meeting with the Engineer,
all affected subcontractors, and all utility owners and their contractors prior to beginning
onsite work.

The following addresses and telephone numbers of utility companies or their Contractors
that will be adjusting, relocating, replacing or constructing utilities within the project limits
are supplied for the Contractor's use:

| Call Before You Dig One Call Center | 1-800-424-5555 |
| Embarq | 409 S. 5th Street, Sunny side, WA 98944 | (509) 839-6660 |
| Pacific Power & Light Co. | 500 N Keys Road., Yakima, WA 98901 | (509) 575-3158 |
| SVID | P. O. Box 239, Sunnyside, WA 98944 | (509) 837-6080 |
| Cascade Natural Gas | 701 S. 1st Avenue, Yakima, WA 98901 | (509) 457-5905 |

1-07.18 Public Liability and Property Damage Insurance

Delete this section in its entirety, and replace it with the following:

1-07.18 Insurance
(May 10, 2006 APWA GSP)

1-07.18(1) General Requirements
A. The Contractor shall obtain the insurance described in this section from insurers approved
by the State Insurance Commissioner pursuant to RCW Title 48. The insurance must be
provided by an insurer with a rating of A:- VII or higher in the A.M. Best's Key Rating
Guide, which is licensed to do business in the state of Washington (or issued as a surplus
line by a Washington Surplus lines broker). The Contracting Agency reserves the right to
approve or reject the insurance provided, based on the insurer (including financial
condition), terms and coverage, the Certificate of Insurance, and/or endorsements.

B. The Contractor shall keep this insurance in force during the term of the contract and for
thirty (30) days after the Physical Completion date, unless otherwise indicated (see C.
below).

C. If any insurance policy is written on a claims made form, its retroactive date, and that of
all subsequent renewals, shall be no later than the effective date of this Contract. The
policy shall state that coverage is claims made, and state the retroactive date. Claims-
made form coverage shall be maintained by the Contractor for a minimum of 36 months
following the Final Completion or earlier termination of this contract, and the Contractor...
shall annually provide the Contracting Agency with proof of renewal. If renewal of the
claims made form of coverage becomes unavailable, or economically prohibitive, the
Contractor shall purchase an extended reporting period ("tail") or execute another form of
guarantee acceptable to the Contracting Agency to assure financial responsibility for
liability for services performed.

D. The insurance polices shall contain a “cross liability” provision.

E. The Contractor’s and all subcontractors’ insurance coverage shall be primary and non-
contributory insurance as respects the Contracting Agency’s insurance, self-insurance, or
insurance pool coverage.

F. All insurance policies and Certificates of Insurance shall include a requirement providing
for a minimum of 30 days prior written notice to the Contracting Agency of any
cancellation in any insurance policy.

G. Upon request, the Contractor shall forward to the Contracting Agency a full and certified
copy of the insurance policy(s).

H. The Contractor shall not begin work under the contract until the required insurance has
been obtained and approved by the Contracting Agency.

I. Failure on the part of the Contractor to maintain the insurance as required shall constitute
a material breach of contract, upon which the Contracting Agency may, after giving five
business days notice to the Contractor to correct the breach, immediately terminate the
contract or, at its discretion, procure or renew such insurance and pay any and all
premiums in connection therewith, with any sums so expended to be repaid to the
Contracting Agency on demand, or at the sole discretion of the Contracting Agency,
offset against funds due the Contractor from the Contracting Agency.

J. All costs for insurance shall be incidental to and included in the unit or lump sum prices
of the contract and no additional payment will be made.

1-07.18(2) Additional Insured

All insurance policies, with the exception of Professional Liability and Workers Compensation,
shall name the following listed entities as additional insured(s):

Yakima County and its officers, elected officials, employees, agents, and volunteers

The above-listed entities shall be additional insured(s) for the full available limits of liability
maintained by the Contractor, whether primary, excess, contingent or otherwise, irrespective of
whether such limits maintained by the Contractor are greater than those required by this
Contract, and irrespective of whether the Certificate of Insurance provided by the Contractor
pursuant to 1-07.18(3) describes limits lower than those maintained by the Contractor.
1-07.18(3) Subcontractors

Contractor shall ensure that each subcontractor of every tier obtains and maintains at a minimum the insurance coverages listed in 1-07.18(5)A and 1-07.18(5)B. Upon request of the Contracting Agency, the Contractor shall provide evidence of such insurance.

1-07.18(4) Evidence of Insurance

The Contractor shall deliver to the Contracting Agency a Certificate(s) of Insurance and endorsements for each policy of insurance meeting the requirements set forth herein when the Contractor delivers the signed Contract for the work. The certificate and endorsements must conform to the following requirements:

1. An ACORD certificate or a form determined by the Contracting Agency to be equivalent.
2. Copies of all endorsements naming Contracting Agency and all other entities listed in 1-07.18(2) as Additional Insured(s), showing the policy number. The Contractor may submit a copy of any blanket additional insured clause from its policies instead of a separate endorsement. A statement of additional insured status on an ACORD Certificate of Insurance shall not satisfy this requirement.
3. Any other amendatory endorsements to show the coverage required herein.

1-07.18(5) Coverages and Limits

The insurance shall provide the minimum coverages and limits set forth below. Providing coverage in these stated minimum limits shall not be construed to relieve the Contractor from liability in excess of such limits. All deductibles and self-insured retentions must be disclosed and are subject to approval by the Contracting Agency. The cost of any claim payments falling within the deductible shall be the responsibility of the Contractor.

1-07.18(5)A Commercial General Liability

A policy of Commercial General Liability Insurance, including:

Per project aggregate
Premises/Operations Liability
Products/Completed Operations – for a period of one year following final acceptance of the work.
Personal/Advertising Injury
Contractual Liability
Independent Contractors Liability
Stop Gap / Employers’ Liability
Explosion, Collapse, or Underground Property Damage (XCU)
Blasting (only required when the Contractor’s work under this Contract includes exposures to which this specified coverage responds)

Such policy must provide the following minimum limits:

$1,000,000 Each Occurrence
2,000,000 General Aggregate
1,000,000 Products & Completed Operations Aggregate
1,000,000 Personal & Advertising Injury, each offence

Stop Gap / Employers’ Liability
1,000,000 Each Accident
1,000,000 Disease - Policy Limit
1,000,000 Disease - Each Employee

1-07.18(5)B Automobile Liability

Automobile Liability for owned, non-owned, hired, and leased vehicles, with an MCS 90
endorsement and a CA 9948 endorsement attached if “pollutants” are to be transported. Such
policy(ies) must provide the following minimum limit:
1,000,000 combined single limit

1-07.18(5)C Workers’ Compensation

The Contractor shall comply with Workers’ Compensation coverage as required by the Industrial
Insurance laws of the state of Washington.

1-07.23 Public Convenience And Safety

(April 2, 2007)

Work Zone Clear Zone

The Work Zone Clear Zone (WZCZ) applies during working and nonworking hours.
The WZCZ applies only to temporary roadside objects introduced by the Contractor’s
operations and does not apply to preexisting conditions or permanent Work. Those
work operations that are actively in progress shall be in accordance with adopted and
approved Traffic Control Plans, and other contract requirements.

During nonworking hours equipment or materials shall not be within the WZCZ unless
they are protected by permanent guardrail or temporary concrete barrier. The use of
temporary concrete barrier shall be permitted only if the Engineer approves the
installation and location.

During actual hours of work, unless protected as described above, only materials
absolutely necessary to construction shall be within the WZCZ and only construction
vehicles absolutely necessary to construction shall be allowed within the WZCZ or
allowed to stop or park on the shoulder of the roadway.

The Contractor’s nonessential vehicles and employees private vehicles shall not be
permitted to park within the WZCZ at any time unless protected as described above.
Deviation from the above requirements shall not occur unless the Contractor has requested the deviation in writing and the Engineer has provided written approval.

Minimum WZCZ distances are measured from the edge of traveled way and will be determined as follows:

<table>
<thead>
<tr>
<th>Posted Speed</th>
<th>Distance From Traveled Way (Feet)</th>
</tr>
</thead>
<tbody>
<tr>
<td>35 mph or less</td>
<td>10 *</td>
</tr>
<tr>
<td>40 mph</td>
<td>15</td>
</tr>
<tr>
<td>45 to 55 mph</td>
<td>20</td>
</tr>
<tr>
<td>60 mph or greater</td>
<td>30</td>
</tr>
</tbody>
</table>

* or 2-feet beyond the outside edge of sidewalk

Minimum Work Zone Clear Zone Distance

1-07.23(2) Construction And Maintenance Of Detours

Section 1-07.23(2) is supplemented with the following:

(******)

Unless otherwise approved, the Contractor shall maintain two-way traffic during construction. Although brief road closures may be necessary to perform portions of this work, it is the intent of Yakima County to maintain two-way traffic at all times. The Contractor shall request closure a minimum of 2 weeks prior to any proposed closure. If a road closure is authorized, the Contractor shall make provisions for emergency vehicle access on short notice.

When using a flagging operation, the Contractor shall limit delays to motorists passing through the construction site to a maximum of 10 minutes.

Any request by the Contractor for a change in the traffic control plans shall be submitted to the Engineer a minimum of two weeks prior to the desired change date and shall be subject to approval by the Engineer and the Board of County Commissioners. Yakima County will prepare the necessary resolutions and legal notices regarding the road closures at no cost to the Contractor.

1-07.24 Rights Of Way

(October 1, 2005 APWA GSP)

Delete this section in its entirety, and replace it with the following:

Street right of way lines, limits of easements, and limits of construction permits are indicated in the Plans. The Contractor’s construction activities shall be confined within these limits, unless arrangements for use of private property are made.
Generally, the Contracting Agency will have obtained, prior to bid opening, all rights of way and easements, both permanent and temporary, necessary for carrying out the work. Exceptions to this are noted in the Bid Documents or will be brought to the Contractor’s attention by a duly issued Addendum.

Whenever any of the work is accomplished on or through property other than public right of way, the Contractor shall meet and fulfill all covenants and stipulations of any easement agreement obtained by the Contracting Agency from the owner of the private property. Copies of the easement agreements may be included in the Contract Provisions or made available to the Contractor as soon as practical after they have been obtained by the Engineer.

Whenever easements or rights of entry have not been acquired prior to advertising, these areas are so noted in the Plans. The Contractor shall not proceed with any portion of the work in areas where right of way, easements or rights of entry have not been acquired until the Engineer certifies to the Contractor that the right of way or easement is available or that the right of entry has been received. If the Contractor is delayed due to acts of omission on the part of the Contracting Agency in obtaining easements, rights of entry or right of way, the Contractor will be entitled to an extension of time. The Contractor agrees that such delay shall not be a breach of contract.

Each property owner shall be given 48 hours notice prior to entry by the Contractor. This includes entry onto easements and private property where private improvements must be adjusted.

The Contractor shall be responsible for providing, without expense or liability to the Contracting Agency, any additional land and access thereto that the Contractor may desire for temporary construction facilities, storage of materials, or other Contractor needs. However, before using any private property, whether adjoining the work or not, the Contractor shall file with the Engineer a written permission of the private property owner, and, upon vacating the premises, a written release from the property owner of each property disturbed or otherwise interfered with by reasons of construction pursued under this contract. The statement shall be signed by the private property owner, or proper authority acting for the owner of the private property affected, stating that permission has been granted to use the property and all necessary permits have been obtained or, in the case of a release, that the restoration of the property has been satisfactorily accomplished. The statement shall include the parcel number, address, and date of signature. Written releases must be filed with the Engineer before the Completion Date will be established.

**SECTION 1-08, PROSECUTION AND PROGRESS**

1-08 Prosecution and Progress
Add the following new section:

1-08.0 Preliminary Matters
Add the following new section:

1-08.0(1) **Preconstruction Conference**

(May 25, 2006 APWA GSP)

Prior to the Contractor beginning the work, a preconstruction conference will be held between the Contractor, the Engineer and such other interested parties as may be invited. The purpose of the preconstruction conference will be:

1. To review the initial progress schedule;
2. To establish a working understanding among the various parties associated or affected by the work;
3. To establish and review procedures for progress payment, notifications, approvals, submittals, etc.;
4. To establish normal working hours for the work;
5. To review safety standards and traffic control; and
6. To discuss such other related items as may be pertinent to the work.

The Contractor shall prepare and submit at the preconstruction meeting the following:

1. A breakdown of all lump sum items;
2. A preliminary schedule of working drawings submittals; and
3. A list of material sources for approval if applicable.

1-08.4 **Notice to Proceed and Prosecution of the Work**

(October 1, 2005 APWA GSP)

Revise this section to read:

Notice to Proceed will be given after the contract has been executed and the contract bond and evidence of insurance have been approved and filed by the Contracting Agency. The Contractor shall not commence with the work until the Notice to Proceed has been given by the Engineer. The Contractor shall commence construction activities on the project site within ten days of the Notice to Proceed Date, unless otherwise approved in writing. The Contractor shall diligently pursue the work to the physical completion date within the time specified in the contract. Voluntary shutdown or slowing of operations by the Contractor shall not relieve the Contractor of the responsibility to complete the work within the time(s) specified in the contract.

1-08.5 **Time For Completion**

(March 13, 1995)

Section 1-08.5 is supplemented with the following:

The project shall be physically completed in **60 working days**.
1-08.5 Time For Completion
(October 1, 2005 APWA GSP)

Revise the fourth and fifth paragraphs to read:

Contract time shall begin on the first working day following the Notice to Proceed Date. The contract provisions may specify another starting date for contract time, in which case, time will begin on the starting date specified.

Each working day shall be charged to the contract as it occurs, beginning on the day after the Notice to Proceed Date, unless otherwise provided in the Contract Provisions, until the contract work is physically complete. If substantial completion has been granted and all the authorized working days have been used, charging of working days will cease. Each week the Engineer will provide the Contractor a statement that shows the number of working days: (1) charged to the contract the week before; (2) specified for the physical completion of the contract; and (3) remaining for the physical completion of the contract. The statement will also show the nonworking days and any partial or whole day the Engineer declares as unworkable. Within 10 calendar days after the date of each statement, the Contractor shall file a written protest of any alleged discrepancies in it. To be considered by the Engineer, the protest shall be in sufficient detail to enable the Engineer to ascertain the basis and amount of time disputed. By not filing such detailed protest in that period, the Contractor shall be deemed as having accepted the statement as correct. If the Contractor elects to work 10 hours a day and 4 days a week (a 4-10 schedule) and the fifth day of the week in which a 4-10 shift is worked would ordinarily be charged as a working day then the fifth day of that week will be charged as a working day whether or not the Contractor works on that day.

Revise the seventh paragraph to read:

The Engineer will give the Contractor written notice of the completion date of the contract after all the Contractor's obligations under the contract have been performed by the Contractor. The following events must occur before the Completion Date can be established:

1. The physical work on the project must be complete; and
2. The Contractor must furnish all documentation required by the contract and required by law, to allow the Contracting Agency to process final acceptance of the contract. The following documents must be received by the Project Engineer prior to establishing a completion date:
   a. Certified Payrolls (Federal-aid Projects)
   b. Material Acceptance Certification Documents
   d. FHWA 47 (Federal-aid Projects)
e. Final Contract Voucher Certification
f. Property owner releases per Section 1-07.24

SECTION 1-09 MEASUREMENT AND PAYMENT

1-09.2 Weighing Equipment

(August 6, 2001)
General Requirements for Weighing Equipment
Section 1-09.2(1) is revised to read as follows:

Any highway or bridge construction materials to be proportioned or measured and paid for by weight, shall be weighed on scales. These materials include natural, manufactured or processed materials obtained from natural deposits, stockpiles, bunkers, or mixing plants. The Contractor shall provide, set up, and maintain the scales necessary to perform the weighing or shall designate permanently installed, certified commercial scales for the purpose. Each truck to be weighed shall bear a unique identification number. This number shall be legible and in plain view of both the scale operator and the person receiving the material at the jobsite. Scales provided or designated by the Contractor shall be accurate to within one-half of one percent throughout the range of use.

An agent of the scale manufacturer shall test and service any scale before its use at each new site and then at 6-month intervals. The Contractor shall provide the Engineer a copy of the final results after each test.

All initial weighing at the dispatch site or at another site approved by the Engineer shall be performed by a Contractor employee or by another person designated by the Contractor. The designated weigher shall prepare a weigh or load ticket to accompany each load. Each ticket shall contain the truck identification number, the date and time of weighing the load, a description of the material being weighed and the signature or initials of the weigher.

Each weigh or load ticket shall also contain a determination of the net weight of the load. This shall be a reading from any device which weighs as material is loaded or a calculation including gross weight and tare weight when the method of loading does not include weighing. It shall also identify the weighed material. When used, tare weights shall be taken of each hauling vehicle at least twice a day. The ticket shall be provided to the inspector at the jobsite immediately after the material is delivered.

Except as noted below, all weighing shall be subject to confirmation testing through random checks made with a separate scale. The secondary scale shall be described in the contract provisions, either as a designated independent commercial scale or as a platform scale installed by the Contractor at a location named in the provisions. The inspector will select loaded trucks at random and weigh them with the secondary scale. The same trucks will be weighed empty when the tested load has been delivered. The
frequency of confirmation testing will be such that at least one test is performed for each contract item paid by weight for each $50,000 of payment for that item and at least one test weekly for each weighed contract item performed during that week.

Confirmation testing will not be routinely conducted for small quantities of weighed material. A small quantity shall be defined as one whose estimated proposal quantity, multiplied by its unit price, has a value of less than $20,000. The inspector may choose to apply confirmation testing to a minor quantity item if, in the inspector’s judgment, there is reason to suspect that the ticket weight might be incorrect.

1-09.6 Force Account

(October 1, 2005 APWA GSP; may be used on FHWA-funded projects)

Supplement this Section with the following:

Owner has estimated and included in the Proposal, dollar amounts for all items to be paid per force account, only to provide a common proposal for Bidders. All such dollar amounts are to become a part of Contractor’s total bid. However, Owner does not warrant expressly or by implication, that the actual amount of work will correspond with those estimates. Payment will be made on the basis of the amount of work actually authorized by the Engineer.

1-09.8 Payment For Material On Hand

(April 28, 1997)

The last paragraph of Section 1-09.8 is revised to read:

The Contracting Agency will not pay for any individual item on hand with a cost of less than $2,000. As materials are used in the work, credits equaling the partial payments for them will be taken on future estimates. Each month, no later than the estimate due date, the Contractor shall submit a letter to the Project Engineer that clearly states: 1) the amount originally paid on the invoice (or other record of production cost) for the items on hand, 2) the dollar amount of the material incorporated into each of the various work items for the month, and 3) the amount that should be retained in material on hand items. If work is performed on the items and the Contractor does not submit a letter, all of the previous material on hand payment will be deducted on the estimate. Partial payment for materials on hand shall not constitute acceptance. Any material will be rejected if found to be faulty even if partial payment for it has been made.

1-09.13(3) Claims $250,000 or Less

(October 1, 2005 APWA GSP; may be used on FHWA-funded projects)

Delete this Section and replace it with the following:

The Contractor and the Contracting Agency mutually agree that those claims that total $250,000 or less, submitted in accordance with Section 1-09.11 and not resolved by
nonbinding ADR processes, shall be resolved through litigation unless the parties mutually
agree in writing to resolve the claim through binding arbitration.

1-09.13(3)A Administration of Arbitration
(October 1, 2005 APWA GSP)

Revise the third paragraph to read:

The Contracting Agency and the Contractor mutually agree to be bound by the decision of
the arbitrator, and judgment upon the award rendered by the arbitrator may be entered in the
Superior Court of the county in which the Contracting Agency's headquarters are located.
The decision of the arbitrator and the specific basis for the decision shall be in writing. The
arbitrator shall use the contract as a basis for decisions.

SECTION 1-10, TEMPORARY TRAFFIC CONTROL

1-10.2 Traffic Control Management

1-10.2(1) General

(August 2, 2004)
Section 1-10.2(1) is supplemented with the following:

The Traffic Control Supervisor shall be certified by one of the following:

The Northwest Laborers-Employers Training Trust
27055 Ohio Ave.
Kingston, WA 98346
(360) 297-3035

Evergreen Safety Council
401 Pontius Ave. N.
Seattle, WA 98109
1-800-521-0778 or
(206) 382-4090

1-10.4 Measurement

Paragraph three of Section 1-10.4(2), supplemented with the following:

(******)
Flaggers and Spotters will be by the hour for each person actually performing the work
described in Section 1-10.3(1)A. Portions of an hour will be rounded up to the one half
hour.
DIVISION 2
EARTHWORK

SECTION 2-01 CLEARING, GRUBBING, AND ROADSIDE CLEANUP

2-01.1 Description

Section 2-01.1 is supplemented with the following:

(March 13, 1995)
Clearing and grubbing on this project shall be performed within the following limits:

The Contractor shall clear and grub as staked unless otherwise directed by the Engineer.
The Contractor shall remove and dispose of all existing shrubs, trees, etc., whether or not
they are shown on the plans. Those areas identified on the Plans as having construction
easements shall only be cleared as needed for improvements.

(******)
As part of a right-of-way acquisition agreement on Edison Road, W., Yakima County is
obligated to salvage for the property owner, the wood from trees removed adjacent to Parcel
No. 221026-23004. Therefore, the Contractor shall salvage for firewood, all tree
components (except roots) 4 inches in diameter and larger, from Sta. 4+60+/- Lt. to Sta.
6+20+/- Lt.

The Contractor shall sawcut the tree limbs into approximate 16” lengths, or longer if
approved by the property owner, and shall stack them at a location on the property selected
by the owner. For specific location information, please contact the owner, Mr. Kirk
Killingstad, 1130 W. Edison Rd., Sunnyside, WA (Tel. 509-837-6244).

All remaining tree components and debris not salvaged for firewood shall be removed from
the project site pursuant to Section 2-01.

2-01.2(1) Disposal Method No. 1 –Open Burning

Section 2-01.2(1) is deleted and replaced with the following:

(******)
No open burning will be allowed on this project.

2-01.2(3) Disposal Method No. 3 –Chipping

Section 2-01.2(3) is deleted and replaced with the following:

(******)
Chipping shall be done by machines that can grind debris into wood chips. Wood chips to
be sold or disposed of outside of this project may be any size. Wood chips to be used within
the project site shall be no larger than 6 square inches and no thicker than 1/2-inch. The
Contractor may spread the unsold chips evenly on the fill slopes only, and tractor walk them into the ground to the satisfaction of the Engineer.

2-01.3(2) Grubbing

Section 2-01.3(2) is supplemented with the following:

(******)
Edison Road, W. from Sta. 3+30 Lt. to Sta. 6+25 Lt., the Contractor shall grub all existing tree stumps and new tree stumps from clearing, roots, buried logs, and other vegetative matter within the right-of-way, a minimum of 3 feet below the finished grade.

2-01.4 Measurement

Section 2-01.4 is supplemented with the following:

(******)
No unit of measurement shall apply to the lump sum price for clearing and grubbing, including the salvaging for firewood all tree components larger than 4 inches in diameter, from Sta. 4+60+/- Lt. to Sta. 6+20+/- Lt.

2-01.5 Payment

Section 2-01.5 is revised as follows:

(******)
There shall be no payment for roadside cleanup. Any work performed for roadside cleanup shall be incidental to the Bid Item "Clearing and Grubbing" per Lump Sum, and no further payment shall be made.

(******)
The Lump Sum payment for Clearing and Grubbing shall include all costs to clear and grub to the limits staked by the Engineer, including salvaging for firewood all tree components 4 inches in diameter and larger as discussed above, and for grubbing all existing tree stumps and new tree stumps within the right-of-way from near Edison Road, W. Sta. 4+60+/- Lt. to Sta. 6+20+/- Lt., and no further payment shall be made.

SECTION 2-02 REMOVAL OF STRUCTURES AND OBSTRUCTIONS

2-02.1 General

(September 30, 1996)
The Contractor is advised that asbestos may be present on this project.

(******)
The Contractor shall have a "Good Faith Survey" performed by a certified AHERA building inspector to check for asbestos materials prior to beginning demolition of the block structure. Cost for removal and disposal will be as provided in Section 1-04.7.
The following are specific requirements that shall be met for this phase of the project:

A. Contractor shall file a “Notice of Demolition and Renovation” with Yakima County Clean Air Authority. Copy shall be provided to Yakima County.

B. The Contractor shall obtain a Demolition Permit as required by Yakima County Public Services Building and Fire Division.

2-02.3 Construction Requirements

Section 2-02.3 of the Standard Specifications is supplemented with the following:

(September 30, 1996)

Asbestos Handling And Disposal

Prior to and during, the performance of any contract work, the Contractor shall verify that no asbestos containing materials are involved or will be disturbed. When asbestos is encountered, the Contractor shall be responsible for obtaining all permits from, and provide notification to, the Washington State Department of Labor and Industries, the U.S. EPA, the local air pollution control agency, and other permitting and regulatory agencies with jurisdiction over the work involving asbestos as the law requires.

Prior to commencing asbestos related work, the Contractor shall provide the Engineer with written verification of approvals and notifications that have been given and/or obtained from the required jurisdictional agencies, and the Contractor’s schedule for all work involving asbestos removal. The schedule shall include the sequencing and scheduling of asbestos related work, and coordination with subcontractors. The Contractor shall notify the Engineer when all approvals have been received and notifications have been made, as required by the agencies involved.

The Contractor shall ensure the safety of all workers, visitors to the site, and the general public in accordance with all applicable laws, rules, and regulations.

The Contractor shall designate a Washington State Certified Asbestos Supervisor (CAS) to personally supervise the asbestos removal and to ensure that the handling and removal of asbestos is accomplished by certified asbestos workers, pursuant to Washington State Department of Labor and Industries standards. The Contractor shall ensure that the removal and disposal of asbestos meets the requirements of EPA regulations 40 CFR Part 61, local health department regulations, and all other applicable regulations.

(*****)

Prior to beginning any demolition or removal, the Contractor shall verify that all public utilities have been located and disconnected from the structures. The Contractor shall notify the Engineer immediately if any utilities are found to be connected.
(February 17, 1998)

Removal of Obstructions

The following items shall be removed, disposed of or reset as directed by the Engineer in accordance with the requirements of Section 2-02 of the Standard Specifications:

Woodin Road, W.

1. Remove existing driveway culvert Sta. 12+05, Rt.
2. Remove existing 12" Conc. cross culvert Sta. 17+70.
3. Remove existing driveway culvert Sta. 20+40, Lt.
4. Remove existing driveway culvert Sta. 20+80, Rt.
5. Remove existing driveway culvert Sta. 21+55, Rt.
6. Remove existing 3" PVC irrigation Line.
7. Remove large wood poles for driveway entrance Sta. 24+85, Rt.
8. Remove portion of existing 10" drain and stand pipe to install new drain pipe Sta. 30+90, Lt.
9. Remove wood fencing Sta. 35+50 Rt. to 36+50 Rt.
10. Remove existing Concrete block structure Sta. 36+95, Rt. Building and footings to be removed, well to remain.
11. Remove wood fence Sta. 46+50, Rt. to Sta. 46+55, Rt.
12. Saw cut and remove portion of sidewalk as needed Sta. 47+05 Rt.
13. Remove wood fence Sta. 55+60, Rt.
14. Remove existing driveway culvert Sta. 57+25, Rt.
15. Remove existing driveway culvert Sta. 58+25, Rt.
16. Remove existing 36" conc. standpipe Sta. 60+45, Rt.
17. Remove existing conc. irrigation structure Sta. 61+60, Rt.

Edison Road, W.

1. Remove existing driveway culvert Sta. 4+50, Lt.
2. Remove existing 4" PVC irrigation pipe Sta. 3+25 Lt. to Sta. 6+10, Lt.
3. Remove existing wood fencing if not relocated by others Sta. 8+25, Lt. to Sta. 10+25 Lt. and Sta. 12+00, Lt. to Sta. 12+40, Lt.

All other items encountered, which are not covered by Section 2-01 of the Standard Specifications (Clearing, Grubbing, and Roadside Cleanup) shall be considered incidental to the bid item “Removal of Structures and Obstructions”.

2-02.5 Payment

Section 2-02.3 of the Standard Specifications is supplemented with the following:

(September 30, 1996)

Payment for asbestos removal, handling, disposal, cost of permits, and all other work will be as provided in Section 1-04.7, unless such work is explicitly included as a part of another pay item in the contract.
The Contract Unit Price for "Asbestos Inspection" per Lump Sum, shall be full compensation for all labor, tools, and testing necessary to prepare the Good Faith Survey asbestos inspection report.

SECTION 2-03 ROADWAY EXCAVATION AND EMBANKMENT

2-03.1 Description

Section 2-03.1 of the Standard Specifications is deleted and replaced with the following:

(******)

Any material hauled from the project will be subject to the requirements of the Yakima County Excavation and Grading Ordinance. All sites shall be approved by the Engineer prior to use to ensure compliance with the Excavation and Grading Ordinance and SEPA compliance. All costs incurred by the Contractor to obtain a Grading Permit shall be included in the various Unit Bid Prices, and no further Payment shall be made.

The Yakima County Excavation and Grading Ordinance may be reviewed in the County Engineer's Office, 4th Floor, Yakima County Courthouse.

2-03.3(14) Embankment Construction

2-03.3(14)C Compacting Earth Embankments

Compacting embankments and excavations shall be by Method "C" as specified under Section 2-03.3(14)C of the Standard Specifications.

2-03.4 Measurement

Section 2-03.4 of the Standard Specifications is deleted and replaced with the following:

(******)

Only one determination of the original ground elevations shall be made on this project. Measurement for roadway excavation and embankment shall be based on the original ground elevations recorded previous to the award of this Contract and the alignment, profile, grade, and roadway section as shown on the plans and as staked by the Engineer. Control stakes shall be set during construction to provide the Contractor with all essential information for the construction of excavation and embankments.

If discrepancies are discovered in the ground elevations which will materially effect the quantities of earthwork, the original computations of earthwork shall be adjusted accordingly.

Earthwork quantities shall be computed either manually or by means of electronic data processing equipment, by use of the average end area method.
Copies of the ground cross-section notes shall be available for the bidder's inspection, before
the opening of bids, at the office of the County Engineer. Upon award of the Contract,
copies of the original ground cross-sections shall be furnished to the successful bidder on
request to the County Engineer.

2-03.5 Payment

Section 2-03.5 of the Standard Specifications is deleted and replaced with the following:

(******)
The Contract Unit Price for "Roadway Excavation Incl. Haul," per Cubic Yard, shall be full
compensation for all labor, equipment, tools, and materials necessary to excavate, load, haul,
place, compact, shape, or otherwise dispose of the materials including existing hot mix
asphalt pavements, and any other work required to complete this item as specified and no
further payment shall be made.

No separate payment shall be made for embankment compaction and all costs to perform
this work as required shall be included in the Unit Bid Price per Cubic Yard for "Roadway
Excavation Incl. Haul."

SECTION 2-07 WATERING

Section 2-07 is deleted and replaced with the following:

(******)
The Contractor shall be solely responsible for dust control on this project and shall
protect the motoring public, adjacent homes, orchards and crops from damage due to
dust, by whatever means necessary. The Contractor shall be responsible for any claims
for damages and shall protect the County from any and all such claims.

When directed by the Engineer, the Contractor shall provide water for dust control within
two hours of such order and have equipment and manpower available at all times
including weekends and holidays to respond to orders for dust control measures.

If County forces are required to respond to a dust control problem, the Contractor shall
be charged liquidated damages to offset County expenditures. For each time that the
County is required to provide dust control measures, the Contractor shall be assessed
damages in the amount of $500.00, which shall be deducted from any moneys due the
Contractor under this contract.

Payment for water used for dust control, compaction, processing of base course and top
course, and other work shall be included in the other Bid Items involved, and no further
payment shall be made.

SECTION 2-09 STRUCTURE EXCAVATION
2-09.4 Measurement

Section 2-09.4 of the Standard Specification shall be supplemented with the following:

(******)
Structure Excavation Class B for storm sewers and culverts shall not be measured for payment.

2-09.5 Payment

Section 2-09.5 of the Standard Specification shall be supplemented with the following:

(******)
There shall be no separate payment for Structure Excavation Class B. All costs associated with excavation backfill and compaction of new storm sewer, and culvert trenches shall be included in the linear foot price of the pipe or concrete box culvert.

SECTION 2-10, DITCH EXCAVATION

2-10.1 Description

Section 2-10.1 is supplemented with the following:

(******)
The cutoff trench shall consist of excavating the trench as detailed in the plans, disposing of the excavated material, placing geotextile fabric, and backfilling the trench with gravel backfill for drywells.

2-10.2 Materials New Section

(******)
The crushed rock backfill shall meet the requirements of Gravel Backfill for Drywells per Section 9-03.12(5). The underground drainage geotextile fabric used to enclose the trench shall meet the requirements of Section 9-33. for Underground Drainage, Moderate Survivability, Class B.

2-10.3 Construction Requirements

Section 2-10.3 is supplemented with the following:

(******)
The cutoff trench shall be completely encased in geotextile fabric for Underground Drainage, Moderate Survivability, Class B, in accordance with the plans and with Section 2-12.1.

2-10.4 Measurement

Section 2-10.4 is supplemented with the following:

(******)
The cutoff trench shall not be measured.
2-10.5 Payment

Section 2-10.5 is supplemented with the following:

(*****)
The cutoff trench shall be paid by the bid item "Gravel Backfill for Drywells" per Ton and include all costs for excavation, supplying and placing construction geotextile, gravel backfill for drywells, haul, labor, equipment and all other costs necessary to complete the item as specified and no further payment shall be made.

3-01 PRODUCTION FROM QUARRY AND PIT SITES

3-01.3 County Furnished Material Sources,

Section 3-01.3 of the Standard Specifications shall be supplemented with the following:

(*****)
The Contractor shall use County Supplied Rock for the BST.

(*****)
Alternate A
If the Contractor bids the contract using Alternate A, County Supplied Crushed Surfacing Materials, then the following shall apply.

If County-owned Crushed Rock is used on this project, then the provisions of WAC 458-20-178 shall apply.

(*****)
The following source of stockpiled materials is made available at no cost to the Contractor:

Yakima County shall make available to the Contractor Aggregate From Stockpile for BST (5/8 Minus US No. 4) and Crushed Surfacing Base Course located at Yakima County's Liberty Pit, for these projects. Liberty Pit is located in the E 1/2 of the NE 1/4 of Section 23, Township 11 North, Range 21 East, W.M., approximately 7 road miles north of the Woodin Road, W. project and 8 road miles north of the Edison Road, W. Project.

No source is being provided for any of the other materials necessary for the construction of this project. The Contractor shall make arrangements to obtain the necessary materials and all costs of acquiring, producing, and placing these materials in the finished work shall be included in the Unit Contract Prices for the various items involved.

3-01.4 Contractor Furnished Material Sources, Alternate B

Section 3-01.4 of the Standard Specifications is supplemented with the following:
If the Contractor bids the contract using Contractor Supplied Crushed Surfacing Materials, then the following shall apply.

If the sources of materials provided by the Contractor necessitate hauling over roads other than County roads, the Contractor shall, at his own expense, make all arrangements for the use of the haul routes.

DIVISION 5
SURFACE TREATMENTS AND PAVEMENTS

SECTION 5-04 HOT MIX ASPHALT

5-04.3 Construction Requirements

5-04.3(8) Mixing

5-04.3(8)A Acceptance Sampling and Testing

Section 5-04.3(8)A of the Standard Specifications shall be deleted.

5-04.3(10) Compaction

5-04.3(10)B Control

(*******)

The first paragraph of Section 5-04.3(10)B of the Standard Specifications is deleted and replaced with the following:

HMA used in traffic lanes, including lanes for ramps, truck climbing, weaving, and speed change, and having specified compacted course thickness greater than 0.10 foot, shall be compacted to a specified level relative density. The specified level of relative density shall be a minimum of 91.0 percent of the reference maximum density as determined by WSDOT for AASHTO T 209. The reference maximum density shall be determined as the moving average of the most recent five determinations for the lot of asphalt concrete being placed. The specified level of density attained will be determined by five nuclear gauge tests taken in accordance with WAQTC FOP TM8 and WSDOT SOP T 729 on the day the mix is placed (after completion of the finish rolling) at locations determined by the stratified random sampling procedure conforming to WSDOT Test Method 716 within each density lot. The quantity represented by each density lot will be no greater than a single day’s production or approximately 400 tons, whichever is less. The Engineer will furnish the Contractor with a copy of the results of all acceptance testing performed in the field by 7:00 a.m. the morning of the next workday after testing, of for nighttime work within four hours after the beginning of the next paving shift.

The last paragraph of Section 5-04.3(10)B of the Standard Specifications is deleted and replaced with the following:
In addition to the randomly selected locations for tests of density, the Engineer may also isolate from a normal lot any area that is suspected of being defective in relative density. Such isolated material will not include an original sample location. A minimum of 5 randomly located density tests will be taken. The isolated area then will be evaluated for price adjustment in accordance with the price reduction formula in the Special Provisions, considering it as a separate lot.

Control lots not meeting the minimum density standard shall be removed and replaced with satisfactory material. At the option of the Engineer, noncomplying material may be accepted at reduced price as computed below.

**FACTORS INVOLVED:**

**Quantity of HMA involved** (from Compaction Control Report)

**Percent compaction** (from Compaction Control Report)

**Pay adjustment factor** (see table below)

**Liquid asphalt used** = Percent liquid asphalt from "Amount Ordered" or "Calculated from Production" (whichever is less) from Daily Report of Asphalt Plant Operations (when producing from a commercial plant, always use the "Amount Ordered")

**Price liquid asphalt** = Invoice price f.o.b. job site (if invoice unavailable then use average monthly refinery price.)

**Unit Contract Price** (from Contract Proposal)

**CALCULATION PROCEDURE:**

Equations:  
\[ PA = Q \times AUCP \times PAF \]
\[ AUCP = UCP - VLA \]
\[ VLA = PLA \times RLAU \]
\[ RLAU = LAU/100 \]

**PA** = Price adjustment

**UCPA** = Unit contract price adjustment

**Q** = Quantity HMA involved

**AUCP** = Adjusted unit contract price

**PAF** = Pay adjustment factor

**UCP** = Unit contract price

**VLA** = Value liquid asphalt

**PLA** = Price liquid asphalt

**RLAU** = Rate liquid asphalt used
LAU = Liquid asphalt used

EXAMPLE:

Q = 200 tons
Percent compaction = 90.5
LAU = 5.0%
UCP = $25.00/ton
PLA = $200.00/ton f.o.b. job site
PAF = 0.05
RLAU = LAU/100
= 5.0/100
RLAU = 0.05 ton/ton
VLA = PLA x RLAU
= $200.00/ton x 0.05 ton/ton
VLA = $10.00/ton

AUCP = UCP - VLA
= $25.00/ton - $10.00/ton
AUCP = $15.00/ton

PA = Q x AUCP x PAF
= 200 ton x $15.00/ton x 0.05
PA = $150.00

UCPA = PA/Q
= $150.00/200 ton
UCPA = $0.75/ton

PAY ADJUSTMENT FACTOR

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5-04.3(15) HMA Road Approaches

Section 5-04.3(15) is supplemented with the following:

(******)
Where asphalt driveways are shown on the plans, asphalt driveways (road approaches) shall be constructed with 0.40 foot (compacted depth) of crushed surfacing and 0.20 foot (compacted depth) of Hot Mix Asphalt (HMA) for Approach. The portion of the driveways not paved with asphalt shall be surfaced with 0.30 foot (compacted depth) crushed surfacing top course.
Grades from the edge of pavement to existing driveways (road approaches) shall be constructed to provide safe ingress and egress and shall be constructed of materials in kind as shown on the plans.

Any portion of the existing driveway (road approach) beyond the construction limits that is damaged by the Contractor’s operations shall be replaced in kind at his expense to the satisfaction of the Engineer.

SAWCutting Pavement

All transitions to existing asphalt concrete and cement concrete driveways, curb, asphalt thickened edge for gutter, and walkways shall be vertically sawcut at least two (2) inches with straight, uniform edges. Existing asphalt pavement may be cut with a wheel, provided the wheel cut is full depth and no damage occurs to the pavement which is to remain. No impact tools or pavement breakers can be used for trench crossings of existing pavement. Trench crossing of existing pavement shall be vertically sawcut as directed by the Engineer.

5-04.4 Measurement

(*****)
Measurement for driveway (road approach) reconstruction shall be by the various Bid Items involved in the work, “HMA for Approach”, per Ton, “Crushed Surfacing Top Course” per Ton, “Crushed Surfacing Base Course” per Ton, and "Roadway Excavation Incl. Haul" per Cubic Yard.

5-04.5 Payment

Section 5-04.5 is supplemented with the following:

(*****)
There is no Bid Item "Saw Cutting Asphalt Pavement" or "Saw Cutting Cement Concrete Sidewalk" for this project. All costs associated with the cutting, labor, equipment, etc., or any other costs associated with cutting the existing asphalt or concrete pavement shall be considered incidental to the other Contract Bid Items, and no further payment shall be made.

(*****)
Payment for driveway (road approach) reconstruction shall be by the various Bid Items involved in the work, “HMA for Approach”, per Ton, “Crushed Surfacing Top Course” per Ton, “Crushed Surfacing Base Course” per Ton, and "Roadway Excavation Incl. Haul" per Cubic Yard, and shall include all costs associated with labor, materials, haul etc. to complete the Item as specified, and no further payment shall be made.
5-04.5(1) Quality Assurance Price Adjustments

Section 5-04.5(1) shall be deleted.

5-04.5(1) A Price Adjustment for Quality of HMA

Section 5-04.5(1)A shall be deleted.

5-04.5(1) B Price Adjustment for Quality of HMA Compaction

Section 5-04.5(1)B shall be deleted.

SECTION 5-05 CEMENT CONCRETE PAVEMENT

5-05.3(1) Concrete Mix Design for Paving

(April 3, 2006)

3. Mix Design Modifications. The Contractor may initiate minor adjustments to the approved mix proportions. The combined aggregate gradation may be adjusted provided it remains with the specifications limits detailed above. The mix design will not be required to be resubmitted as long as the water cementitious ratio does not change.

Utilizing admixtures to accelerate the set or to increase workability will be permitted only when approved by the Engineer. Only non-chloride accelerating admixtures that meet the requirements of Section 9-23.6 Admixture for Concrete, shall be used.

The Contractor shall notify the Engineer in writing of any proposed modification. A new mix design will designate a new lot.

DIVISION 6
STRUCTURES

SECTION 6-02 CONCRETE STRUCTURES

6-02.3(2)A Contractor Mix Design

Section 6-02.3(2)A of the Standard Specifications shall be amended as follows:

The first sentence of the first paragraph of Section 6-02.3(2)A is revised to read as follows:

(*****)

The Contractor shall provide a mix design in writing for all classes of concrete.

6-02.3(2)B Commercial Concrete
Section 6-02.3(2)B of the Standard Specifications shall be amended as follows:

(******)
The third sentence of the first paragraph is deleted and replaced with the following:

Commercial concrete requires plant approval, mix design, source approvals for cement, aggregate, and other admixtures.

(******)
In the first sentence of the second paragraph, the terms “luminaire bases, sidewalks, curbs, and gutters,” shall be deleted.

6-02.3(4) Ready-Mix Concrete

Section 6-02.3(4) of the Standard Specifications shall be amended as follows:

(******)
The first sentence of Section 6-02.3(4) is revised to read as follows:

All concrete, including commercial concrete and lean concrete, shall be batched in a prequalified manual, semi-automatic, or automatic plant as described in Section 6-02.3(4)A.

6-02.3(4)B Jobsite Mixing

Section 6-02.3(4)B of the Standard Specifications shall be amended as follows:

(******)
The first sentence of Section 6-02.3(4) is revised to read as follows:

For small quantities of concrete, less than ½ cubic yard, the Contractor may mix concrete on the job site, provided the Contractor has requested in writing and received written permission from the Engineer.

6-02.3(5) Acceptance of Concrete

6-02.3(5)A General

The first sentence of Section 6-02.3(5)A is hereby deleted and replaced with the following:

(******)
Lean concrete will be accepted based on a Certificate of Compliance to be provided by the Supplier as described in Section 6-02.3(5)B.

DIVISION 7
DRAINAGE STRUCTURES, STORM SEWERS, SANITARY SEWERS, WATER MAINS, AND CONDUITS
SECTION 7-01 DRAINS

7-01.4 Measurement

Section 7-01.4 is supplemented with the following:

(******)
Gravel Backfill for Drywells shall be measured per Ton.

7-01.5 Payment

Section 7-01.5 is supplemented with the following:

(******)
The Unit Contract Price for "Gravel Backfill for Drywells" per Ton, shall be full compensation for all labor, equipment, tools, and materials necessary to supply, excavate, load, haul, compact, furnish and place geotextile fabric, and any other work required to complete the item as specified and no further payment will be made.

SECTION 7-02 CULVERTS

7-02.2 Materials

Section 7-02.2 is supplemented with the following:

(******)
Solid Wall PVC Culvert Pipe, Profile Wall PVC Culvert Pipe, and Corrugated Polyethylene Culvert Pipe shall not be allowed for use on driveway approaches or road crossings with exposed ends.

The "Gravel Backfill for Pipe Bedding and Trench" shall conform to Crushed Surfacing Top Course meeting the requirements of Section 9-03.9(3) of the Standard Specifications.

7-02.3 Construction Requirements

Section 7-02.3 is supplemented with the following:

(******)
All pipes, which extend into the slope shall have beveled ends to match the ground slope. On field cuts, the cut surface shall be painted with two coats of paint. The steel pipe to be painted shall be cleaned with solvent to remove contaminants. After cleaning, the pipe shall be painted with two coats of paint conforming to Federal Specifications TT-P-645 (Primer, Paint, Zinc Chromate, Alkyd Vehicle).

The cost of cutting, cleaning and painting the steel pipe surfaces as specified shall be included in the unit contract price per linear foot for steel pipe.
The culvert pipe 36 In. Diam. shall be pushed through the existing 48 In. Diam. CMP culvert pipe located at Woodin Road, W. Sta. 54+95. The void between the existing pipe and the new pipe shall be filled with lean concrete. The Catch Basin Type 2 60 In. Diam., and connection to the existing 24 In. Diam. conc. culvert pipe can be installed in November, 2007, after the irrigation water is shut off. The Contractor shall stock pile material from the initial road construction work to fill the area around the catch basin to grade once the 60 In. Diam catch basin and the 24 In. Diam. culvert pipe have been installed. It is anticipated that there will still be some water flowing in the SVID irrigation drain after the irrigation is shut off.

7-02.5 Payment

Section 7-02.5 of the Standard Specifications shall be supplemented with the following:

(******)
When the Engineer directs the Contractor to backfill trenches with "Gravel Backfill for Pipe Bedding and Trench", payment shall be made by the Contract Bid Item "Gravel Backfill for Pipe Bedding and Trench" per Ton, which shall include all costs associated with labor, equipment, materials, etc, and no further payment shall be made.

SECTION 7-04 STORM SEwers

7-04.2 Materials

Section 7-04.2 of the Standard Specifications shall be supplemented with the following:

(******)
The "Gravel Backfill for Pipe Bedding and Trench" shall conform to Crushed Surfacing Top Course meeting the requirements of Section 9-03.9(3) of the Standard Specifications.

7-04.3 Construction Requirements

Section 7-04.3 of the Standard Specifications is supplemented with the following:

(******)
When directed by the Engineer, street crossing trenches and other locations shall be backfilled as to the depth specified by the Engineer with "Gravel Backfill for Pipe Bedding and Trench".

Section 7-04.3(1)E is deleted

Section 7-04.3(1)F is deleted

7-04.5 Payment

Section 7-04.5 of the Standard Specifications is supplemented with the following:

(******)
All pipefittings including elbows, tees, gaskets, bands, etc., are considered incidental to individual pipe Bid Items involved, and no further payment shall be made.

(******)
There shall be no separate measurement and payment for excavation, backfill, and compaction. All costs associated with excavation and backfill of new pipeline trenches, including cutting and removal of existing surfacing, shall be included in the various pipe installation bid items.

(******)
When the Engineer directs the Contractor to backfill trenches with "Gravel Backfill for Pipe Bedding and Trench", payment shall be made by the Contract Bid Item "Gravel Backfill for Pipe Bedding and Trench" per Ton, which shall include all costs associated with labor, equipment, materials, etc, and no further payment shall be made.

SECTION 7-05 MANHOLES, INLETS, CATCH BASINS, AND DRYWELLS

7-05.3 Construction Requirements

Section 7-05.3 of the Standard Specifications is supplemented with the following:

(******)
The Drywell Infiltration Trench shall be constructed per the detail in the plans. The Drywell Infiltration Trench shall be completely encased in "Moderate Survivability" Class B underground drainage geotextile in accordance with the plans and with Section 2-12 and Section 9-33 of the Standard Specifications. The drain rock shall meet the requirements of Gravel Backfill for Drywells in Section 9-03.12(5) of the Standard Specifications.

7-05.3(1) Adjusting Manholes and Catch Basins to Grade

Section 7-05.3.1 of the Standard Specifications is supplemented with the following:

(******)
If in the opinion of the Engineer, the manhole ring and lid are damaged due to the actions of the Contractor prior to and during the grade adjustment process and are not useable, the Contractor shall replace it with a new manhole frame and grate of the same size and type at no cost to Yakima County.

7-05.5 Payment

Section 7-05.5 of the Standard Specifications is supplemented with the following:

(******)
The Unit Contract Price for "Catch Basin Type 2 48 In. Diam." per Each, shall be full compensation for all labor, equipment, tools, and materials necessary to excavate, load, haul, compact, supply and place Catch Basin Type 2 48 In. Diam., Tee, and any other work
required to complete the item as detailed in the plans and contract documents and no further payment will be made.

(*****)
The Unit Contract Price for "Gravel Backfill for Drywells" per Ton, shall be full compensation for all labor, equipment, tools, and materials necessary to supply, excavate, load, haul, compact, furnish and place geotextile fabric, and any other work required to complete the item as specified and no further payment will be made.

(*****)
No separate payment shall be made for supplying and placing the underground drainage geotextile fabric and all costs to perform this work as required shall be included in the unit bid price per ton for "Gravel Backfill for Drywells".

SECTION 7-08 GENERAL PIPE INSTALLATION REQUIREMENTS

7-08.2 Materials

Section 7-08.2 is supplemented with the following:

(*****)
Gravel Backfill for Pipe Bedding and Trench 9-03.9(3).

7-08.3(3) Backfilling

Section 7-08.3(3) is supplemented with the following:

(*****)
Where directed by the Engineer, trenches shall be backfilled to the depth specified by the Engineer with "Gravel Backfill for Pipe Bedding and Trench".

7-08.4 Measurement

Section 7-08.4 is supplemented with the following:

(*****)
"Gravel Backfill for Pipe Bedding and Trench" shall be measured by the Ton.

The first sentence of paragraph 4 is deleted and replaced with the following:

Structure Excavation Class B, and Structure Excavation Class B, including haul shall not be measured.

7-08.5 Payment
Section 7-08.5 is supplemented with the following:

(*****)

When the Engineer directs the Contractor to backfill trenches with "Gravel Backfill for Pipe Bedding and Trench" payment shall be made by the Contract Bid Item "Gravel Backfill for Pipe Bedding and Trench" per Ton, which shall include all costs associated with labor, equipment, materials, etc., and no further payment shall be made.

All costs associated with Structure Excavation Class B, and Structure Excavation Class B, Including Haul for the various drainage items shall be included in the unit contract price for the type and size of pipe or catch basin installed.

DIVISION 8
MISCELLANEOUS CONSTRUCTION

SECTION 8-02 ROADSIDE RESTORATION

8-02.3(15) B Seeding and Fertilizing
Section 8-03.3(15) B of the Standard Specifications is supplemented with the following:

(*****)

Grass seed, of the following composition, proportion, and quality, shall be applied at the rate of 52 pounds per acre on all areas requiring seeding within the project:

<table>
<thead>
<tr>
<th>Grass Species</th>
<th>Scientific Name</th>
<th>Pounds per Acre</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sandburg Bluegrass</td>
<td>Poa sandbergii</td>
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</tr>
<tr>
<td>Bluebunch Wheatgrass</td>
<td>Agropyron spicatum</td>
<td>4</td>
</tr>
<tr>
<td>Indian Ricegrass</td>
<td>Oryzopsis hymenoides</td>
<td>2</td>
</tr>
<tr>
<td>Basin Wild Rye</td>
<td>Elymus cinereus</td>
<td>4</td>
</tr>
<tr>
<td>Annual Rye</td>
<td>Lolium multiforum</td>
<td>40</td>
</tr>
</tbody>
</table>

Total Pounds per Acre

010304B1.FR8
(January 5, 1998)

Sufficient quantities of fertilizer shall be applied to supply the following amounts of nutrients:

Total Nitrogen as N - 80 pounds per acre

Available Phosphoric Acid as P$_2$O$_5$ - 40 pounds per acre
Soluble Potash as $\text{K}_2\text{O} - 40$ pounds per acre

Ninety percent of nitrogen applied per acre shall be derived from isobutylidene diurea (IBDU),
cyclo-di-urea (CDU), or sulfur-coated urea (SCU). The remainder may be derived from any
source.

The fertilizer formulation and application rate shall be approved by the Engineer before use.

8-02.3(15)D Mulching

Section 8-01.3(5) of the Standard Specifications is supplemented with the following:

(******)
Wood cellulose fiber mulch shall be applied at a rate of 2,000 pounds per acre.

8-02.3(15)F Soil Binder or Tacking Agent

Section 8-01.3(6)B of the Standard Specifications is supplemented with the following:

(******)
Tacking agent shall be Type A in accordance with Section 9-14.4(7) of the Standard
Specifications. Application rate shall be per manufacturer’s written recommendations.

8-02.5 Payment

Section 8-02.5 of the Standard Specifications is supplemented with the following:

(******)
The per-acre price for “Seeding, Fertilizing, and Mulching” shall also include providing
tacking agent.

SECTION 8-04 CURBS, GUTTERS, AND SPILLWAYS

8-04.3 Construction Requirements

8-04.3(1) Cement Concrete Curbs, Gutters and Spillways

The first paragraph of Section 8-04.3(1) of the Standard Specifications is deleted and replaced with
the following:

(******)
Cement concrete curb, curb and gutter, gutter, spillway, Cement Concrete Sidewalk Ramps,
and stairs shall be constructed with air entrained concrete Class 4000 conforming to the
requirements of Section 6-02.

8-04.3(1)A Extruded Cement Concrete Curb
Section 8-04.3(1)A of the Standard Specifications is supplemented with the following:

(******)
Should the Contractor elect to have the curbs and gutters cast by the extruded method, then a modified Class 4000 concrete mix shall be used. The proposed mix shall be submitted for review and approval by the Engineer a minimum of ten working days prior to the date of intended use.

The following new section is added to Division 8.

SECTION 8-05 DRIVEWAY APPROACHES

8-05.1 Description

(******)
The Contractor shall excavate gravel driveway approaches and field entrances adjacent to the roadway, place and compact Crushed Surfacing Top Course as directed by the Engineer. Unless shown otherwise on the attached Plans or directed otherwise by the Engineer, driveway approaches shall be excavated at a constant slope from the finished roadway surface to the right of way line. The Contractor shall place 0.3 Feet compacted depth Crushed Surfacing Top Course on gravel driveway approaches.

All costs associated with removing and disposing of hard surfacing shall be considered incidental to the other Bid Items of the Contract, and no further payment shall be made.

8-05.3 Construction Requirements

(******)
Where necessary, the Contractor shall excavate the existing driveway approaches to a neat line. Crushed surfacing materials shall be placed in accordance with Section 4-04 of the Standard Specifications.

8-05.5 Payment

(******)
The Contract Unit Price for "Roadway Excavation Incl. Haul" per Cubic Yard, shall be full compensation for all materials, labor, equipment, tools, excavating and hauling to complete the work as specified, and no further payment shall be made.

The Contract Unit Price for "Crushed Surfacing Top Course" per Ton, shall be full compensation for furnishing all materials, labor, tools, and equipment necessary to complete the work as specified and no further payment shall be made.

SECTION 8-18, MAILBOX SUPPORT
8-18.3 Construction Requirements

Section 8-18.3 is supplemented with the following:

(******)
Prior to construction, the Contractor shall inventory all mailboxes to be relocated along
the project and either salvage the existing mailboxes or replace in kind.

Mailbox supports shall be replaced as shown on the attached Standard Plans and
according to the locations shown on construction plans, or at the location directed by
the Engineer.

All mailboxes shall be installed such that the front face of the mailbox is flush with the
new edge of road and as per the direction of the Engineer.

Mailbox List

See the appropriate Construction Plan sheet for the mailbox Schedule.

8-18.5 Payment

Section 8-18.5 is supplemented with the following:

(******)
Payment for the Contract Bid Item "Mailbox Support Type _" per Each, shall include
all costs for materials, haul, labor, equipment and all other costs necessary to complete
the item as specified and no further payment shall be made.

All costs associated with transferring the existing mailboxes and newspaper tubes to the
new mailbox supports, including support hardware, clamps, etc. shall be considered
incidental to the Bid Items "Mailbox Support Type _" per Each, and no further payment
shall be made.

SECTION 9-03 AGGREGATES

9-03.8(6) Proportions of Materials

Section 9-03.8(6) is supplemented with the following:

(******)
For the determination of a project mix design, the Contractor shall submit to the
Engineer's representative, samples of the various aggregates to be used, along with the
gradation data showing stockpile averages and variation of the aggregate produced, along
with proposed combining ratios and average gradation of the completed mix. The initial
asphalt content shall be determined by the Engineer from the aggregates and data provided.

9-03.8(6)A Basis of Acceptance

(******)
Section 9-03.8(6)A is deleted.

SECTION 9-28 SIGNING MATERIALS AND FABRICATION

9-28.8 Sheet Aluminum Signs

The second paragraph of Section 9-28.8 is supplemented with the following:

(******)
All sheet thickness over 36 inches shall be 0.125 inch.

STANDARD PLANS

April 2, 2007

The State of Washington Standard Plans for Road, Bridge and Municipal Construction M21-01 transmitted under Publications Transmittal No. PT 07-008, effective April 2, 2007 is made a part of this contract.

The Standard Plans are revised as follows:

All Standard Plans
All references in the Standard Plans to "Asphalt Concrete Pavement" shall be revised to read "Hot Mix Asphalt".

All references in the Standard Plans to the abbreviation "ACP" shall be revised to read "HMA".

C-11b Sheets 1 and 2
In the PRECAST FOOTING, ELEVATION view (Sheet 1) and in the CAST-IN-PLACE FOOTING, ELEVATION view (Sheet 2), COMMERCIAL CONCRETE is revised to CONCRETE CLASS 4000.

In the BREAKAWAY ANCHOR ANGLE, ELEVATION view (Sheet 2), the welding symbols are revised to indicate that the 1/4" Inside Gussets have 1/4" fillet weld joints, and the 1/2" End Gussets have 1/2" fillet weld joints.

D-1a Sheet 2 & D-1b Sheet 2
Reinforcing Steel Bar marked “R1” (see lower left corner): the dimension 1’ - 2 1/2” is revised to 1’ - 0 1/2”.
G-8g Sheet 1

In the ELEVATION views, in the labels LOWER SIGN POST SUPPORT: the parenthetical specification “12 GAGE” is revised to “7 GAGE”.

I-10

In NOTE 1: the reference to Standard Specification 8-01.3(5)A is revised to Standard Specification 8-01.3(6)A.

The following are the Standard Plan numbers applicable at the time this project was advertised. The date shown with each plan number is the publication approval date shown in the lower right-hand corner of that plan. Standard Plans showing different dates shall not be used in this contract.

A-1 .............. 10/24/06  A-3 .............. 5/30/02  A-6 .............. 2/24/03
A-2 .............. 12/20/06  A-5 .............. 2/24/03  A-7 .............. 10/04/05
B-5.20-00 ....... 6/01/06  B-30.50-00 ....... 6/01/06  B-75.20-00 ....... 6/01/06
B-5.40-00 ....... 6/01/06  B-30.70-00 ....... 6/01/06  B-75.50-00 ....... 6/08/06
B-5.60-00 ....... 6/01/06  B-30.80-00 ....... 6/08/06  B-75.60-00 ....... 6/08/06
B-10.20-00 ....... 6/01/06  B-30.90-00 ....... 6/08/06  B-80.20-00 ....... 6/08/06
B-10.40-00 ....... 6/01/06  B-35.20-00 ....... 6/08/06  B-80.40-00 ....... 6/01/06
B-10.60-00 ....... 6/08/06  B-35.40-00 ....... 6/08/06  B-82.20-00 ....... 6/01/06
B-15.20-00 ....... 6/01/06  B-40.20-00 ....... 6/01/06  B-85.10-00 ....... 6/01/06
B-15.40-00 ....... 6/01/06  B-40.40-00 ....... 6/01/06  B-85.20-00 ....... 6/01/06
B-15.60-00 ....... 6/01/06  B-45.20-00 ....... 6/01/06  B-85.30-00 ....... 6/01/06
B-20.20-01 ....... 11/21/06  B-45.40-00 ....... 6/01/06  B-85.40-00 ....... 6/08/06
B-20.40-01 ....... 11/21/06  B-50.20-00 ....... 6/01/06  B-85.50-00 ....... 6/08/06
B-20.60-01 ....... 11/21/06  B-55.20-00 ....... 6/01/06  B-90.10-00 ....... 6/08/06
B-25.20-00 ....... 6/08/06  B-60.20-00 ....... 6/08/06  B-90.20-00 ....... 6/08/06
B-25.40-00 ....... 6/08/06  B-65.20-00 ....... 6/08/06  B-90.30-00 ....... 6/08/06
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B-30.30-00 ....... 6/01/06  B-70.20-00 ....... 6/01/06  B-95.20-00 ....... 6/08/06
B-30.40-00 ....... 6/01/06  B-70.60-00 ....... 6/01/06  B-95.40-00 ....... 6/08/06
C-1 .............. 2/06/07  C-3b .............. 10/04/05  C-13 .............. 4/16/99
C-1a .............. 7/31/98  C-3c .............. 6/21/06  C-13a .............. 4/16/99
C-1b .............. 10/31/03  C-3d .............. 3/03/05  C-13b .............. 4/16/99
C-1c .............. 5/30/97  C-4 .............. 2/21/07  C-14a .............. 7/26/02
C-1d .............. 10/31/03  C-4a .............. 2/21/07  C-14b .............. 7/26/02
C-2 .............. 1/06/00  C-4b .............. 6/08/06  C-14c .............. 7/26/02
C-2a .............. 6/21/06  C-4e .............. 2/20/03  C-14d .............. 7/26/02
C-2b .............. 6/21/06  C-4f .............. 6/30/04  C-14e .............. 7/26/02
C-2c .............. 6/21/06  C-5 .............. 10/31/03  C-14f .............. 9/02/05
C-2d .............. 6/21/06  C-6 .............. 5/30/97  C-14g .............. 11/21/06
C-2e .............. 6/21/06  C-6a .............. 3/14/97  C-14h .............. 1/11/06
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Prevailing Wage Rates
Washington State Prevailing Wage Rates For Public Works Contracts
The PREVAILING WAGES listed here include both the hourly wage rate and the hourly rate of fringe benefits.
On public works projects, workers' wage and benefit rates must add to not less than this total. A brief description of overtime calculation requirements is provided on the Benefit Code Key.

**YAKIMA COUNTY**
Effective 03-03-07

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## YAKIMA COUNTY
### Effective 03-03-07

(See Benefit Code Key)

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<td>Concrete Finish Machine - Laser Screed</td>
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<td>Concrete Pumps</td>
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<td>Concrete Pump-Truck Mount with Boom Attachment</td>
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<td>Conveyors</td>
<td>$41.93</td>
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<td>Cranes, 19 Tons, with Attachments</td>
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<td>Cranes, 20 - 44 Tons, with Attachments</td>
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<td>Cranes, 45 Tons - 99 Tons, Under 150 ft of Boom (Including Jib with Attachments)</td>
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<td>Cranes, 100 Tons - 199 Tons, or 150 ft of Boom (Including Jib with Attachments)</td>
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<td>Cranes, 200 Tons to 500 Tons, or 250 ft of Boom (Including Jib with Attachments)</td>
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<td>Time Code</td>
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<tr>
<td>WITH ATTACHMENTS)</td>
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<tr>
<td>CRANES, A-FRAME, 10 TON AND UNDER</td>
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<td>1M</td>
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<td>CRANES, A-FRAME, OVER 10 TON</td>
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<td>CRANES, OVER 300 TONS, OR 300' OF BOOM INCLUDING JIB WITH ATTACHMENTS</td>
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<td>CRANES, TOWER CRANE UP TO 175' IN HEIGHT, BASE TO BOOM</td>
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<td>DRILL OILERS - AUGER TYPE, TRUCK OR CRANE MOUNT</td>
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<td>ELEVATOR AND MANLIFT, PERMANENT AND SHAFT-TYPE</td>
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<td>GRADER/CHECKER AND STAKEMAN</td>
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<td>MECHANICS, ALL</td>
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<td>MOTOR PATROL GRADER (FINISHING)</td>
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<td>OIL DISTRIBUTORS, BLOWER DISTRIBUTION AND MULCH SEEDING OPERATOR</td>
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<td>PILEDRIVER (OTHER THAN CRANE MOUNT)</td>
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<td>PUMPS, WATER</td>
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<td>ROLLERS, PLANTMIX OR MULTILIFT MATERIALS</td>
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<td>8L</td>
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<td>SCRAPERS - SELF PROPELLED, HARD TAIL END DUMP, ARTICULATING</td>
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<td>8L</td>
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<td>Classification</td>
<td>PREVAILING WAGE</td>
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<td>Note Code</td>
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<tr>
<td>OFF-ROAD EQUIPMENT (45 YD AND OVER)</td>
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<td>SCREAM MAN</td>
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**YAKIMA COUNTY**

Effective 03-03-07

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(See Benefit Code Key)
OVERTIME CODES

OVERTIME CALCULATIONS ARE BASED ON THE HOURLY RATE ACTUALLY PAID TO THE WORKER. ON PUBLIC WORKS PROJECTS, THE HOURLY RATE MUST BE NOT LESS THAN THE PREVAILING RATE OF WAGE MINUS THE HOURLY RATE OF THE COST OF FRINGE BENEFITS ACTUALLY PROVIDED FOR THE WORKER.

1. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

A. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS SHALL ALSO BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

B. ALL HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

C. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TEN (10) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER OVERTIME HOURS WORKED SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

D. THE FIRST TWO (2) HOURS BEFORE OR AFTER A FIVE - EIGHT (8) HOUR WORKWEEK DAY OR A FOUR - TEN (10) HOUR WORKWEEK DAY AND THE FIRST EIGHT (8) HOURS WORKED THE NEXT DAY AFTER EITHER WORKWEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL ADDITIONAL HOURS WORKED AND ALL WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

E. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST EIGHT (8) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER HOURS WORKED MONDAY THROUGH SATURDAY, AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

F. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TEN (10) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER OVERTIME HOURS WORKED, EXCEPT LABOR DAY, SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT THREE TIMES THE HOURLY RATE OF WAGE.

G. THE FIRST TEN (10) HOURS WORKED ON SATURDAYS AND THE FIRST TEN (10) HOURS WORKED ON A FIFTH CALENDAR WEEKDAY IN A FOUR - TEN HOUR SCHEDULE, SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF TEN (10) HOURS PER DAY MONDAY THROUGH SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

H. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS IF WORK IS LOST DUE TO INCLEMENT WEATHER CONDITIONS OR EQUIPMENT BREAKDOWN) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED MONDAY THROUGH SATURDAY OVER TWELVE (12) HOURS AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

J. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TEN (10) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED OVER TEN (10) HOURS MONDAY THROUGH SATURDAY, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

K. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

L. ALL HOURS WORKED IN EXCESS OF TEN (10) HOURS PER DAY MONDAY THROUGH SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

M. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS IF WORK IS LOST DUE TO INCLEMENT WEATHER CONDITIONS) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

N. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

O. THE FIRST TEN (10) HOURS WORKED ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS, HOLIDAYS AND AFTER TWELVE (12) HOURS, MONDAY THROUGH FRIDAY, AND AFTER TEN (10) HOURS ON SATURDAY SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

P. ALL HOURS WORKED ON SATURDAYS (EXCEPT MAKEUP DAYS IF CIRCUMSTANCES WARRANT) AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.
1. Q. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND UP TO TEN (10) HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF TEN (10) HOURS PER DAY MONDAY THROUGH SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS (EXCEPT CHRISTMAS DAY) SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON CHRISTMAS DAY SHALL BE PAID AT TWO AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

R. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

S. THE FIRST TWO (2) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST EIGHT (8) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL OTHER OVERTIME HOURS WORKED, EXCEPT LABOR DAY, SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT THREE TIMES THE HOURLY RATE OF WAGE.

T. ALL HOURS WORKED ON SATURDAYS, EXCEPT MAKE-UP DAYS, SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED AFTER 6:00PM SATURDAY TO 6:00AM MONDAY AND ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

U. ALL HOURS WORKED ON SATURDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON LABOR DAY SHALL BE PAID AT THREE TIMES THE HOURLY RATE OF WAGE.

V. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS (EXCEPT THANKSGIVING DAY AND CHRISTMAS DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON THANKSGIVING DAY AND CHRISTMAS DAY SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

W. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS (EXCEPT MAKE-UP DAYS DUE TO CONDITIONS BEYOND THE CONTROL OF THE EMPLOYER) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

X. THE FIRST FOUR (4) HOURS AFTER EIGHT (8) REGULAR HOURS MONDAY THROUGH FRIDAY AND THE FIRST TWELVE (12) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED OVER TWENTY (20) HOURS MONDAY THROUGH SATURDAY, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. WHEN HOLIDAY FALLS ON SATURDAY OR SUNDAY, THE DAY BEFORE SATURDAY, FRIDAY, AND THE DAY AFTER SUNDAY, MONDAY, SHALL BE CONSIDERED THE HOLIDAY AND ALL WORK PERFORMED SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

2. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

A. THE FIRST SIX (6) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF SIX (6) HOURS ON SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

B. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

C. ALL HOURS WORKED ON SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

D. ALL HOURS WORKED ON SATURDAYS AND SUNDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. THE FIRST EIGHT (8) HOURS WORKED ON HOLIDAYS SHALL BE PAID AT STRAIGHT TIME IN ADDITION TO THE HOLIDAY PAY. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS ON HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

E. ALL HOURS WORKED ON SATURDAYS OR HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS OR ON LABOR DAY SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

F. THE FIRST EIGHT (8) HOURS WORKED ON HOLIDAYS SHALL BE PAID AT THE STRAIGHT HOURLY RATE OF WAGE IN ADDITION TO THE HOLIDAY PAY. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS ON HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

G. ALL HOURS WORKED ON SUNDAY SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT TWO AND ONE-HALF TIMES THE HOURLY RATE OF WAGE INCLUDING HOLIDAY PAY.

H. ALL HOURS WORKED ON SUNDAY SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.
BENEFIT CODE KEY - EFFECTIVE 03-03-07
-3-

2. I. ALL HOURS WORKED ON SATURDAYS AND HOLIDAYS (EXCEPT LABOR DAY) SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SUNDAYS AND ON LABOR DAY SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

J. ALL HOURS WORKED ON SUNDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON PAID HOLIDAYS SHALL BE PAID AT TWO AND ONE-HALF TIMES THE HOURLY RATE OF WAGE, INCLUDING THE HOLIDAY PAY. ALL HOURS WORKED ON UNPAID HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

K. ALL HOURS WORKED ON HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE IN ADDITION TO THE HOLIDAY PAY.

M. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

O. ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE.

P. THE FIRST EIGHT (8) HOURS ON SATURDAY SHALL BE PAID AT ONE AND ONE-HALF TIMES THE HOURLY RATE OF WAGE. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS ON SATURDAY AND ALL HOURS WORKED ON SUNDAYS AND HOLIDAYS SHALL BE PAID AT TWO TIMES THE HOURLY RATE OF WAGE.

4A. ALL HOURS WORKED IN EXCESS OF EIGHT (8) HOURS PER DAY OR FORTY (40) HOURS PER WEEK SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE. ALL HOURS WORKED ON SATURDAYS, SUNDAYS AND HOLIDAYS SHALL BE PAID AT DOUBLE THE HOURLY RATE OF WAGE.

HOLIDAY CODES

5. A. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (7).

B. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE DAY BEFORE CHRISTMAS, AND CHRISTMAS DAY (8).

C. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).

D. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AND SATURDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).

E. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, PRESIDENTIAL ELECTION DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (8).


G. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE LAST WORK DAY BEFORE CHRISTMAS, AND CHRISTMAS DAY (7).


I. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS (6).

J. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS EVE DAY, AND CHRISTMAS DAY (7).

N. HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, VETERANS' DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (9).

P. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AND SATURDAY AFTER THANKSGIVING DAY, THE DAY BEFORE CHRISTMAS, AND CHRISTMAS DAY (9).

Q. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS DAY (6).

R. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, ONE-HALF DAY BEFORE CHRISTMAS DAY, AND CHRISTMAS DAY (7 1/2).
5. S. PAID HOLIDAYS: NEW YEAR'S DAY, PRESIDENTS' DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, AND CHRISTMAS DAY (7).


V. PAID HOLIDAYS: SIX (6) PAID HOLIDAYS.

W. PAID HOLIDAYS: NINE (9) PAID HOLIDAYS.

X. HOLIDAYS: AFTER 520 HOURS - NEW YEAR'S DAY, THANKSGIVING DAY AND CHRISTMAS DAY. AFTER 2080 HOURS - NEW YEAR'S DAY, WASHINGTON'S BIRTHDAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, CHRISTMAS DAY AND A FLOATING HOLIDAY (8).

Y. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, PRESIDENTIAL ELECTION DAY, THANKSGIVING DAY, THE FRIDAY FOLLOWING THANKSGIVING DAY, AND CHRISTMAS DAY (8).

Z. HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, VETERANS DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, AND CHRISTMAS DAY (8).


B. PAID HOLIDAYS: NEW YEAR'S EVE DAY, NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, THE FRIDAY AFTER THANKSGIVING DAY, CHRISTMAS EVE'S DAY, AND CHRISTMAS DAY (9).


I. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, FRIDAY AFTER THANKSGIVING DAY, AND CHRISTMAS DAY (7).


Q. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, VETERANS DAY, THANKSGIVING DAY, THE DAY AFTER THANKSGIVING DAY AND CHRISTMAS DAY (8). UNPAID HOLIDAY: PRESIDENTS' DAY.


V. PAID HOLIDAYS: NEW YEAR'S DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS EVE DAY, CHRISTMAS DAY, AND ONE DAY OF THE EMPLOYEE'S CHOICE (9).

W. PAID HOLIDAYS: NEW YEAR'S DAY, DAY BEFORE NEW YEAR'S DAY, PRESIDENTS DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, DAY BEFORE OR AFTER CHRISTMAS DAY (10).

X. PAID HOLIDAYS: NEW YEAR'S DAY, DAY BEFORE OR AFTER NEW YEAR'S DAY, PRESIDENTS DAY, MEMORIAL DAY, INDEPENDENCE DAY, LABOR DAY, THANKSGIVING DAY, DAY AFTER THANKSGIVING DAY, CHRISTMAS DAY, DAY BEFORE OR AFTER CHRISTMAS DAY, EMPLOYEE'S BIRTHDAY (11).
8. A. THE STANDBY RATE OF PAY FOR DIVERS SHALL BE ONE-HALF TIMES THE DIVERS RATE OF PAY. IN ADDITION TO THE HOURLY WAGE AND FRINGE BENEFITS, THE FOLLOWING DEPTH PREMIUMS APPLY TO DEPTHS OF FIFTY FEET OR MORE:
   OVER 50' TO 100' - $1.00 PER FOOT FOR EACH FOOT OVER 50 FEET
   OVER 100' TO 175' - $2.25 PER FOOT FOR EACH FOOT OVER 100 FEET
   OVER 175' TO 250' - $5.50 PER FOOT FOR EACH FOOT OVER 175 FEET
   OVER 250' - DIVERS MAY NAME THEIR OWN PRICE, PROVIDED IT IS NO LESS THAN THE SCALE LISTED FOR 250 FEET

C. THE STANDBY RATE OF PAY FOR DIVERS SHALL BE ONE-HALF TIMES THE DIVERS RATE OF PAY. IN ADDITION TO THE HOURLY WAGE AND FRINGE BENEFITS, THE FOLLOWING DEPTH PREMIUMS APPLY TO DEPTHS OF FIFTY FEET OR MORE:
   OVER 50' TO 100' - $1.00 PER FOOT FOR EACH FOOT OVER 50 FEET
   OVER 100' TO 150' - $1.50 PER FOOT FOR EACH FOOT OVER 100 FEET
   OVER 150' TO 200' - $2.00 PER FOOT FOR EACH FOOT OVER 150 FEET
   OVER 200' - DIVERS MAY NAME THEIR OWN PRICE

D. WORKERS WORKING WITH SUPPLIED AIR ON HAZMAT PROJECTS RECEIVE AN ADDITIONAL $1.00 PER HOUR.

L. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS - LEVEL A: $0.75, LEVEL B: $0.50, AND LEVEL C: $0.25.

M. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS: LEVELS A & B: $1.00, LEVELS C & D: $0.50.

N. WORKERS ON HAZMAT PROJECTS RECEIVE ADDITIONAL HOURLY PREMIUMS AS FOLLOWS - LEVEL A: $1.00, LEVEL B: $0.75, LEVEL C: $0.50, AND LEVEL D: $0.25.
Washington State Department of Labor and Industries

Policy Statement

(Regarding the Production of "Standard" or "Non-standard" Items)

Below is the department's (State L&I's) list of criteria to be used in determining whether a prefabricated item is "standard" or "non-standard". For items not appearing on WSDOT's predetermined list, these criteria shall be used by the Contractor (and the Contractor's subcontractors, agents to subcontractors, suppliers, manufacturers, and fabricators) to determine coverage under RCW 39.12. The production, in the State of Washington, of non-standard items is covered by RCW 39.12, and the production of standard items is not. The production of any item outside the State of Washington is not covered by RCW 39.12.

1. Is the item fabricated for a public works project? If not, it is not subject to RCW 39.12. If it is, go to question 2.

2. Is the item fabricated on the public works jobsite? If it is, the work is covered under RCW 39.12. If not, go to question 3.

3. Is the item fabricated in an assembly/fabrication plant set up for, and dedicated primarily to, the public works project? If it is, the work is covered by RCW 39.12. If not, go to question 4.

4. Does the item require any assembly, cutting, modification or other fabrication by the supplier? If not, the work is not covered by RCW 39.12. If yes, go to question 5.

5. Is the prefabricated item intended for the public works project typically an inventory item which could reasonably be sold on the general market? If not, the work is covered by RCW 39.12. If yes, go to question 6.

6. Does the specific prefabricated item, generally defined as standard, have any unusual characteristics such as shape, type of material, strength requirements, finish, etc? If yes, the work is covered under RCW 39.12.

Any firm with questions regarding the policy, WSDOT's Predetermined List, or for determinations of covered and non-covered workers shall be directed to State L&I at (360) 902-5330.
Below is a list of potentially prefabricated items, originally furnished by WSDOT to Washington State Department of Labor and Industries, that may be considered non-standard and therefore covered by the prevailing wage law, RCW 39.12. Items marked with an X in the "YES" column should be considered to be non-standard and therefore covered by RCW 39.12. Items marked with an X in the "NO" column should be considered to be standard and therefore not covered. Of course, exceptions to this general list may occur, and in that case shall be evaluated according to the criteria described in State and L&I's policy statement.

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<th>ITEM DESCRIPTION</th>
<th>YES</th>
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<tr>
<td>1. Manhole Ring &amp; Cover - manhole type 1, 2, 3, and 4. For use with Catch Basin type 2. The casting to meet AASHTO-M-105, class 30 gray iron casting. See Std. Plan B-30.10, B-30.70, B-30.80, and E-5.</td>
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<td>2. Frame &amp; Grate - frame and Grate for Catch Basin type 1, 1L, 1P, 2, and Concrete Inlets. Cast frame may be grade 70-36 steel, class 30 gray cast iron or grade 80-55-06 ductile iron. The cast grate may be grade 70-36 steel or grade 80-55-06 ductile iron. See Std. Plan B-25.20, B-30.20, B-30.30, B-30.40, and B-30.50.</td>
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<td>3. Grate Inlet &amp; Drop Inlet Frame &amp; Grate - Frame and Grate for Grate Inlets Type 1 or 2 or Drop Inlets Type 1 or 2. Angle iron frame to be cast into top of inlet. See Std. Plan B-35.20, B-40.20, B-40.40, and B-50.20.</td>
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<td>4. Concrete Pipe - Plain Concrete pipe and reinforced concrete pipe Class 2 to 5 sizes smaller than 60 inch diameter.</td>
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<tr>
<td>5. Concrete Pipe - Plain Concrete pipe and reinforced concrete pipe Class 2 to 5 sizes larger than 60 inch diameter.</td>
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6. Corrugated Steel Pipe - Steel lock seam corrugated pipe for culverts and storm sewers, sizes 30 inch to 120 inches in diameter. May also be treated, 1 thru 5.

7. Corrugated Aluminum Pipe - Aluminum lock seam corrugated pipe for culverts and storm sewers, sizes 30 inch to 120 inches in diameter. May also be treated, #5.

8. Anchor Bolts & Nuts - Anchor Bolts and Nuts, for mounting sign structures, luminaries and other items, shall be made from commercial bolt stock. See Contract Plans and Std. Plans for size and material type.

9. Aluminum Pedestrian Handrail - Pedestrian handrail conforming to the type and material specifications set forth in the contract plans. Welding of aluminum shall be in accordance with Section 9-28.14(3).

10. Major Structural Steel Fabrication - Fabrication of major steel items such as trusses, beams, girders, etc., for bridges.

11. Minor Structural Steel Fabrication - Fabrication of minor steel items such as special hangers, brackets, access doors for structures, access ladders for irrigation boxes, bridge expansion joint systems, etc., involving welding, cutting, punching and/or boring of holes. See Contact Plans for item description and shop drawings.

12. Aluminum Bridge Railing Type BP - Metal bridge railing conforming to the type and material specifications set forth in the Contract Plans. Welding of aluminum shall be in accordance with Section 9-28.14(3).
13. Concrete Piling—Precast-Prestressed concrete piling for use as 55 and 70 ton concrete piling. Concrete to conform to Section 9-19.1 of Std. Spec. Shop drawings for approval shall be provided per Section 6-05.3(3) of the Std. Spec. See Std. Plans E-4 and E-4a

14. Manhole Type 1, 2, 3 and 4 - Precast Manholes with risers and flat top slab and/or cones. See Std. Plans B-15.20, B-15.40, and B-15.60.

15. Drywell - Drywell as specified in Section 9-12.7 of the Std. Sec. See Std. Plan B-20.20, B-20.40, and B-20.60.

16. Catch Basin - Catch Basin type 1, 1L, 1P, and 2, including risers, frames maybe cast into riser. See Std. Plans B-5.20, B-5.40, B-5.60, B-10.20, B-10.40, and B-10.60.

17. Precast Concrete Inlet - Concrete Inlet with risers, frames may be cast into risers. See Std. Plan B-25.60.

18. Drop Inlet Type 1 - Drop Inlet Type 1 with support angles and grate. See Std. Plans B-45.20.

19. Drop Inlet Type 2 - Drop Inlet type 2 with support angles and grate. See Std. Plans B-45.40.

20. Grate Inlet Type 2 - Grate Inlet Type 2 with risers and top unit with bearing angles. See Std. Plans B-35.40.

21. Precast Concrete Utility Vaults - Precast Concrete utility vaults of various sizes. Used for in ground storage of utility facilities and controls. See Contract Plans for size and construction requirements. Shop drawings

Supplemental To Wage Rates
Page 4
are to be provided for approval prior to casting.

22. Vault Risers - For use with Valve Vaults and Utilities Vaults. X

23. Valve Vault - For use with underground utilities. See Contract Plans for details. X

24. Precast Concrete Barrier - Precast Concrete Barrier for use as new barrier or may also be used as Temporary Concrete Barrier. Only new state approved barrier may be used as permanent barrier. X

25. Reinforced Earth Wall Panels - Reinforced Earth Wall Panels in size and shape as shown in the Plans. Fabrication plant has annual approval for methods and materials to be used. See Shop Drawing. Fabrication at other locations may be approved, after facilities inspection, contact HQ. Lab. X

26. Precast Concrete Walls - Precast Concrete Walls - tilt-up wall panel in size and shape as shown in Plans. Fabrication plant has annual approval for methods and materials to be used. X

27. Precast Railroad Crossings - Concrete Crossing Structure Slabs. X

28. 12, 18 and 26 inch Standard Precast Prestressed Girder - Standard Precast Prestressed Girder for use in structures. Fabricator plant has annual approval of methods and materials to be used. Shop Drawing to be provided for approval prior to casting girders. See Std. Spec. Section 6-02.3(25)c. X
29. Prestressed Concrete Girder Series 4-14 - Prestressed Concrete Girders for use in structures. Fabricator plant has annual approval of methods and materials to be used. Shop Drawing to be provided for approval prior to casting girders. See Std. Spec. Section 6-02.3(25)c.

30. Prestressed Tri-Beam Girder - Prestressed Tri-Beam Girders for use in structures. Fabricator plant has annual approval of methods and materials to be used. Shop Drawing to be provided for approval prior to casting girders. See Std. Spec. Section 6-02.3(25)c.

31. Prestressed Precast Hollow-Core Slab - Precast Prestressed Hollow-core slab for use in structures. Fabricator plant has annual approval of methods and materials to be used. Shop Drawing to be provided for approval prior to casting girders. See Std. Spec. Section 6-02.3(25)c.

32. Prestressed-Bulb Tee Girder - Bulb Tee Prestressed Girder for use in structures. Fabricator plant has annual approval of methods and materials to be used. Shop Drawing to be provided for approval prior to casting girders. See Std. Spec. Section 6-02.3(26)A.

33. Monument Case and Cover - To meet AASHTO-M-105 class 30 gray iron casting. See Std. Plan H-7.

34. Cantilever Sign Structure - Cantilever Sign Structure fabricated from steel tubing meeting AASHTO-M-183. See Std. Plans G-3, G-3a, G-3b, and Contract Plans for details. The steel structure shall be galvanized after fabrication in accordance with AASHTO-M-111.

35. Mono-tube Sign Structures - Mono-tube Sign Bridge fabricated to details shown in the Plans. Shop drawings for approval are required prior to fabrication.
36. Steel Sign Bridges - Steel Sign Bridges fabricated from steel tubing meeting AASHTO-M-138 for Aluminum Alloys. See Std. Plans G-2, G2a, and Contract Plans for details. The steel structure shall be galvanized after fabrication in accordance with AASHTO-M-111.

37. Steel Sign Post - Fabricated steel sign posts as detailed in Std. Plan G-8a, G-8b, G-8c, G-8d, G-8e, G-8f, and G-8g. Shop drawings for approval are to be provided prior to fabrication.

38. Light Standard-Prestressed - Spun, prestressed, hollow, concrete poles.

39. Light Standards - Lighting Standards for use on highway illumination systems, poles to be fabricated to conform with methods and materials as specified on Std. Plan J-1a. See Special Provisions for pre-approved drawings.

40. Traffic Signal Standards - Traffic Signal Standards for use on highway and/or street signal systems. Standards to be fabricated to conform with methods and material as specified on Std. Plans J-7a and J-7c. See Special Provisions for pre-approved drawings.

41. Traffic Curb, Type A or C Precast - Type A or C Precast traffic curb, for use in construction of raised channelization, and other traffic delineation uses such as parking lots, rest areas, etc. NOTE: Acceptance based on inspection of Fabrication Plant and an advance sample of curb section to be submitted for approval by Engineer.
42. Traffic Signs - Prior to approval of a Fabricator of Traffic Signs, the sources of the following materials must be submitted and approved for reflective sheeting, legend material, and aluminum sheeting. **Note:** Fabrication inspection required. Only signs tagged "Fabrication Approved" by WSDOT Sign Fabrication Inspector to be installed.

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43. Cutting & bending reinforcing steel

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44. Guardrail components

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45. Aggregates/Concrete mixes

Covered by WAC 296-127-018

46. Asphalt

Covered by WAC 296-127-018

47. Fiber fabrics

| X |

48. Electrical wiring/components

| X |

49. Treated or untreated timber piles

| X |

50. Girder pads (elastomeric bearing)

| X |

Supplemental To Wage Rates
Page 8
51. Standard Dimension lumber  X

52. Irrigation components  X

53. Fencing materials  X

54. Guide Posts  X

55. Raised Pavement Markers  X

56. Epoxy  X

57. Cribbing  X

58. Water distribution materials  X

59. Steel "H" piles  X

60. Steel pipe for concrete pile casings  X

61. Steel pile tips, standard  X

62. Steel pile tips, custom  X
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| Machine Operator     | 10.53 | 1           |              |           |
| Painter              | 9.76  | 1           |              |           |

| **Counties Covered:** |     |                |              |           |
| Chelan               |     |                |              |           |
| Fitter               | 15.04 | 1           |              |           |
| Welder               | 12.24 | 1           |              |           |
| Machine Operator     | 9.71  | 1           |              |           |
| Painter              | 9.93  | 1           |              |           |
| Laborer              | 9.40  | 1           |              |           |

<p>| <strong>Counties Covered:</strong> |     |                |              |           |
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| Fitter               | 15.18 | 1           |              |           |
| Welder               | 15.18 | 1           |              |           |
| Machine Operator     | 10.66 | 1           |              |           |
| Painter              | 11.41 | 1           |              |           |
| Laborer              | 11.13 | 1           |              |           |</p>
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Supplemental To Wage Rates
Page 13
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Washington State Department of Labor and Industries
Policy Statements
(Regarding Production and Delivery of Gravel, Concrete, Asphalt, etc.)

The following two letters from the State Department of Labor and Industries (State L&I) dated August 18, 1992 and June 18, 1999, clarify the intent and establish policy for administering the provisions of WAC 296-127-018 COVERAGE AND EXEMPTIONS OF WORKERS INVOLVED IN THE PRODUCTION AND DELIVERY OF GRAVEL, CONCRETE, ASPHALT, OR SIMILAR MATERIALS.

Any firm with questions regarding the policy, these letters, or for determinations of covered and non-covered workers shall be directed to State L&I at (360) 902-5330.

Effective September 1, 1993, minimum prevailing wages for all work covered by WAC 296-127-018 for the production and/or delivery of materials to a public works contract will be found under the regular classification of work for Teamsters, Power Equipment Operators, etc.
August 18, 1992

TO: All Interested Parties

FROM: Jim P. Christensen
Acting Industrial Statistician

SUBJECT: Materials Suppliers - WAC 296-127-018

This memo is intended to provide greater clarity regarding the application of WAC 296-127-018 to awarding agencies, contractors, subcontractors, material suppliers and other interested parties. The information contained herein should not be construed to cover all possible scenarios which might require the payment of prevailing wage. The absence of a particular activity under the heading "PREVAILING WAGES ARE REQUIRED FOR" does not mean that the activity is not covered.

Separate Material Supplier Equipment Operator rates have been eliminated. For those cases where a production facility is set up for the specific purpose of supplying materials to a public works construction site, prevailing wage rates for operators of equipment such as crushers and batch plants can be found under Power Equipment Operators.

**PREVAILING WAGES ARE REQUIRED FOR:**

1. Hauling materials away from a public works project site, including excavated materials, demolished materials, etc.

2. Delivery of materials to a public works project site using a method that involves incorporation of the delivered materials into the project site, such as spreading, leveling, rolling, etc.

3. The production of materials at a facility that is established for the specific, but not necessarily exclusive, purpose of supplying materials for a public works project.

4. Delivery of the materials mentioned in #3 above, regardless of the method of delivery.

**PREVAILING WAGES ARE NOT REQUIRED FOR:**

1. The production of materials by employees of an established materials supplier, in a permanent facility, as well as the delivery of these materials, as long as delivery does not include incorporation of the materials into the job site.

2. Delivery of materials by a common or contract carrier, as long as delivery does not include incorporation of the materials into the job site.

3. Production of materials for unspecified future use.
TO: Kerry S. Radcliff, Editor  
Washington State Register

FROM: Gary Moore, Director  
Department of Labor and Industries

SUBJECT: Notice re WAC 296-127-018, Coverage and exemptions of workers involved in the production and delivery of gravel, concrete, asphalt, or similar materials

The department wishes to publish the following Notice in the next edition of the Washington State Register:

NOTICE

Under the current material supplier regulations, WAC 296-127-018, the department takes the position that prevailing wages do not apply to the delivery of wet concrete to public works sites, unless the drivers do something more than just deliver the concrete. Drivers delivering concrete into a crane and bucket, hopper of a pump truck, or forms or footings, are not entitled to prevailing wages unless they operate machinery or use tools that screed, float, or put a finish on the concrete.

This position applies only to the delivery of wet concrete. It does not extend to the delivery of asphalt, sand, gravel, crushed rock, or other similar materials covered under WAC 296-127-018. The department’s position applies only to this regulation.

If you need additional information regarding this matter, please contact Greg Mowat, Program Manager, Employment Standards, at P.O. Box 44510, Olympia, WA 98504-4510, or call (360) 902-5310.

Please publish the above Notice in WSR 99-13. If you have questions or need additional information, please call Selwyn Walters at 902-4206. Thank you.

Cc: Selwyn Walters, Rules Coordinator  
Patrick Woods, Assistant Director  
Greg Mowat, Program Manager
Standard Plans
Pipe Allowances:

<table>
<thead>
<tr>
<th>Pipe Material</th>
<th>Maximum Inside Diameter</th>
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<tbody>
<tr>
<td>Reinforced or Plain Concrete</td>
<td>12&quot;</td>
</tr>
<tr>
<td>All Metal Pipe</td>
<td>15&quot;</td>
</tr>
<tr>
<td>CPSSP (Std. Spec. 9-05.20)</td>
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</tr>
<tr>
<td>Solid Wall PVC (Std. Spec. 9-05.12(1))</td>
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</tr>
<tr>
<td>Profile Wall PVC (Std. Spec. 9-05.12(2))</td>
<td>15&quot;</td>
</tr>
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</table>

* Corrugated Polyethylene Storm Sewer Pipe

Notes:

1. As acceptable alternatives to the rebar shown in the Precast Base Section, fibers (placed according to the Standard Specifications) or wire mesh having a minimum area of 0.12 square inches per foot shall be used with the minimum required rebar shown in the Alternative Precast Base Section. Wire mesh shall not be placed in the knockouts.

2. The knockout diameter shall not be greater than 20". Knockouts shall have a wall thickness of 2" minimum to 2.5" maximum. Provide a 1.5" minimum gap between the knockout wall and the outside of the pipe. After the pipe is installed, fill the gap with joint mortar in accordance with Standard Specification 9-04.3.

3. The maximum depth from the finished grade to the lowest pipe invert shall be 5".

4. The frame and grate may be installed with the flange up or down. The frame may be cast into the adjustment section.

5. The Precast Base Section may have a rounded floor, and the walls may be sloped at a rate of 1:24 or steeper.

6. The opening shall be measured at the top of the precast base section.

7. All pickup holes shall be grouted full after the basin has been placed.
**NOTES**

1. As acceptable alternatives to the rebar shown in the PRECAST BASE SECTION, fibers (placed according to the Standard Specifications), or wire mesh having a minimum area of 0.12 square inches per foot shall be used with the minimum required rebar shown in the ALTERNATIVE PRECAST BASE SECTION. Wire mesh shall not be placed in the knockouts.

2. The knockout diameter shall not be greater than 26”. Knockouts shall have a wall thickness of 2” minimum to 2.5” maximum. Provide a 1.5” minimum gap between the knockout wall and the outside of the pipe. After the pipe is installed, fill the gap with joint mortar in accordance with Standard Specification 9-04.3.

3. The maximum depth from the finished grade to the lowest pipe invert shall be 5’.

4. The frame and grate may be installed with the flange up or down. The frame may be cast into the adjustment section.

5. The Precast Base Section may have a rounded floor, and the walls may be sloped at a rate of 1:24 or steeper.

6. The opening shall be measured at the top of the precast base section.

7. All pickup holes shall be grouted full after the basin has been placed.

---

**PIPE ALLOWANCES**

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<tr>
<th>PIPE MATERIAL</th>
<th>MAXIMUM INSIDE DIAMETER</th>
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<tbody>
<tr>
<td>REINFORCED OR PLAIN CONCRETE</td>
<td>18”</td>
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<tr>
<td>ALL METAL PIPE</td>
<td>21”</td>
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<tr>
<td>CPS SP (STD. SPEC. 9-05.20)</td>
<td>18”</td>
</tr>
<tr>
<td>SOLID WALL PVC (STD. SPEC. 9-05.12(1))</td>
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<tr>
<td>PROFILE WALL PVC (STD. SPEC. 9-05.12(2))</td>
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* CORRUGATED POLYETHYLENE STORM SEWER PIPE

---

**CATCH BASIN TYPE 1L**

**STANDARD PLAN B-5.40-00**

*Approved for Publication*

**Mark Miller 6-1-06**

**Edward Princis 6-1-06**

**Washington State Department of Transportation**
NOTES

1. This frame is designed to accommodate 20" x 24" grates or covers as shown on Standard Plans B-30.20, B-30.30, B-30.40 and B-30.50.

2. When bolt-down grates or covers are specified in the Contract, provide two holes in the frame that are vertically aligned with the grate or cover slots. Tap each hole to accept a 5/8" - 11 NC x 2" Allen head cap screw. Location of bolt down holes varies among different manufacturers.

3. Refer to Standard Specification 9-O5.15(2) for additional requirements.
1. When bolt-down grates are specified in the Contract, provide two slots in the grate that are vertically aligned with the holes in the frame. Location of bolt-down slots varies among different manufacturers.

2. Refer to Standard Specification 9-05.15(2) for additional requirements.

3. For frame details, see Standard Plan B-30.10.

4. The thickness of the grate shall not exceed 1 5/8".

NOTES
NOTES

1. For use with Circular Frames (rings) detailed in Standard Plan B-30.70.
2. Studded Manhole Covers are intended for use with Drywells only. See Standard Plans B-20.20 and B-20.60.
84" or 96" FLAT SLAB TOP

TYPICAL ORIENTATION FOR ACCESS AND STEPS

72" FLAT SLAB TOP

ECCENTRIC CONE SECTION

48", 54", or 60" FLAT SLAB TOP

RECTANGULAR ADJUSTMENT SECTION

CIRCULAR ADJUSTMENT SECTION

As an acceptable alternative to rebar, wire mesh having a minimum area of 0.12 square inches per foot may be used for adjustment sections.
NOTES
1. See Standard Specifications Section 7-08.3(3) for Pipe Zone Backfill.
2. See Standard Specifications Section 9-03.12(3) for Gravel Backfill for Pipe Zone Bedding.
4. For sanitary sewer installation, concrete pipe shall be bedded to spring line.

CLEARANCE BETWEEN PIPES FOR MULTIPLE INSTALLATIONS

<table>
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<tr>
<th>PIPE TYPE</th>
<th>SIZE</th>
<th>MINIMUM DISTANCE BETWEEN BARRELS</th>
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<td></td>
<td>30&quot; to 96&quot;</td>
<td>DIAM. /2</td>
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<tr>
<td></td>
<td>102&quot; to 180&quot;</td>
<td>48&quot;</td>
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<tr>
<td>PIPE ARCH (SPAN)</td>
<td>16&quot; to 39&quot;</td>
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<td>METAL ONLY</td>
<td>43&quot; to 142&quot;</td>
<td>SPAN /3</td>
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<td>148&quot; to 20X&quot;</td>
<td>48&quot;</td>
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CONCRETE COLLAR OPTION

NOTES

1. The Concrete Collar width shall be one half of the outside pipe diameter of the largest pipe. The minimum Concrete Collar width shall be 12". Concrete Collars may be used with all pipe materials and diameters. The Concrete Collar option shall only be used to extend existing pipes.

2. Steel Welded Wire Fabric shall be in accordance with Standard Specification 9-07.7. Install two wraps for sizes 6 x 8 W1.4 x W1.4 (10 gage) Steel Welded Wire Fabric or one wrap for any of the following sizes:
   - 6 x 6 W2.1 x W2.1 (8 gage)
   - 0 x 6 W2.8 x W2.8 (6 gage)
   - 4 x 4 W2.9 x W2.9 (6 gage)
   - 4 x 4 W4.0 x W4.0 (4 gage)

3. When a Coupling Band connection requires attachment to the bell end of a concrete pipe, the bell end of the pipe shall be removed before the connection is installed.

4. Increase the outside diameter of the metal pipe to match the outside diameter of the concrete pipe by installing 12" wide rubber gaskets, thickness as required (Coupling Band only). The rubber gaskets shall be in accordance with Standard Specification 9-04.4.3

5. Use a flat Type K Coupling Band. Type K Coupling Bands with dimples are not allowed for the installation detail shown. The Coupling Band option shall only be used for extending existing pipes that have an inside diameter of 36" or less.

CONNECTION DETAILS FOR DISSIMILAR CULVERT PIPE

STANDARD PLAN B-60.20-00

Sheet 1 of 1 Sheet

APPROVED FOR PUBLICATION

WASHINGTON STATE DEPARTMENT OF TRANSPORTATION

Signature: [Signature]

Date: 6/8/06

EXPRESS JULY 1, 2007

CONCRETE PIPE - SEE NOTE 3

RUBBER GASKETS - SEE NOTE 4

24" WEB, 1" THICK RUBBER GASKET
IN ACCORDANCE WITH STD. SPEC. B-04.4.3

TYPE "K" COUPLING BAND - SEE NOTE 5

CORRUGATED METAL PIPE

24" CIRC.
NOTES

1. The culvert ends shall be beveled to match the embankment or ditch slope and shall not be beveled flatter than 4H:1V. When slopes are between 4H:1V and 5H:1V, shape the slope in the vicinity of the culvert end to ensure that no part of the culvert protrudes more than 4" above the ground line.

2. Field cutting of culvert ends is permitted when approved by the Engineer. All field-cut culvert pipe shall be treated with treatment as shown in the Standard Specifications or General Special Provisions.

FOR CULVERTS 30" DIAMETER OR LESS

BEVELED END SECTIONS

STANDARD PLAN B-70.20-00

WASHINGTON STATE DEPARTMENT OF TRANSPORTATION
FOR SANITARY SEWER USE

8 INCH SEWER CLEAN-OUT
STANDARD PLAN B-85.40-00

CAST IRON RING AND COVER
NOTE
JOINTS MAY BE FORMED DURING INSTALLATION USING A RUBBER DIVIDER OR SAWCUT AFTER CONCRETE CURES TO MINIMUM STRENGTH.

EXTRUDED CURB
STANDARD PLAN F-10.42-00
Sheet 1 of 1 Sheet
APPROVED FOR PUBLICATION
MAILBOX SUPPORT
TYPE 1
STANDARD PLAN H-12
SHEET 1 OF 2 SHEETS

NOTES

1. A socket and wedge anchoring system that meets the NCHRP 350 crash test criteria may be substituted in lieu of the anti-twist plate designs shown. Anti-twist plates are not required for wood post installations.

2. The platform design shown on this plan features slots that accommodate several types of mailbox supports, only those slots necessary for assembling the type being installed are required. An adjustable platform may be used in lieu of this design, but it must fit the bracket design shown on this plan. Brackets are required for all single-post installations. Field drilling may be necessary.

3. Center the mailbox on the platform to ensure space for the mailbox door to open and to allow space for installing the fasteners (See ALIGNMENT DETAIL, Sheet 2). Spacing of mailbox mounting holes varies among manufacturers. Attachment of the mailbox to the platform may require drilling additional holes through the mailbox to fit the platform.

4. Attach a newspaper box to a steel post with two 1 7/8” Muffler Clamps spaced 4” apart. Field drill 7/16” holes in the newspaper box to fit. Use 2 1/2” x 1/4” lag bolts to attach newspaper boxes to wood posts. Newspaper boxes must not extend beyond the front of the mailbox when the mailbox door is closed.

5. A Type 2 Support (Standard Plan H-12a) is required when 2 or more mailboxes are to be installed on one support.
NOTES

1. The anchoring system shall meet NCHRP 350 crash test criteria. Use a socket and wedge system, or the anchoring system supplied by or recommended by the Type 2 Support manufacturer.

2. A maximum of 5 mailboxes may be installed on a Type 2 Support.

3. The platform design shown in this plan is detailed in the PLATFORM DETAIL, Standard Plan H-12, Sheet 2. This design features slots that accommodate several types of mailbox supports; only those slots necessary for assembling the type being installed are required. An adjustable platform may be used in lieu of this platform design. Adjustable platforms must fit the 1 7/8" M-Clamp.

4. Center the mailbox on the platform to ensure space for the mailbox door to open and to allow space for installing the fasteners (See ALIGNMENT DETAIL). Spacing of mailbox mounting holes varies among manufacturers. Attachment of the mailbox to the platform may require drilling additional holes through the mailbox to fit the platform.

5. Attach a newspaper box to a Type 2 Support with two 1 7/8" Muffer Clamps spaced 4" apart. Field drill 7/16" holes in the newspaper box to fit. Newspaper boxes must not extend beyond the front of the mailbox when the mailbox door is closed.
MAILBOX SUPPORT
TYPE 2
STANDARD PLAN H-12a
SHEET 2 OF 2 SHEETS

MAILBOX SUPPORT TYPE 1
(WOOD POST SHOWN)
FOR DETAILS
SEE STD. PLAN H-12

MAILBOX SUPPORTS TYPE 2

SPACING DETAIL

ANCHORING SYSTEM -
(SOCKET AND WEDGE SHOWN)
SEE NOTE 1

NEWSPAPER BOX - SEE NOTE 6

SNOw GUARD - WHEN REQUIRED,
PLACE ON LEADING END OF
SUPPORT (SEE DETAIL)
NOTES

1. MAXIMIZE DETENTION OF STORMWATER BY PLACING FENCE AS FAR AWAY FROM THE TOE OF SLOPE AS POSSIBLE WITHOUT ENCROACHING ON SENSITIVE AREAS OR OUTSIDE OF THE CLEARING BOUNDARIES.

2. INSTALL SILT FENCING ALONG CONTOURS WHENEVER POSSIBLE.

3. INSTALL THE ENDS OF THE SILT FENCE TO POINT SLIGHTLY UP-SLOPE TO PREVENT SEDIMENT FROM FLOWING AROUND THE ENDS OF THE FENCE.

4. PERFORM MAINTENANCE IN ACCORDANCE WITH STANDARD SPECIFICATIONS 8-01.3(9A) AND 8-01.3(15).

ELEVATION VIEW

6' MAX. SPACING WITHOUT BACKUP SUPPORT
10' MAX. SPACING WITH BACKUP SUPPORT

WHEN SPECIFIED IN THE CONTRACT, INSTALL BACKUP SUPPORT FOR THE GEOTEXTILE (SEE STD. SPEC. 8-01.3(9A)

GEOTEXTILE FOR TEMPORARY SILT FENCE (SEE STD. SPEC. 9-33.2, TABLE 9)

POST (SEE STD. SPEC. 8-01.3(9A))

BURY GEOTEXTILE IN TRENCH

SIDE VIEW

FLOW

BACKFILL WITH NATIVE SOIL AND COMPACT

STATE OF
WASHINGTON
LANDSCAPE ARCHITECT

MARK W. MEIN
CERTIFICATE NO. 006598
7/17/2003

SILT FENCE

STANDARD PLAN 1-4

SHEET 1 OF 1 SHEET

APPROVED FOR PUBLICATION

WASHINGTON STATE DEPARTMENT OF TRANSPORTATION
NOTES

1. PREFABRICATED UNITS MAY BE USED IN LIEU OF THE DESIGN SHOWN ON THIS PLAN UPON APPROVAL OF THE ENGINEER.

2. STRUCTURE SHALL BE CONSTRUCTED SUCH THAT GEOTEXTILE MATERIAL SHALL BE FASTENED TO POSTS CREATING A SEAMLESS JOINT.

3. ENSURE THAT PONDING HEIGHT OF WATER DOES NOT CAUSE FLOODING ON ADJACENT ROADWAYS OR PRIVATE PROPERTY.

4. PERFORM MAINTENANCE IN ACCORDANCE WITH STANDARD SPECIFICATION 8-01.3(15).

PLAN VIEW
(CROSS BRACES NOT SHOWN)

SECTION A

GEOTEXTILE FOR TEMPORARY SILT FENCE
(SEE STD. SPEC. 8-03.2, TABLE 8)

COMPACTED NATIVE SOIL

bury geotextile in 4" x 4" trench

POST (SEE STD. SPEC. 8-01.3(9A))

FLOW

ATTACH WOOD OR METAL CROSS BRACES TO STABILIZE POSTS

SILT FENCE (STD. PLAN 1-4)

FASTEN CROSS BRACES TOGETHER WITH SCREWS, NAILS, NYLON TIES OR WIRE

ISOMETRIC VIEW
(ENTIRE FENCE NOT SHOWN FOR ILLUSTRATIVE PURPOSES)
NOTES

1. PERFORM MAINTENANCE IN ACCORDANCE WITH STANDARD SPECIFICATION 5-01.3(15).

2. SIZE THE BELOW GRADE INLET DEVICE (BGID) FOR THE STORM WATER STRUCTURE IT WILL SERVICE.

3. THE BGID SHALL HAVE A BUILT-IN HIGH-FLOW RELIEF SYSTEM (OVERFLOW BYPASS).

4. THE RETRIEVAL SYSTEM MUST ALLOW REMOVAL OF THE BGID WITHOUT SPILLING THE COLLECTED MATERIAL.

PREFABRICATED BELOW GRADE INLET DEVICE DETAILS
NOTES
2. INSTALL THE SLOPED ENDS OF THE CHECK DAM A MINIMUM OF 3' HIGHER THAN THE TOP OF THE CHECK DAM IN THE CHANNEL TO ENSURE THAT WATER FLOWS OVER THE DAM AND NOT AROUND IT.
3. FLAT BOTTOM DITCH DESIGN SHOWN, CHECK DAM INSTALLATION DETAILS ARE SIMILAR FOR "V" BOTTOM DITCHES.
4. PERFORM MAINTENANCE IN ACCORDANCE WITH STANDARD SPECIFICATION 8.01.3(15).

DIG TRENCH APPROXIMATELY 8' WIDE AND DEEP, STAPLE END OF GEOTEXTILE AND BACKFILL WITH NATIVE MATERIAL
ORIENT THE SEWN EDGE OF THE CHECK DAM TOWARD THE UPSTREAM SIDE

STATE OF WASHINGTON
LICENSED ARCHITECT
MARY P. BAUMAN
CERTIFICATE NO. 03545
7/17/2003

GEOTEXTILE ENCASED
CHECK DAM
INSTALLATION
STANDARD PLAN 1-0

WASHINGTON STATE DEPARTMENT OF TRANSPORTATION
APPROVED FOR PUBLICATION
7-11-93

303000505
**Temporary Sediment Trap**

- **Elevation**
  - Outflow Channel:
    - 1'-0" Depth Overflow
    - 8'-0" Min.
  - Sediment Trap Bottom:
    - 1'-0" Depth of 3/4" - 1 1/2" Washed Gravel Backfill
  - Compacted Native Material:
    - Constructed by Excavation or Embankment

- **Section A**
  - 2'-0" Settling Depth
  - 1'-0" Sediment Storage
  - Quarry Spalls
  - Temporary Silt Fence

**Section View**

- Compost BERM Detail
  - X = 1'-0" for slopes 4H:1V or flatter
  - X = 1'-6" for slopes steeper than 4H:1V

**Isometric View**

- Stabilized Construction Entrance
  - 2'-0" R Min. (Typ.)
  - 4'-0" Quarry Spalls
  - As Required, 10'-0" Min.
  - Except, may be reduced to 50'-0" for sites with less than one acre of exposed soil.

**Miscellaneous Erosion Control Details**

- Standard Plan 1-14
- Sheet 1 of 1 Sheet

**Approval for Publication**

- Howard Rutland 7-17-03

- Washington State Department of Transportation
NOTES
1. THE SIGN SHOWN IS NOT REQUIRED IF THE WORK SPACE IS BEHIND A BARRIER, MORE THAN 2' BEHIND THE CURB, OR 15' OR MORE FROM THE EDGE OF ANY ROADWAY.

SIGN SPACING x K (feet)
- Rural Roads 45/55 MPH 500**
- Urban Arterials 35/40 MPH 350**
- Urban Streets
  Residential Areas & 25/30 MPH 200**
  Business Districts
- All signs are block on orange unless otherwise designated.

WORK BEYOND THE SHOULDER

1. SHOULDER EXCAVATION SHALL BE LIMITED TO ONE SIDE AT A TIME.

SHOULDER WORK AREAS
STANDARD PLAN K-11
SHEET 1 OF 1 SHEET
NOTES

1. A Protective Vehicle is recommended regardless if a Truck Mounted Attenuator (TMA) is available; a work vehicle may be used. When no TMA is used, the Protective Vehicle shall be strategically located to shield workers, with no specific Roll Ahead Stopping Distance.

2. Extend device taper (L) across shoulder – recommended.


4. Traffic Safety Drums for all tapers on high speed roadway – recommended.

5. Transverse Drums in closed lane every 100’ – recommended.

6. Channelizing Device spacing for the downstream taper option shall be 20’ O.C.

7. Use advance notice for any overwidth loads prior to lane closure for alternative routes if applicable – recommended.

8. For signs size refer to Manual on Uniform Traffic Control Devices (MUTCD) and WSDOT Sign Fabrication Manual M55-05.

FOR LOCAL AGENCY USE ONLY
NOT FOR USE ON STATE ROUTES

WASHINGTON STATE DEPARTMENT OF TRANSPORTATION
EFFECTIVE AUGUST 8, 2007

SINGLE LANE CLOSURE
WITH ENCROACHMENT
STANDARD PLAN K-24.20-00

SECTION A
NOTES

1. For sign installation details, see Std Plan 6 - series.
2. In rural areas, the "V" Height can be a minimum of 7 feet for primary signs and 8 feet for the supplemental plaque for greater visibility, as directed by the engineer.
3. The "V" height for signs, with an area of more than 50 square feet and two or more sign supports, is 7 feet in both rural and urban areas.

<table>
<thead>
<tr>
<th>HEIGHT V</th>
<th>TO BOTTOM OF SIGN</th>
<th>TO BOTTOM OF SIGN</th>
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<tbody>
<tr>
<td>(NO SUPPLEMENTAL PLAQUE)</td>
<td>(WHEN REQUIRED)</td>
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</tr>
<tr>
<td>RURAL</td>
<td>6 MINIMUM</td>
<td>4 MINIMUM</td>
</tr>
<tr>
<td>URBAN</td>
<td>7 MINIMUM</td>
<td>6 MINIMUM</td>
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</table>
Traffic Control Plans
NOTE: CONTRACTOR IS RESPONSIBLE FOR SUBMITTING SITE SPECIFIC TRAFFIC CONTROL PLANS TO THE PROJECT ENGINEER FOR REVIEW AND APPROVAL.
# GENERAL TRAFFIC CONTROL SIGN SPECIFICATIONS

<table>
<thead>
<tr>
<th>SIGN NO.</th>
<th>MUTCD SIGN #:</th>
<th>LOCATION</th>
<th>SIGN SIZE</th>
<th>POST MATERIAL</th>
<th>POST SIZE</th>
<th>POST LENGTH</th>
<th>CLEARANCE</th>
<th>NOTES</th>
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<tbody>
<tr>
<td>1</td>
<td>W20-1</td>
<td>MAPLE GROVE ROAD, 650 FT SOUTH OF WOODEN ROAD, W.</td>
<td>45° 46°</td>
<td>WOOD</td>
<td>4&quot;x4&quot;</td>
<td>16'</td>
<td>7'</td>
<td>T10'</td>
</tr>
<tr>
<td>2</td>
<td>620-3</td>
<td>MAPLE GROVE ROAD, 500 FT SOUTH OF WOODEN ROAD, W.</td>
<td>36° 18°</td>
<td>WOOD</td>
<td>4&quot;x4&quot;</td>
<td>14'</td>
<td>7'</td>
<td>T10'</td>
</tr>
<tr>
<td>3</td>
<td>620-2</td>
<td>MAPLE GROVE ROAD, 500 FT NORTH OF WOODEN ROAD, W.</td>
<td>45° 46°</td>
<td>WOOD</td>
<td>4&quot;x4&quot;</td>
<td>14'</td>
<td>7'</td>
<td>T10'</td>
</tr>
<tr>
<td>4</td>
<td>W20-1</td>
<td>MAPLE GROVE ROAD, 600 FT NORTH OF WOODEN ROAD, W.</td>
<td>45° 46°</td>
<td>WOOD</td>
<td>4&quot;x4&quot;</td>
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<td>7'</td>
<td>T10'</td>
</tr>
<tr>
<td>5</td>
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<td>SGOON ROAD, 575 FT SOUTH OF WOODEN ROAD, W.</td>
<td>36° 18°</td>
<td>WOOD</td>
<td>4&quot;x4&quot;</td>
<td>14'</td>
<td>7'</td>
<td>8'</td>
</tr>
<tr>
<td>6</td>
<td>W20-1</td>
<td>SGOON ROAD, 450 FT SOUTH OF WOODEN ROAD, W.</td>
<td>45° 46°</td>
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<td>4&quot;x4&quot;</td>
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<td>7'</td>
<td>8'</td>
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<tr>
<td>7</td>
<td>620-2</td>
<td>SGOON ROAD, 575 FT NORTH OF WOODEN ROAD, W.</td>
<td>45° 46°</td>
<td>WOOD</td>
<td>4&quot;x4&quot;</td>
<td>10'</td>
<td>7'</td>
<td>8'</td>
</tr>
</tbody>
</table>

**NOTE:** CONTRACTOR IS RESPONSIBLE FOR SUBMITTING SITE-SPECIFIC TRAFFIC CONTROL PLANS TO THE PROJECT ENGINEER FOR REVIEW AND APPROVAL.

---

**TYPICAL SIGN INSTALLATION**

NTS

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**NOTES:**

1. MUTCD (MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES).
2. FOR STRUCTURE AND MOUNTING DETAILS, SEE STANDARD PLANS FOR ROAD AND BRIDGE CONSTRUCTION, SERIES H.
3. FOR CODE REFERENCES AND STANDARD SIGN LAYOUT DETAILS, SEE STANDARD HIGHWAY SIGN BOOK.
4. POST LENGTHS SHOWN ARE APPROXIMATE. FINAL VALUES SHALL BE DETERMINED IN THE FIELD BY THE CONTRACTOR.
5. W-DISTANCE FROM THE EXISTING SHOULDER OR FACE OF CURB TO THE SIGN POST.
6. ALL SIGNS, POSTS AND ANY OTHER TRAFFIC CONTROL DEVICES SHALL BE SUPPLIED, ERECTED AND MAINTAINED BY THE CONTRACTOR.
7. THE POSTS SHALL NOT PROTRUDE ABOVE THE SIGNS.
# General Traffic Control Sign Specifications

<table>
<thead>
<tr>
<th>Sign No.</th>
<th>MUTCD Sign #</th>
<th>Location</th>
<th>Sign Size</th>
<th>Post Material</th>
<th>Post Size</th>
<th>Post Length</th>
<th>Clearance</th>
<th>Notes</th>
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<tr>
<td>1</td>
<td>620-2</td>
<td>Snipes Canal Road, 500 ft South of Swan Road</td>
<td>36&quot;</td>
<td>Wood</td>
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<td>Snipes Canal Road, 400 ft South of Swan Road</td>
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<td>Wood</td>
<td>4&quot;x4&quot;</td>
<td>18&quot;</td>
<td>7</td>
<td>10'</td>
</tr>
<tr>
<td>3</td>
<td>620-2</td>
<td>Swan Road, 500 ft South of W. Edison Road</td>
<td>36&quot;</td>
<td>Wood</td>
<td>4&quot;x4&quot;</td>
<td>15&quot;</td>
<td>7</td>
<td>10'</td>
</tr>
<tr>
<td>4</td>
<td>620-1</td>
<td>Swan Road, 250 ft South of W. Edison Road</td>
<td>46&quot;</td>
<td>Wood</td>
<td>4&quot;x4&quot;</td>
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<td>5</td>
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<td>Wood</td>
<td>4&quot;x4&quot;</td>
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<td>7</td>
<td>MG2-1</td>
<td>Alm. Crescent Avenue, 250 ft East of Swan Road</td>
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<td>Wood</td>
<td>4&quot;x4&quot;</td>
<td>16&quot;</td>
<td>7</td>
<td>10'</td>
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<td>8</td>
<td>MG2-2</td>
<td>Alm. Crescent Avenue, 510 ft East of Swan Road</td>
<td>36&quot;</td>
<td>Wood</td>
<td>4&quot;x4&quot;</td>
<td>16&quot;</td>
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<td>10'</td>
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<td>9</td>
<td>MG2-1</td>
<td>Edison Road, W., 250 ft East of ECP</td>
<td>46&quot;</td>
<td>Wood</td>
<td>4&quot;x4&quot;</td>
<td>17&quot;</td>
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</table>

**Note:** Contractor is responsible for submitting site specific traffic control plans to the project engineer for review and approval.

**Typical Sign Installation**

**Notes:**
1. MUTCD (Manual on Uniform Traffic Control Devices).
2. For structure and mounting details, see Standard Plans for Road and Bridge Construction, Series 6.
3. For code references and standard sign layout details, see Standard Highway Sign Book.
4. Post lengths shown are approximate. Final values shall be determined in the field by the contractor.
5. W-distance from the existing shoulder, or face of curb, to the sign post.
6. All signs, posts and any other traffic control devices shall be supplied, erected and maintained by the contractor.
7. The posts shall not protrude above the signs.
Improvement Plans
<table>
<thead>
<tr>
<th>ITEM NO.</th>
<th>ITEM DESCRIPTION</th>
<th>UNIT</th>
<th>TOTAL QUANTITY</th>
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<td>DRAINAGE SPILLS (INCL. MEASURES)</td>
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<td>PIPELINE, BACKFILL FOR PIPE BORING AND TRENCH</td>
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</tbody>
</table>
WOODIN ROAD, W.

WOODIN ROAD, W.
IMPROVEMENT PROJECT

MAPLE GROVE ROAD
TO SCOWN ROAD
C 3120

PREPARED UNDER
THE DIRECTION OF:

COUNTY ENGINEER
DATE: 6/24/7

PROJECT ENGINEER:
MARK DEZOSA

PAVEMENT MARKINGS
BOP STA. 10+50.00
TO
STA. 20+00

CHANNELIZATION NOTES

1. PAINTED YELLOW SKIP CENTER LINE.
2. PAINTED EDGE LINE.
3. PAINTED YELLOW NO-PASS LINE.

CENTER OF ROAD

- TYPICAL DOUBLE YELLOW CENTER LINE
- TYPICAL YELLOW NO-PASS LINE
- TYPICAL SKIP PATTERN

NOTE:
1) THE PAVEMENT MARKINGS SHALL BE SPOTTED BY THE ENGINEER PRIOR TO PAINTING. THE ENGINEER SHALL BE NOTIFIED AT LEAST 5 WORKING DAYS PRIOR TO PAINTING TO SPOT THE PAVEMENT MARKINGS.

NOTE:
FOR REFERENCE ONLY. NOT PART OF THIS PROJECT. WORK TO BE PERFORMED BY OTHERS.

SEC. 23, T.10 N., R.22 E., W.M.

SHEET 1 OF 6
WOODIN ROAD, W.

CHANNELIZATION NOTES

1. PAINTED YELLOW SKIP CENTER LINE.
2. PAINTED EDGE LINE.
3. PAINTED YELLOW NO-PASS LINE.

CENTER OF ROAD

TYPICAL DOUBLE YELLOW CENTER LINE

TYPICAL YELLOW NO-PASS LINE

TYPICAL SKIP PATTERN

NOTES:
1) THE PAVEMENT MARKINGS SHALL BE SPOTTED BY THE ENGINEER PRIOR TO PAINTING. THE ENGINEER SHALL BE NOTIFIED AT LEAST 5 WORKING DAYS PRIOR TO PAINTING TO SPOT THE PAVEMENT MARKINGS.

NOTE:
FOR REFERENCE ONLY. NOT PART OF THIS PROJECT. WORK TO BE PERFORMED BY OTHERS.
WOODIN ROAD, W.

CHANNELIZATION NOTES

1) PAINTED YELLOW SKIP CENTER LINE.
2) PAINTED EDGE LINE.
3) PAINTED YELLOW NO-PASS LINE.

CENTER OF ROAD

TYPICAL DOUBLE YELLOW CENTER LINE

TYPICAL YELLOW NO-PASS LINE

TYPICAL SKIP PATTERN

NOTES:
1) THE PAVEMENT MARKINGS SHALL BE SPOTTED BY THE ENGINEER PRIOR TO PAINTING. THE ENGINEER SHALL BE NOTIFIED AT LEAST 5 WORKING DAYS PRIOR TO PAINTING TO SPOT THE PAVEMENT MARKINGS.

NOTE:
FOR REFERENCE ONLY, NOT PART OF THIS PROJECT. WORK TO BE PERFORMED BY OTHERS.

40 0 40 80
SEC. 23, T.10 N., R.22 E., W.M.
WOODIN ROAD, W.

CHANNELIZATION NOTES

1. PAINTED YELLOW SKIP CENTER LINE.
2. PAINTED DOUBLE YELLOW CENTER LINE.
3. PAINTED EDGE LINE.
4. PAINTED YELLOW NO-PASS LINE.

CENTER OF ROAD

TYPICAL DOUBLE YELLOW CENTER LINE

TYPICAL YELLOW NO-PASS LINE

TYPICAL SKIP PATTERN

NOTES:
1) THE PAVEMENT MARKINGS SHALL BE SPOTTED BY THE ENGINEER PRIOR TO PAINTING. THE ENGINEER SHALL BE NOTIFIED AT LEAST 5 WORKING DAYS PRIOR TO PAINTING TO SPOT THE PAVEMENT MARKINGS.

NOTE:
FOR REFERENCE ONLY. NOT PART OF THIS PROJECT. WORK TO BE PERFORMED BY OTHERS.

MATCHLINE STA 40+00 TO STA 50+00

SEC. 23, T.10 N., R.22E., W.M.
WOODIN ROAD, W.

MATCHLINE STA 50+00

MATCHLINE STA 55+00

MATCHLINE STA 56+00

MATCHLINE STA 60+00

CHANNELIZATION NOTES

1. Painted Yellow Skip Line.
2. Painted Edge Line.
3. Painted Yellow No-Pass Line.

NOTE:
1) The pavement markings shall be spotted by the engineer prior to painting. The engineer shall be notified at least 5 working days prior to painting to spot the pavement markings.

NOTE:
For reference only, not part of this project. Work to be performed by others.

40 0 40 80
SEC. 23, T.10N., R.22E., W.M.
WOODIN ROAD, W.

MATCH EXISTING EDGE LINE

STA 62+00
STA 62+60
STA 63+10
EOP STA = 63+07.56

MATCH EXISTING EDGE LINE

SCAN ROAD

CHANNELIZATION NOTES

1. PAINTED YELLOW SKIP CENTER LINE.
2. PAINTED DOUBLE YELLOW CENTERLINE.
3. PAINTED EDGE LINE.
4. PAINTED YELLOW NO-PASS LINE.

CENTER OF ROAD

TYPICAL DOUBLE YELLOW CENTER LINE

TYPICAL YELLOW NO-PASS LINE

TYPICAL SKIP PATTERN

NOTES:
1) THE PAVEMENT MARKINGS SHALL BE SPOTTED BY THE ENGINEER PRIOR TO PAINTING. THE ENGINEER SHALL BE NOTIFIED AT LEAST 5 WORKING DAYS PRIOR TO PAINTING TO SPOT THE PAVEMENT MARKINGS.

NOTE:
FOR REFERENCE ONLY. NOT PART OF THIS PROJECT. WORK TO BE PERFORMED BY OTHERS.

WOODIN ROAD, W. IMPROVEMENT PROJECT

MAPLE GROVE ROAD TO SCNOC ROAD C 3120

PREPARED UNDER THE DIRECTION OF:

COUNTY ENGINEER DATE: 8/3/67

PROJECT ENGINEER:
MARK DRZOSKA

DRAWN:

CHECKED:

EXPIRES:
8/13/67

2739B

SHEET 6 OF 6
### SUMMARY OF QUANTITIES

<table>
<thead>
<tr>
<th>ITEM NO.</th>
<th>ITEM DESCRIPTION</th>
<th>UNIT</th>
<th>TOTAL QUANTITY</th>
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<td>MONOCULTURE</td>
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<td>L.S.</td>
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<tr>
<td>4</td>
<td>ASBESTOS INSULATION</td>
<td>L.S.</td>
<td>0</td>
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<tr>
<td>5</td>
<td>REMOV ING WIRE FENCE</td>
<td>L.P.</td>
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**GRADING**

<table>
<thead>
<tr>
<th>ITEM NO.</th>
<th>ITEM DESCRIPTION</th>
<th>UNIT</th>
<th>TOTAL QUANTITY</th>
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<tbody>
<tr>
<td>6</td>
<td>ROADWAY EXCAVATION, INC. PAUL</td>
<td>C.Y.</td>
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**DRAINAGE**

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<tr>
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<tbody>
<tr>
<td>7</td>
<td>QUANTITY SKEWS (STICK MEASURED)</td>
<td>C.Y.</td>
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<tr>
<td>8</td>
<td>UNDERGROUND PIPE 10 IN. DIAM.</td>
<td>L.P.</td>
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<tr>
<td>9</td>
<td>UNDERGROUND PIPE 12 IN. DIAM.</td>
<td>L.P.</td>
<td>0</td>
</tr>
<tr>
<td>10</td>
<td>SCHEDULE A GLV FITTING 12 IN. DIAM.</td>
<td>L.P.</td>
<td>0</td>
</tr>
<tr>
<td>11</td>
<td>SCHEDULE A GLV FITTING 16 IN. DIAM.</td>
<td>L.P.</td>
<td>0</td>
</tr>
<tr>
<td>12</td>
<td>SCHEDULE A GLV FITTING 24 IN. DIAM.</td>
<td>L.P.</td>
<td>0</td>
</tr>
<tr>
<td>13</td>
<td>SCHEDULE A GLV FITTING 36 IN. DIAM.</td>
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<td>0</td>
</tr>
<tr>
<td>14</td>
<td>CATCH BASSIN TYPE I</td>
<td>EACH</td>
<td>0</td>
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<tr>
<td>15</td>
<td>CATCH BASSIN TYPE 2</td>
<td>EACH</td>
<td>0</td>
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<tr>
<td>16</td>
<td>CATCH BASSIN TYPE 3 &amp; 4</td>
<td>EACH</td>
<td>4</td>
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<td>17</td>
<td>CATCH BASSIN TYPE 5 &amp; 6</td>
<td>EACH</td>
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<tr>
<td>18</td>
<td>SCHEDULE A STORM SEWER 12 IN. DIAM.</td>
<td>L.P.</td>
<td>323</td>
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**STRUCTURE**

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<th>ITEM DESCRIPTION</th>
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<tr>
<td>19</td>
<td>LEAN CONCRETE</td>
<td>C.Y.</td>
<td>0</td>
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<tr>
<td>20</td>
<td>CRUSHED SURFACING BASE COURSE</td>
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<td>21</td>
<td>CRUSHED SURFACING TOP COURSE</td>
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<td>22</td>
<td>ASPHALT CO2-20</td>
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<td>0</td>
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<td>23</td>
<td>BITUMINOUS SURFACE TREATMENT</td>
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<td>0</td>
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<td>24</td>
<td>ASPHALT CONCRETE PAYMENT</td>
<td>C.Y.</td>
<td>0</td>
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<tr>
<td>25</td>
<td>HMA FOR APPROACH</td>
<td>TON</td>
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<tr>
<td>26</td>
<td>PVC FOR REbars 4 IN.</td>
<td>L.P.</td>
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<tr>
<td>27</td>
<td>PVC FOR REbars 6 IN.</td>
<td>L.P.</td>
<td>500</td>
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<tr>
<td>28</td>
<td>PVC FOR APPROACH</td>
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**INTERSECTION LAYOUT**

**MAILBOX SUPPORT SCHEDULE**

<table>
<thead>
<tr>
<th>STATION</th>
<th>SUPPORT TYPE</th>
<th>NO. OF MAILBOXES</th>
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</table>
| 2-1/2 LT. | 1 | 1
| 4-2-1/2 LT. | 2 | 1
| 5-1/2 LT. | 1 | 1
| 10-1/2 LT. | 1 | 1
| 12-1/2 LT. | 2 | 1
| 17-1/2 LT. | 1 | 1
| 20-1/2 LT. | 1 | 1

**SUMMARY OF QUANTITIES, LAYOUT, & SCHEDULE**

**PROJECT ENGINEER:**

**DRAWN BY:**

**CHECKED BY:**

**COUNTY ENGINEER DATE:**
## APPROACH ROAD SCHEDULE

<table>
<thead>
<tr>
<th>APPROACH ROAD STATION</th>
<th>WIDTH (FT.)</th>
<th>SURFACING MATERIAL</th>
<th>HORIZONTAL DIMENSIONS (FT.)</th>
<th>VERTICAL CURVE LENGTHS (FT.)</th>
<th>SLOPES (DEG.)</th>
<th>QUANTITIES</th>
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<tbody>
<tr>
<td>D.F.T.</td>
<td></td>
<td></td>
<td>A</td>
<td>B</td>
<td>C</td>
<td>D</td>
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<tr>
<td>64.00 FT.</td>
<td>12.0</td>
<td>HMA</td>
<td>12.5</td>
<td>27.5</td>
<td>60.0</td>
<td>65.0</td>
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<tr>
<td>50.00 FT.</td>
<td>20.0</td>
<td>HMA</td>
<td>12.5</td>
<td>17.5</td>
<td>30.0</td>
<td>35.0</td>
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<tr>
<td>40.00 FT.</td>
<td>20.0</td>
<td>HMA</td>
<td>12.5</td>
<td>25.0</td>
<td>27.5</td>
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</tr>
<tr>
<td>50.00 FT.</td>
<td>20.0</td>
<td>HMA</td>
<td>12.5</td>
<td>27.5</td>
<td>30.0</td>
<td>5.0</td>
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<tr>
<td>40.00 FT.</td>
<td>20.0</td>
<td>GRAVEL</td>
<td>12.5</td>
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<tr>
<td>67.00 FT.</td>
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<td>GRAVEL</td>
<td>12.5</td>
<td>17.5</td>
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<td>35.0</td>
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<tr>
<td>50.00 FT.</td>
<td>20.0</td>
<td>GRAVEL</td>
<td>12.5</td>
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<tr>
<td>50.00 FT.</td>
<td>20.0</td>
<td>GRAVEL</td>
<td>12.5</td>
<td>---</td>
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<tr>
<td>50.00 FT.</td>
<td>20.0</td>
<td>HMA</td>
<td>12.5</td>
<td>27.5</td>
<td>50.0</td>
<td>65.0</td>
</tr>
<tr>
<td>65.00 FT.</td>
<td>20.0</td>
<td>HMA</td>
<td>12.5</td>
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</tr>
<tr>
<td>50.00 FT.</td>
<td>20.0</td>
<td>HMA</td>
<td>11.0</td>
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<tr>
<td>70.00 FT.</td>
<td>11.0</td>
<td>HMA</td>
<td>11.0</td>
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NOTE: CONSTRUCT 30' WIDE CURB DEPRESSIONS FOR SUNNYSIDE VALLEY BRIDGATION DISTRICT ACCESS NEAR STA. 1573, RT. 6+00, RT. AND STA. 14+50, RT.
VERIFY LOCATIONS PRIOR TO CONSTRUCTION. CALL (509) 935-6960. PLACE C.S.T.C. AS DIRECTED.

### APPROACH ROAD NOMENCLATURE

- **E EDISON RD., W.**
- **D (FT.)**
- **C (FT.)**
- **V.C. #1**
- **SLOPE A (DEG.)**
- **SLOPE B (DEG.)**
- **SLOPE C (DEG.)**
- **SLOPE D (DEG.)**
- **VPI #1**
- **VPI #2**
- **MATCH EXISTING**
- **LENGTH (FT.)**
- **LENGTH (FT.)**

### TYPICAL APPROACH ROAD SECTION

- **0.40' COMPACTED DEPTH CRUSHED SURFACING TOP COURSE**
- **0.20' COMPACTED DEPTH HMA FOR APPROACH**

NOTE: FOR GRAVEL APPROACH ROADS, PLACE 0.30' C.S.T.C ONLY.
## DRAINAGE STRUCTURE NOTES

<table>
<thead>
<tr>
<th>ITEM NO.</th>
<th>STATION/ OFFSET</th>
<th>CATCH BASIN DESCRIPTION</th>
<th>INLET ELEVATION</th>
<th>BM ELEVATION</th>
<th>I. E. OUT</th>
<th>I. E. IN</th>
<th>I. E. TRENCH</th>
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<tbody>
<tr>
<td>1</td>
<td>STA. 2+15: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>770.63</td>
<td>---</td>
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<tr>
<td>2</td>
<td>STA. 2+15: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 6' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 20.0'</td>
<td>779.63</td>
<td>---</td>
<td>770.20</td>
<td>770.41</td>
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<tr>
<td>3</td>
<td>STA. 2+15: 20.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 2, 45' IN. DIAM. WITH MANHOLE LDL AND 6'X5'X30' INFILTRATION TRENCH</td>
<td>---</td>
<td>770.60</td>
<td>774.00</td>
<td>775.00</td>
<td>772.00</td>
</tr>
<tr>
<td>4</td>
<td>STA. 5+15: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>775.33</td>
<td>---</td>
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<tr>
<td>5</td>
<td>STA. 5+15: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 175' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>775.00</td>
<td>775.12</td>
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<tr>
<td>6</td>
<td>STA. 7+15: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>774.99</td>
<td>774.00</td>
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<tr>
<td>7</td>
<td>STA. 7+15: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 5' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 24.0'</td>
<td>779.63</td>
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<td>773.97</td>
<td>773.99</td>
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<tr>
<td>8</td>
<td>STA. 7+15: 20.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 2, 45' IN. DIAM. WITH MANHOLE LDL AND 6'X5'X30' INFILTRATION TRENCH</td>
<td>---</td>
<td>770.60</td>
<td>775.30</td>
<td>772.50</td>
<td>769.50</td>
</tr>
<tr>
<td>9</td>
<td>STA. 13+25: 10.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>774.99</td>
<td>---</td>
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</tr>
<tr>
<td>10</td>
<td>STA. 13+25: 10.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>774.50</td>
<td>774.75</td>
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<tr>
<td>11</td>
<td>STA. 15+35: 10.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
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<td>773.10</td>
<td>768.50</td>
<td>767.50</td>
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<tr>
<td>12</td>
<td>STA. 17+00: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 21' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 0.0'</td>
<td>779.63</td>
<td>---</td>
<td>775.03</td>
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<tr>
<td>13</td>
<td>STA. 17+00: 12.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 1 AND 11' L.P. OF SCH. A STORM SEWER PIPE 12 IN. DIAM. AT 25.0'</td>
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<td>774.75</td>
<td>774.82</td>
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<tr>
<td>14</td>
<td>STA. 17+00: 25.0' LT.</td>
<td>INSTALL NEW CATCH BASIN TYPE 2, 45' IN. DIAM. WITH MANHOLE LDL AND 6'X5'X30' INFILTRATION TRENCH</td>
<td>---</td>
<td>770.70</td>
<td>771.00</td>
<td>772.00</td>
<td>769.00</td>
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</tbody>
</table>

---

EDISON ROAD, W. IMPROVEMENT PROJECT

SWAN ROAD TO SUNNYSIDE CITY LIMITS C 2963

PREPARED UNDER THE DIRECTION OF:

COUNTY ENGINEER:

DATE: 1/29/77

PROJECT ENGINEER: WILLIAM MAGEED

DRAGEN: J. POTTS

DRAINAGE STRUCTURE NOTES AND DETAILS

SHEET 4 OF 9
EDISON ROAD, W. IMPROVEMENT PROJECT

SWAN ROAD TO SUNNYSIDE CITY LIMITS C 2963

PREPARED UNDER THE DIRECTION OF:

COUNTY ENGINEER DATE: 5/27/87

PROJECT ENGINEER:
WILLIAM MAGNARD

CHECKED BY: G. FRENZEL

SIGN REMOVAL SPECIFICATIONS

<table>
<thead>
<tr>
<th>SIGN NO.</th>
<th>MUTCD SIGN #</th>
<th>LOCATION</th>
<th>SIGN SIZE</th>
<th>POST MATERIAL</th>
<th>POST SIZE</th>
<th>REMARKS</th>
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<tbody>
<tr>
<td>R2-1</td>
<td></td>
<td>EDISON ROAD, W, 50 FT EAST OF SWAN ROAD</td>
<td>30&quot; 30&quot;</td>
<td>WOOD 4½&quot;</td>
<td></td>
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<tr>
<td>3-101(600)</td>
<td>SAME</td>
<td>EDISON ROAD, W, 140 FT EAST OF SWAN ROAD</td>
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<td>2½&quot;2½&quot;</td>
<td>&quot;25 MPH&quot;</td>
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<tr>
<td>3-101(600)</td>
<td>SAME</td>
<td>EDISON ROAD, W, 255 FT EAST OF SWAN ROAD</td>
<td>30&quot; 30&quot;</td>
<td>METAL 2½&quot;2½&quot;</td>
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<td>&quot;25 MPH&quot;</td>
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<td>W1-5L</td>
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<td>EDISON ROAD, W, 255 FT EAST OF SWAN ROAD</td>
<td>18&quot; 18&quot;</td>
<td>SAME SAME</td>
<td></td>
<td>&quot;20 MPH&quot;</td>
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<tr>
<td>R5-1</td>
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<td>EDISON ROAD, W, 550 FT EAST OF SWAN ROAD</td>
<td>30&quot; 30&quot;</td>
<td>WOOD 4½&quot;</td>
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<tr>
<td>R2-1</td>
<td></td>
<td>EDISON ROAD, W, AT EOP</td>
<td>24&quot; 30&quot;</td>
<td>WOOD 4½&quot;</td>
<td></td>
<td>&quot;25 MPH&quot;</td>
</tr>
</tbody>
</table>

NOTES:
1. MUTCD (MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES).
2. FOR CODE REFERENCES AND STANDARD SIGN LAYOUT DETAILS, SEE STANDARD HIGHWAY SIGN BOOK.
3. THE SIGNS AND POSTS SHALL BE DISASSEMBLED AND DELIVERED TO THE YAKIMA COUNTY DEPARTMENT OF PUBLIC SERVICES MAINTENANCE SHOP AT 521 S. 1ST ST., SUNNYSIDE, WASHINGTON. CONTACT RON PRICE AT TELEPHONE NO. (509)-839-3430.
NOTE:
FOR REFERENCE ONLY. NOT PART OF THIS
PROJECT. WORK TO BE PERFORMED BY OTHERS.
PERMANENT SIGNING SPECIFICATIONS

<table>
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<th>METCD SIGN NO.</th>
<th>LOCATION</th>
<th>SIGN SIZE ONLY</th>
<th>SHEETING TYPE</th>
<th>POST MATERIAL</th>
<th>POST SIZE DIAM</th>
<th>POST LENGTH</th>
<th>CLEARANCE PT.13</th>
<th>REMARKS</th>
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<tr>
<td>1</td>
<td>RI-1</td>
<td>Edison Road, W, 30 ft East of Swan Road</td>
<td>36&quot; 36&quot; 18</td>
<td>METAL 2&quot;x2&quot;</td>
<td>13&quot; 7&quot; 10&quot;</td>
<td>MOUNTED ABOVE SIGN NO. 1, &quot;E EDISON RD&quot;</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>2</td>
<td>D3-1018MOOJ</td>
<td>Same</td>
<td>40&quot; 6&quot; 18</td>
<td>SAME SAME SAME</td>
<td>SAME</td>
<td>MOUNTED ABOVE SIGN NO. 2, &quot;SWAN RD&quot;</td>
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<td>TQ-1018MOOJ</td>
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<td>SAME SAME SAME</td>
<td>SAME 10.5&quot; 10&quot;</td>
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<tr>
<td>4</td>
<td>WC-1</td>
<td>Edison Road, W, 240 ft East of Swan Road</td>
<td>36&quot; 36&quot; 18</td>
<td>METAL 2&quot;x2&quot;</td>
<td>14&quot; 7&quot; 10&quot;</td>
<td>23 MPH</td>
<td></td>
<td></td>
<td></td>
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<tr>
<td>5</td>
<td>RO-1</td>
<td>Swan Road, N, 220 ft East of Swan Road</td>
<td>24&quot; 30&quot; 18</td>
<td>METAL 2&quot;x2&quot;</td>
<td>14&quot; 7&quot; 10&quot;</td>
<td>25 MPH</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>6</td>
<td>RO-2</td>
<td>Swan Road, N, 100 ft West of EOP</td>
<td>24&quot; 30&quot; 18</td>
<td>METAL 2&quot;x2&quot;</td>
<td>7&quot; 5.5&quot; 6&quot;</td>
<td>SPECIAL SIGN NO. 1, &quot;COUNTY&quot;</td>
<td></td>
<td></td>
<td></td>
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<tr>
<td>7</td>
<td>Special</td>
<td>Edison Road, N, AT EOP</td>
<td>5&quot; 10&quot; 18</td>
<td>METAL 2&quot;x2&quot;</td>
<td>7&quot; 5.5&quot; 6&quot;</td>
<td>SPECIAL SIGN NO. 2, &quot;CITY&quot;</td>
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</table>

YAKIMA COUNTY PUBLIC WORKS
ROAD NAME SIGN

NOTE: ROAD NAME ON BOTH SIDES.

E MAIN ST

NOTE: FOR REFERENCE ONLY. NOT PART OF THIS PROJECT. WORK TO BE PERFORMED BY OTHERS.

NOTES:
1. MUTCD (MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES).
2. FOR STRUCTURE AND MOUNTING DETAILS, SEE STANDARD PLANS FOR ROAD AND BRIDGE CONSTRUCTION, SERIES G.
3. FOR CODE REFERENCES AND STANDARD SIGN LAYOUT DETAILS, SEE STANDARD HIGHWAY SIGN BOOK.
4. POST LENGTHS SHOWN ARE APPROXIMATE. FINAL VALUES SHALL BE DETERMINED IN THE FIELD BY THE CONTRACTOR.
5. W = DISTANCE FROM THE EXISTING SHOULDER TO THE SIGN POST.
6. ALL SIGNS, POSTS AND ANY OTHER TRAFFIC CONTROL DEVICES SHALL BE SUPPLIED, ERECTED AND MAINTAINED BY THE CONTRACTOR.